



SCHEMATIC PLAN

HEN-NEW MAUMEE RIVER BRIDGE

S.R. 110
CURVE DATA
P.I. STA. 97+48.28
 $\Delta = 21^\circ 40' 09''$ (RT)
 $Dc = 2^\circ 00' 00''$
 $R = 2,864.80'$
 $T = 548.28'$
 $L = 1,083.46'$
 $E = 51.99'$
 $emax = 0.045$

S.R. 110
CURVE DATA
P.I. STA. 103+94.64
 $\Delta = 20^\circ 59' 50''$ (LT)
 $Dc = 9^\circ 32' 57''$
 $R = 600.00'$
 $T = 111.19'$
 $L = 219.88'$
 $E = 10.22'$
 $emax = NC$

S.R. 110
EX. CURVE DATA
P.I. STA. 90+33.37
 $\Delta = 49^\circ 52' 00''$ (RT)
 $Dc = 2^\circ 00' 00''$
 $R = 2,864.80'$
 $T = 1,331.82'$
 $L = 2,493.34'$
 $E = 294.45'$
 $emax = 0.045$

S.R. 110
CURVE DATA
P.I. STA. 105+83.15
 $\Delta = 18^\circ 08' 21''$ (RT)
 $Dc = 11^\circ 27' 33''$
 $R = 500.00'$
 $T = 79.81'$
 $L = 158.29'$
 $E = 6.33'$
 $emax = NC$

S.R. 110
CURVE DATA
P.I. STA. 107+61.70
 $\Delta = 11^\circ 25' 43''$ (RT)
 $Dc = 5^\circ 43' 46''$
 $R = 1,000.00'$
 $T = 100.06'$
 $L = 199.47'$
 $E = 4.99'$
 $emax = NC$

S.R. 110
CURVE DATA
P.I. STA. 111+03.62
 $\Delta = 10^\circ 20' 30''$ (LT)
 $Dc = 2^\circ 08' 16''$
 $R = 2,680.00'$
 $T = 242.53'$
 $L = 483.74'$
 $E = 10.95'$
 $emax = NC$

INDUSTRIAL DR.
CURVE DATA
P.I. STA. 33+37.88
 $\Delta = 52^\circ 05' 04''$ (LT)
 $Dc = 14^\circ 19' 26''$
 $R = 400.00'$
 $T = 195.46'$
 $L = 363.62'$
 $E = 45.20'$
 $emax = NC$

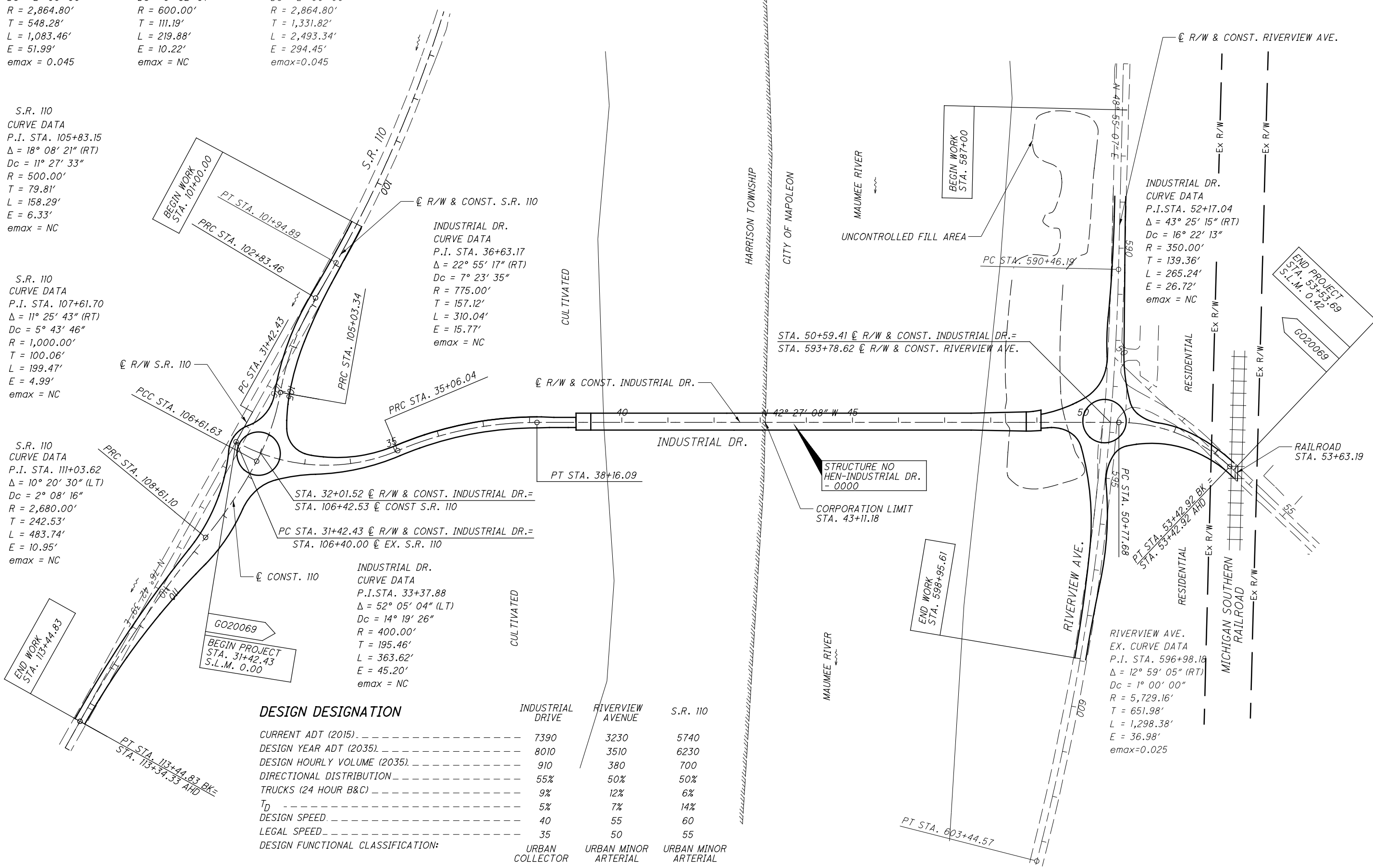
INDUSTRIAL DR.
CURVE DATA
P.I. STA. 36+63.17
 $\Delta = 22^\circ 55' 17''$ (RT)
 $Dc = 7^\circ 23' 35''$
 $R = 775.00'$
 $T = 157.12'$
 $L = 310.04'$
 $E = 15.77'$
 $emax = NC$

INDUSTRIAL DR.
CURVE DATA
P.I. STA. 52+17.04
 $\Delta = 43^\circ 25' 15''$ (RT)
 $Dc = 16^\circ 22' 13''$
 $R = 350.00'$
 $T = 139.36'$
 $L = 265.24'$
 $E = 26.72'$
 $emax = NC$

RIVERVIEW AVE.
EX. CURVE DATA
P.I. STA. 596+98.16
 $\Delta = 12^\circ 59' 05''$ (RT)
 $Dc = 1^\circ 00' 00''$
 $R = 5,729.16'$
 $T = 651.98'$
 $L = 1,298.38'$
 $E = 36.98'$
 $emax = 0.025$

DESIGN DESIGNATION

	INDUSTRIAL DRIVE	RIVERVIEW AVENUE	S.R. 110
CURRENT ADT (2015)	7390	3230	5740
DESIGN YEAR ADT (2035)	8010	3510	6230
DESIGN HOURLY VOLUME (2035)	910	380	700
DIRECTIONAL DISTRIBUTION	55%	50%	50%
TRUCKS (24 HOUR B&C)	9%	12%	6%
T _D	5%	7%	14%
DESIGN SPEED	40	55	60
LEGAL SPEED	35	50	55
DESIGN FUNCTIONAL CLASSIFICATION:	URBAN COLLECTOR	URBAN MINOR ARTERIAL	URBAN MINOR ARTERIAL

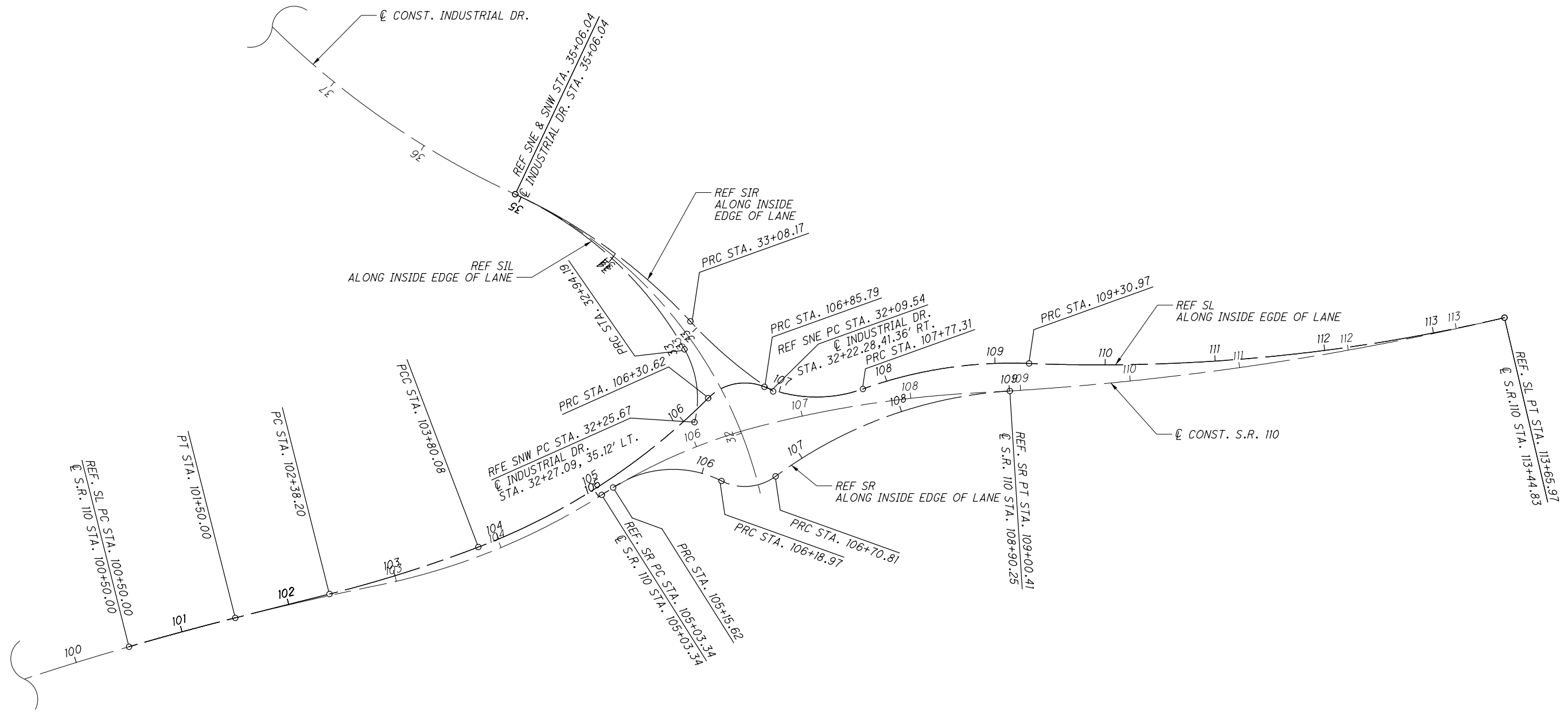


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PRIMARY PROJECT CONTROL INFORMATION				
POINT NO.	NORTHING	EASTING	ELEVATION	DESCRIPTION
ARP1	638573.6970	1528220.3138	678.11	IPINS /CIR
ARP2	638206.8882	1528219.4996	678.39	IPINS /CIR
ARP3	638499.0526	1529338.0111	674.81	IPINS /CIR
ARP4	637071.1187	1527468.4342	671.16	IPINS /CIR
ARP5	636091.1981	1528575.4343	667.46	IPINS /CIR
ARP6	635754.7549	1528038.3710	671.21	IPINS /CIR
JAS500	637266.4642	1527666.6053	670.59	MAG SPIKE
JEW1	636329.8015	1529263.7085	667.21	IPINS /CIR
JEW2	636549.8196	1530198.8952	665.77	IPINS /CIR
JEW3	636112.8419	1530882.4102	671.58	IPINS /CIR
JEW4	637659.8855	1528148.4581	678.13	IPINS /CIR
JEW5	637834.1903	1528370.4531	677.75	IPINS /CIR
JEW6	637483.2673	1528308.0899	657.38	IPINS /CIR
JEW7	637594.2634	1528367.7320	658.49	IPINS /CIR
JEW8	637725.4669	1528460.3426	660.03	MAG
JEW9	637889.5540	1528167.4761	678.45	IPINS /CAP
JEW10	637533.8146	1528056.8241	676.36	IPIN -CAP
JEW11	637404.8315	1527907.9181	675.19	IPIN -CAP
JEW12	637242.5817	1527729.1355	673.40	IPIN -CAP
JJL1	638118.1772	1528248.0499	678.06	IPINS /CIR
JJL2	637886.5906	1528504.3997	675.99	IPINS
JJL3	638056.5414	1528759.0865	674.94	IPINS
JJL4	638204.6910	1529006.4512	673.59	IPINS
JJL5	638131.5868	1528882.9361	674.10	MAGS
MSGBASE1	637271.9148	1526923.5899	678.73	IPINS /CIR

VERTICAL CONTROL INFORMATION			
POINT NO.	ELEVATION	DESCRIPTION	
BM18	680.83	CMON /BRASS DISC	STA. 65+94.80-134.94' LT CL INDUSTRIAL DR.
BM38	672.71	CMON /BRASS DISC	STA. 55+02.58-19.45' RT CL SR110
BM2	677.06	CMON /BRASS DISC	STA. 578+65.55-540.18' LT CL RIVERVIEW

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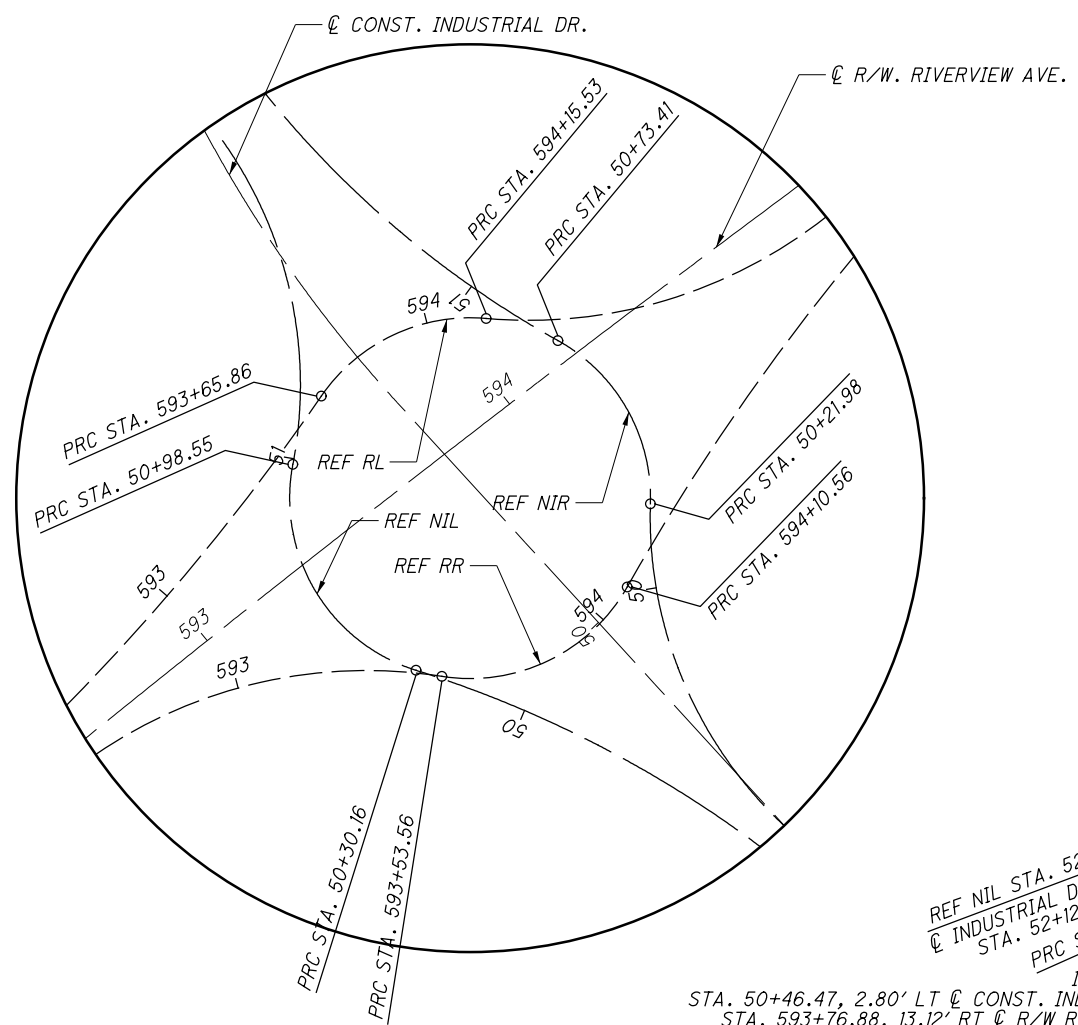
SEE SHEET 6 FOR GEOMETRIC DATA



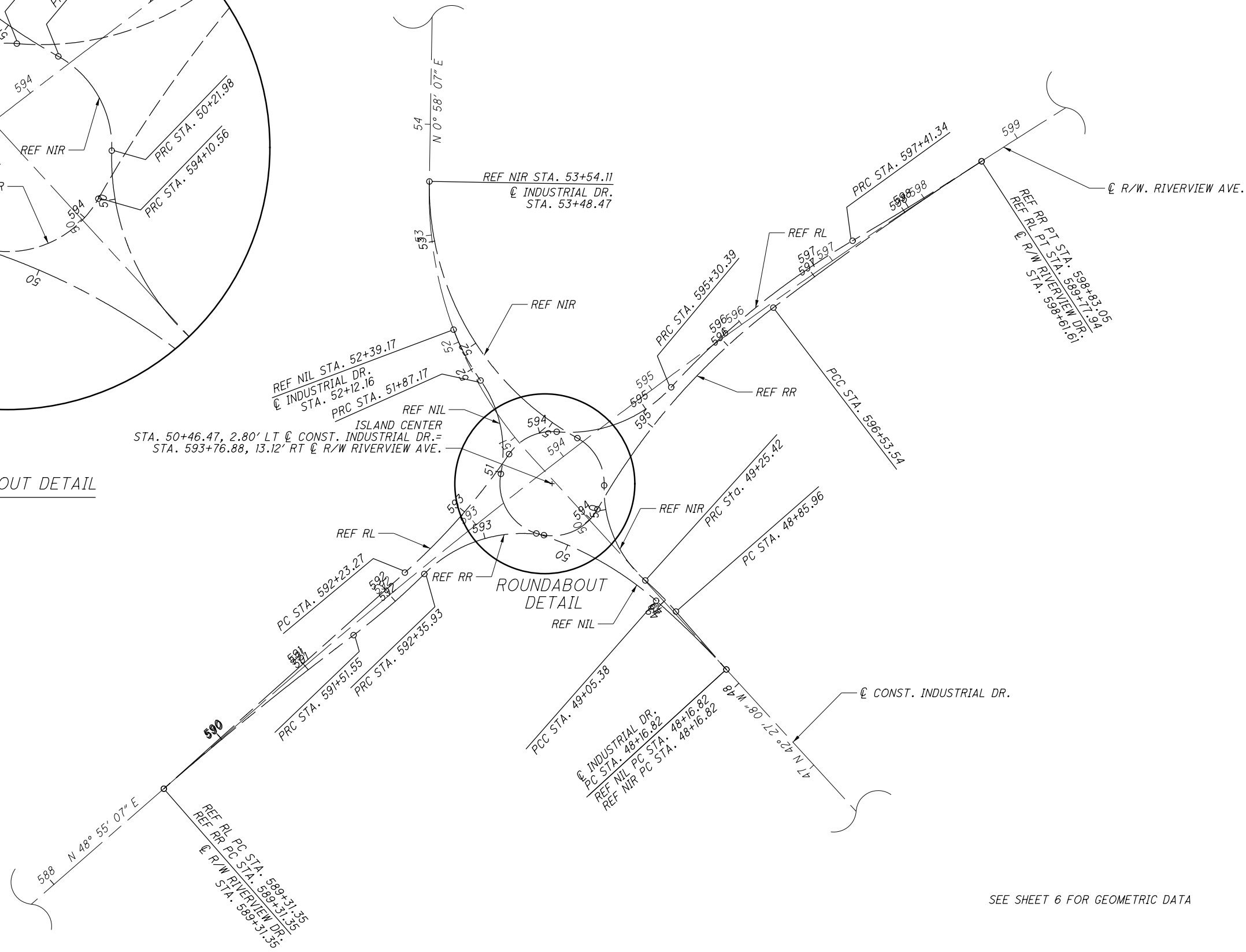
**ROUNDABOUT GEOMETRIC LAYOUT
S.R.110 & INDUSTRIAL DR.**

**HEN-NEW MAUMEE
RIVER BRIDGE**

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ROUNDABOUT DETAIL



ROUNDABOUT DETAIL

SEE SHEET 6 FOR GEOMETRIC DATA

CALCULATED

CHECKED

0 50 100

HORIZONTAL SCALE IN FEET

ROUNDABOUT GEOMETRIC LAYOUT
RIVERVIEW AVE. & INDUSTRIAL DR.

HEN-NEW MAUMEE
RIVER BRIDGE

REF. ALIGNMENT	P.I./P.O.T. STATION	NORTHING	EASTING	Degree	Delta	T (FT.)	L (FT.)	R (FT.)	BEGIN CURVE/CHAIN	END CURVE/CHAIN	AHEAD/BACK BEARING
SL		636247.79	1529035.18						100+50.00		
	101+00.01	636261.73	1529083.21	2° 00' 00"	2° 00' 00"	50.00	100.00	2864.80			
		636273.98	1529131.69							101+50.00	75° 48' 47"
		636295.60	1529217.19						102+38.20		
	103+09.22	636313.01	1529286.05	4° 40' 38"	6° 38' 10"	71.02	141.88	1225.00			
		636338.25	1529352.43							103+80.08	
	105+07.25	636383.46	1529471.30	9° 40' 42"	24° 14' 52"	127.17	250.54	592.00			
		636473.50	1529561.11							106+30.62	
	106+61.89	636495.63	1529583.19	121° 58' 06"	67° 17' 54"	31.27	55.18	46.98			
		636483.80	1529612.14							106+85.79	
	107+33.71	636465.68	1529656.50	45° 50' 12"	41° 56' 56"	47.92	91.52	125.00			
		636481.85	1529701.60							107+77.31	
	108+55.10	636508.10	1529774.83	14° 19' 26"	22° 00' 35"	77.79	153.66	400.00			
		636504.99	1529852.55							109+30.97	
	111+49.82	636496.25	1530071.23	3° 34' 52"	15° 34' 38"	218.85	435.00	1600.00			
	636546.56	1530284.22							113+65.97		
SR		636385.63	1529464.50						105+03.34		
	105+09.48	636388.93	1529469.67	9° 32' 57"	1° 10' 21"	6.14	12.28	600.00			
		636392.34	1529474.78							105+15.62	
	105+72.68	636423.99	1529522.26	58° 27' 54"	60° 25' 29"	57.07	103.35	98.00			
		636398.32	1529573.23							106+18.97	
	106+47.88	636385.31	1529599.05	121° 54' 21"	63° 11' 45"	28.91	51.84	47.00			
		636402.49	1529622.30							106+70.81	
107+88.87	636472.65	1529717.26	14° 19' 26"	32° 53' 17"	118.06	229.60	400.00				
	636480.00	1529835.09							109+00.41		
SIL		636451.62	1529548.76						32+25.67		
	32+61.69	636486.57	1529557.44	63° 39' 43"	43° 37' 15"	36.02	68.52	90.00			
		636517.87	1529539.61							32+94.19	
	34+03.68	636613.00	1529485.41	16° 51' 06"	35° 42' 04"	109.49	211.86	340.00			
	636658.63	1529385.87							35+06.04		
SIR		636479.63	1529620.09						32+09.54		
	32+59.11	636506.75	1529578.60	14° 19' 26"	14° 07' 41"	49.57	98.63	400.00			
		636543.18	1529544.99							33+08.17	
	34+08.42	636616.86	1529477.00	11° 27' 33"	22° 40' 28"	100.25	197.87	500.00			
	636658.63	1529385.87							35+06.04		

REF. ALIGNMENT	P.I./P.O.T. STATION	NORTHING	EASTING	Degree	Delta	T (FT.)	L (FT.)	R (FT.)	BEGIN CURVE/CHAIN	END CURVE/CHAIN	AHEAD/BACK BEARING
RL		637470.89	1527954.44							589+31.35	48° 07' 23"
		637665.76	1528171.80							592+23.27	
	592+94.90	637713.57	1528225.14	9° 32' 57"	13° 36' 58"	71.63	142.59	600.00			
		637772.60	1528265.72							593+65.86	
	593+93.30	637795.21	1528281.26	121° 54' 21"	60° 33' 21"	27.44	49.67	47.00			
		637792.79	1528308.59							594+15.53	
	594+77.38	637787.33	1528370.19	45° 50' 12"	52° 38' 48"	61.84	114.86	125.00			
		637832.99	1528411.91							595+30.39	
	596+36.67	637911.45	1528483.59	8° 11' 06"	17° 15' 58"	106.28	210.95	700.00			
		637965.10	1528575.34							597+41.34	
	598+09.65	637999.58	1528634.31	1° 54' 35"	2° 36' 32"	68.31	136.60	3000.00			
	638036.71	1528691.64							598+77.94		
RR		637470.89	1527954.44							589+31.35	
	590+41.50	637543.27	1528037.47	1° 54' 35"	4° 12' 20"	110.15	220.20	3000.00			
		637609.37	1528125.58							591+51.55	
	591+93.81	637634.73	1528159.39	9° 32' 57"	8° 03' 27"	42.26	84.38	600.00			
		637664.57	1528189.30							592+35.93	
	592+99.51	637709.48	1528234.31	45° 50' 12"	53° 54' 59"	63.58	117.63	125.00			
		637699.55	1528297.11							593+53.56	
	593+86.16	637694.46	1528329.31	121° 54' 21"	69° 29' 43"	32.60	57.00	47.00			
		637722.84	1528345.36							594+10.56	
	595+33.74	637830.06	1528405.99	9° 32' 57"	23° 12' 09"	123.18	242.97	600.00			
	637904.72	1528503.96							596+53.54		
597+68.35	637974.31	1528595.28	1° 54' 35"	4° 23' 00"	114.81	229.51	3000.00				
	638036.71	1528691.64							598+83.05		
NIL		637578.42	1528461.52							48+16.82	
	48+61.14	637611.12	1528431.60	7° 03' 53"	6° 15' 24"	44.33	88.56	811.00			
		637640.37	1528398.30							49+05.38	
	49+68.69	637682.14	1528350.73	19° 05' 55"	23° 49' 54"	63.31	124.78	300.00			
		637701.14	1528290.34							50+30.16	
	50+72.01	637713.70	1528250.42	121° 54' 21"	83° 22' 00"	41.85	68.39	47.00			
		637754.80	1528258.28							50+98.55	
	51+45.42	637800.84	1528267.09	52° 05' 14"	46° 09' 40"	46.87	88.62	110.00			
	637839.08	1528239.98							51+87.17		
52+13.32	637860.41	1528224.85	28° 38' 52"	14° 53' 47"	26.15	52.00	200.00				
	637884.91	1528215.72							52+39.17		
NIR		637578.42	1528461.52							48+16.82	40° 57' 08"
		637630.64	1528416.20							48+85.96	
	49+05.72	637645.56	1528403.25	19° 05' 55"	7° 32' 06"	19.76	39.45	300.00			
		637658.65	1528388.46							49+25.42	
	49+77.06	637692.88	1528349.79	52° 05' 13"	50° 17' 40"	51.64	96.56	110.00			
		637744.49	1528351.42							50+21.98	
	50+50.61	637773.11	1528352.33	121° 54' 21"	62° 41' 51"	28.63	51.43	47.00			
		637787.04	1528327.31							50+73.41	
	52+29.20	637862.83	1528191.20	22° 02' 13"	61° 51' 32"	155.79	280.71	260.00			
		638018.59	1528193.83							53+54.11	

HEN-NEW MAUMEE RIVER BRIDGE

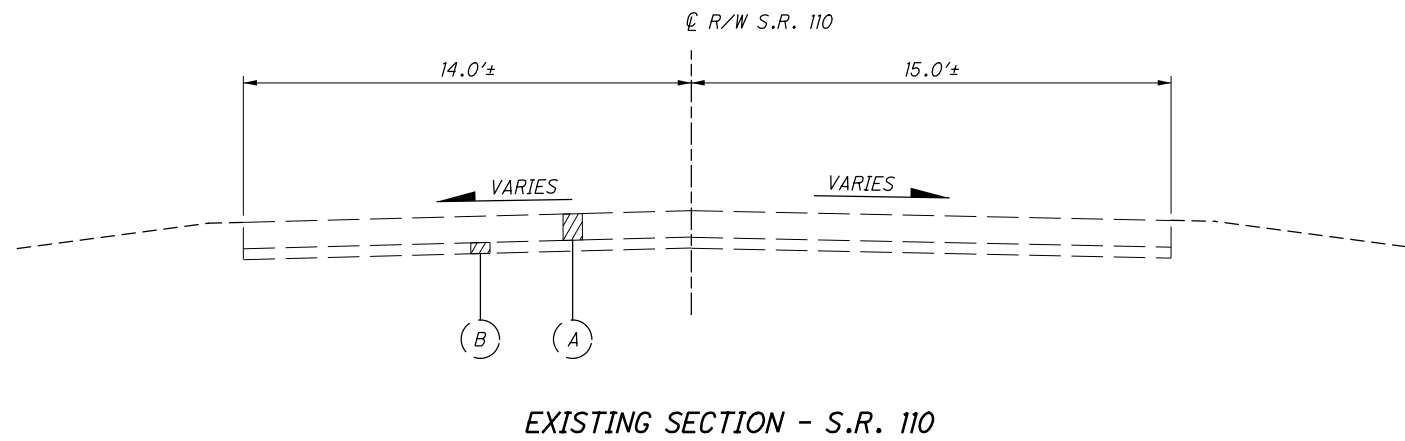
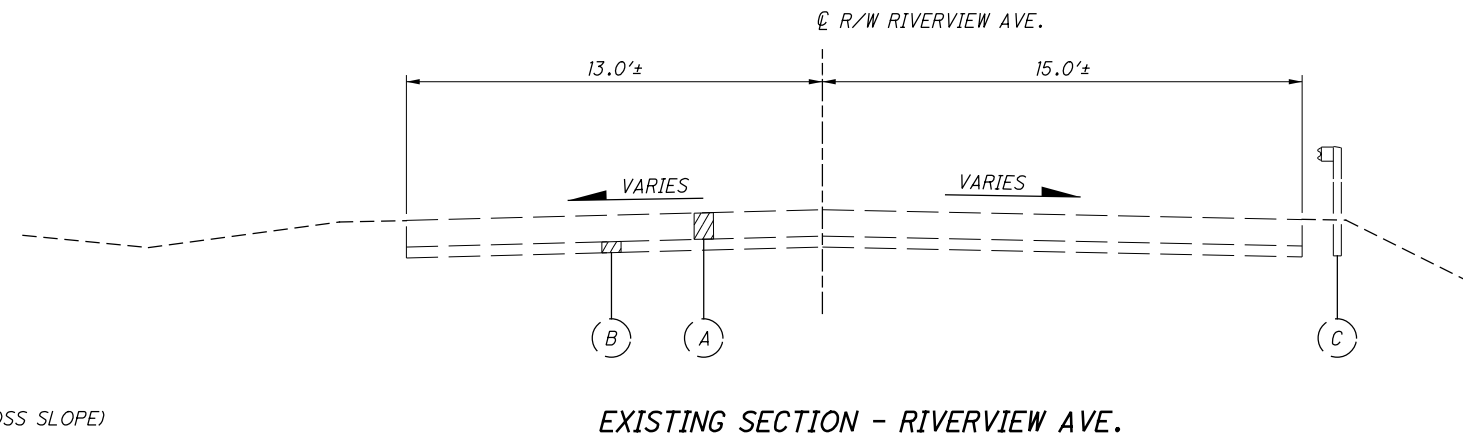
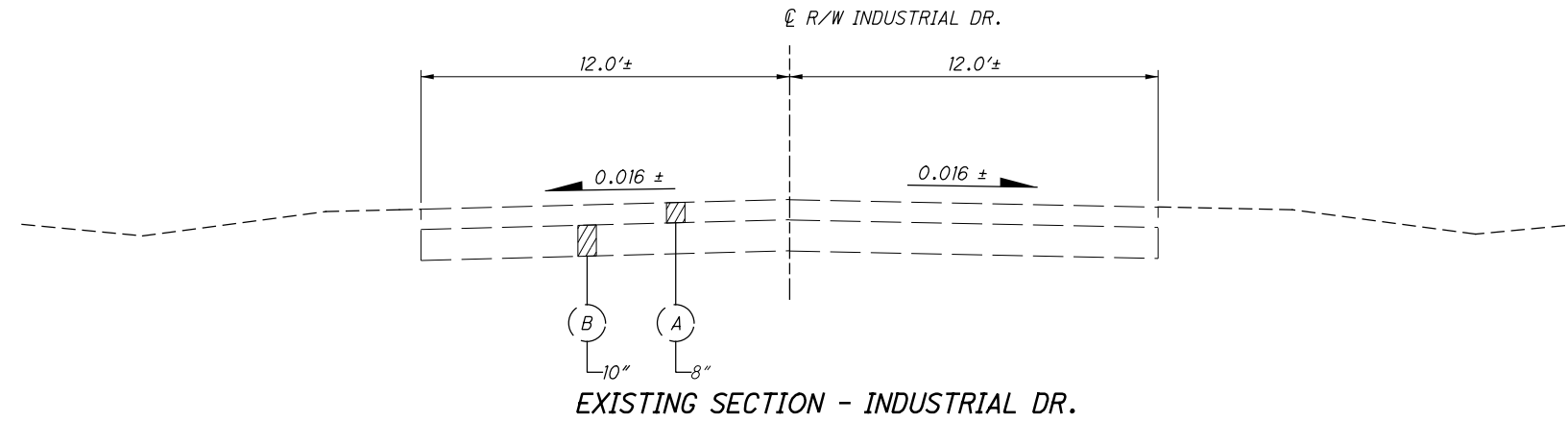
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LEGEND

- ① ITEM 441 - 1 1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (446), AS PER PLAN
- ② ITEM 442 - 1 3/4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (446), AS PER PLAN
- ③ ITEM 301 - 6" ASPHALT CONCRETE BASE, PG64-22
- ④ ITEM 304 - 6" AGGREGATE BASE
- ⑤ ITEM 407 - TACK COAT FOR INTERMEDIATE COURSE (0.04 GAL/SY)
- ⑥ ITEM 407 - TACK COAT (0.075 GAL/SY)
- ⑦ ITEM 203 - SUBGRADE COMPACTION
- ⑧ ITEM 605 - 6" SHALLOW PIPE UNDERDRAIN WITH FABRIC WRAP
- ⑨ ITEM 609 - COMBINATION CURB AND GUTTER, TYPE 2
- ⑩ ITEM 609 - COMBINATION CURB AND GUTTER, TYPE 3, AS PER PLAN
- ⑪ ITEM 609 - CURB, TYPE 2-A
- ⑫ ITEM 609 - CURB, TYPE 6
- ⑬ ITEM 452 - NON-REINFORCED CONCRETE PAVEMENT, MISC.: DECORATIVE, 8" THICK (TRUCK APRON)
- ⑭ ITEM 608 WALKWAY MISC.: DECORATIVE CONCRETE, 6" THICK (SPLITTER ISLAND)
- ⑮ ITEM 608 - 4" CONCRETE WALK
- ⑯ ITEM 659 - SEEDING AND MULCHING
- ⑰ ITEM 526 - REINFORCED CONCRETE APPROACH SLAB. (T=17"), AS PER PLAN
- ⑱ ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
- ⑲ ITEM 204 - EXCAVATION OF SUBGRADE
ITEM 204 - GRANULAR MATERIAL, TYPE B
- ⑳ ITEM 609 - CURB, TYPE 4C
- ㉑ ITEM 608 - 1" AGGREGATE WALK (LIMESTONE SCREENINGS)
- ㉒ ITEM 441 - ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 1, (448) (DEPTH VARIES TO ESTABLISH CROSS SLOPE)
- ㉓ ITEM 254 - PAVEMENT PLANING, ASPHALT CONCRETE (DEPTH VARIES FROM 1.25" TO 1.75")
- ㉔ ITEM 409 - SEALING MISC.: LONGITUDINAL JOINT SEALER

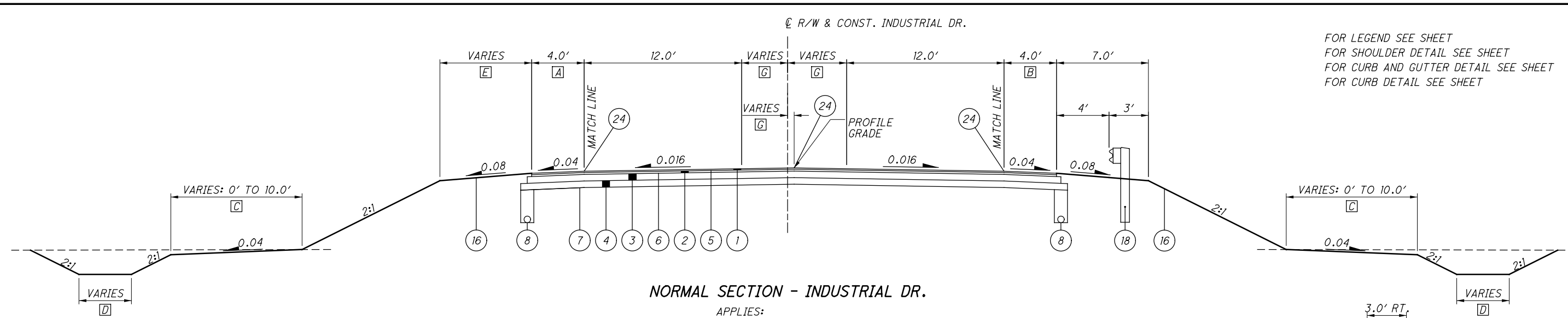
- (A) 10" ASPHALT (UNLESS OTHERWISE SHOWN)
- (B) 4" AGGREGATE BASE (UNLESS OTHERWISE SHOWN)
- (C) GUARDRAIL



TYPICAL SECTIONS - EXISTING ROAD

HEN-NEW MAUMEE RIVER BRIDGE

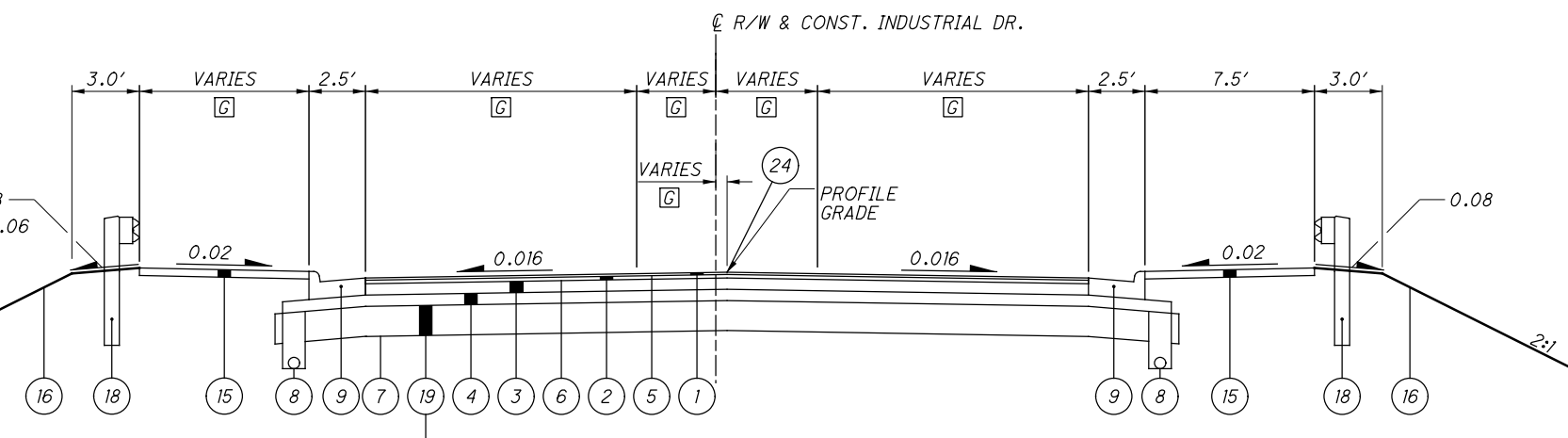
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NORMAL SECTION - INDUSTRIAL DR.

APPLIES:
STA. 33+64.83 TO STA. 35+06.04

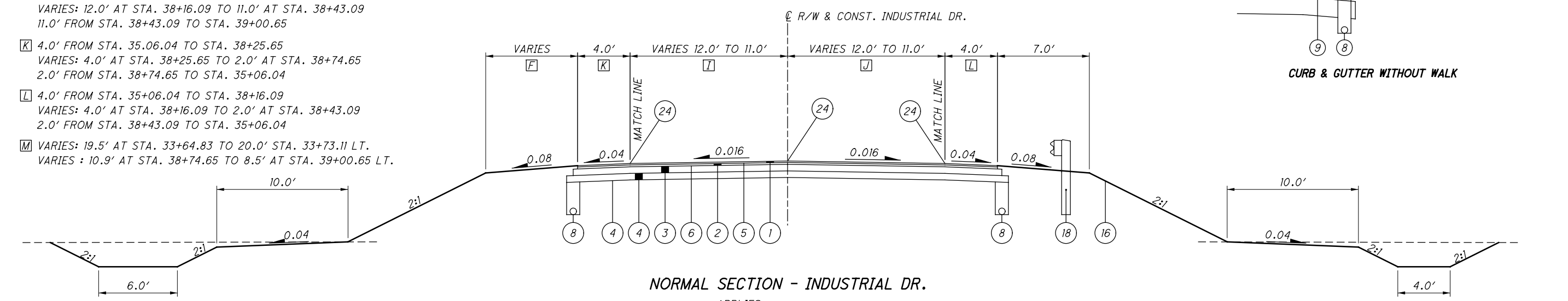
- [A] VARIES: 2.0' AT STA. 33+64.83 TO 4.0' AT STA. 33+73.11
4.0' FROM STA. 33+73.11 TO STA. 35+06.04
- [B] VARIES: 2.0' AT STA. 33+64.90 TO 4.0' AT STA. 33+72.99
4.0' FROM STA. 33+72.99 TO STA. 35+06.04
- [C] VARIES: 0' AT STA. 33+75.00 TO 10.0' AT STA. 34+75.00
10.0' FROM STA. 34+75 TO STA. 35+06.04
- [D] VARIES: 4.0' AT STA. 33+75.00 TO 6.0' AT STA. 34+75.00
6.0' FROM STA. 34+75 TO STA. 35+06.04
- [E] VARIES: 20.5' FROM STA. 33+74.49 TO 24.5' AT STA. 33+92.06
24.5' FROM STA. 33+92.06 TO STA. 35+06.04



NORMAL SECTION - INDUSTRIAL DR.

APPLIES:
STA. 49+08.32 TO STA. 49+27.57
STA. 52+15.45 TO STA. 53+64.45

- [F] 24.5' FROM STA. 35+06.04 TO STA. 37+65.15
VARIES: 24.5' AT STA. 37+65.15 TO 11.4' AT STA. 38+74.65
- [G] VARIES: SEE GEOMETRIC DETAILS
- [H] VARIES: SEE GRADING PLAN
- [I] 12.0' FROM STA. 35+06.04 TO STA. 38+16.09
VARIES: 12.0' AT STA. 38+16.09 TO 11.0' AT STA. 38+52.65
11.0' FROM STA. 38+52.65 TO STA. 39+00.65
- [J] 12.0' FROM STA. 35+06.04 TO STA. 38+16.09
VARIES: 12.0' AT STA. 38+16.09 TO 11.0' AT STA. 38+43.09
11.0' FROM STA. 38+43.09 TO STA. 39+00.65
- [K] 4.0' FROM STA. 35+06.04 TO STA. 38+25.65
VARIES: 4.0' AT STA. 38+25.65 TO 2.0' AT STA. 38+74.65
2.0' FROM STA. 38+74.65 TO STA. 35+06.04
- [L] 4.0' FROM STA. 35+06.04 TO STA. 38+16.09
VARIES: 4.0' AT STA. 38+16.09 TO 2.0' AT STA. 38+43.09
2.0' FROM STA. 38+43.09 TO STA. 35+06.04
- [M] VARIES: 19.5' AT STA. 33+64.83 TO 20.0' AT STA. 33+73.11 LT.
VARIES : 10.9' AT STA. 38+74.65 TO 8.5' AT STA. 39+00.65 LT.



NORMAL SECTION - INDUSTRIAL DR.

APPLIES:
STA. 35+06.04 TO STA. 39+00.65

FOR LEGEND SEE SHEET
FOR SHOULDER DETAIL SEE SHEET
FOR CURB AND GUTTER DETAIL SEE SHEET
FOR CURB DETAIL SEE SHEET

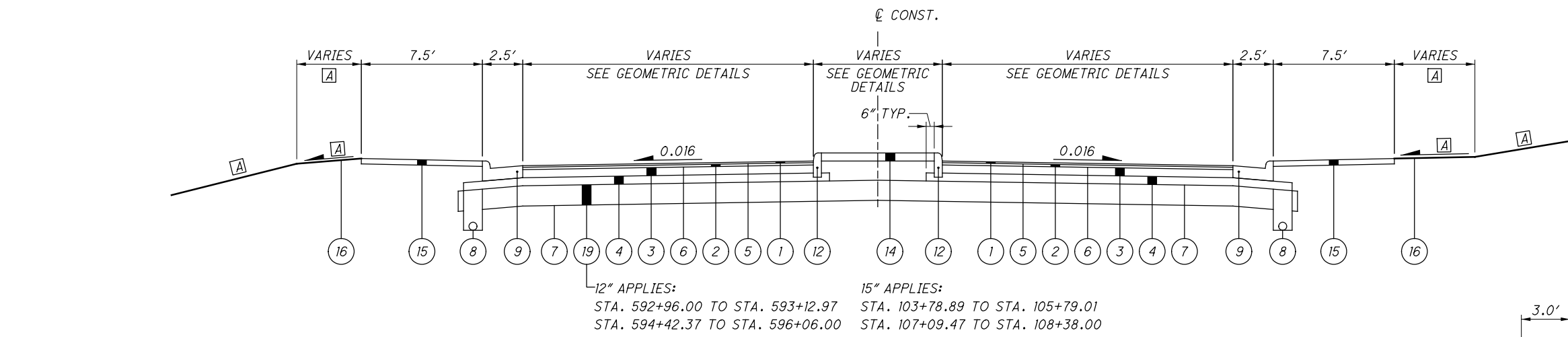
CURB SECTION
APPLIES:
STA. 33+64.83 TO STA. 33+73.11
STA. 38+43.09 TO STA. 39+00.65

CURB & GUTTER WITHOUT WALK

TYPICAL SECTIONS - INDUSTRIAL DR.

HEN-NEW MAUMEE RIVER BRIDGE

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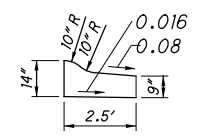
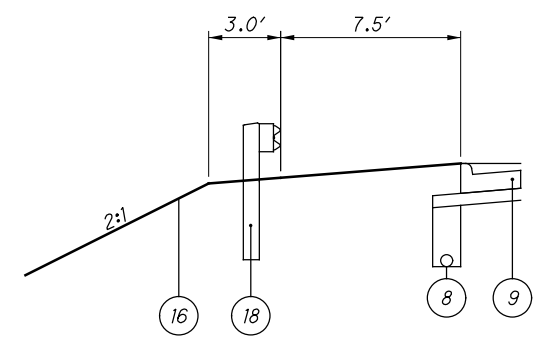
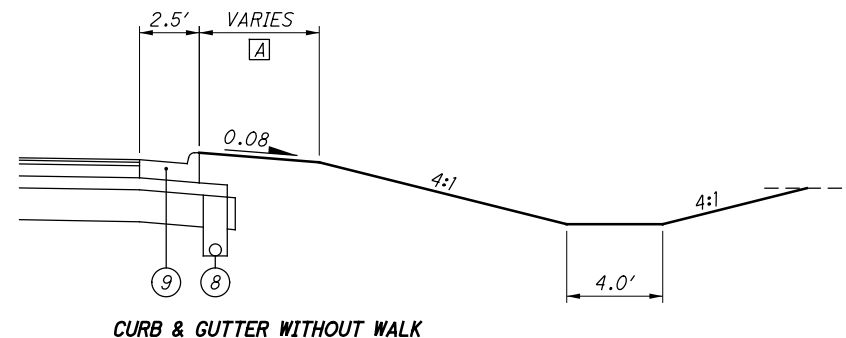


NORMAL SECTION - SPLITTER ISLANDS

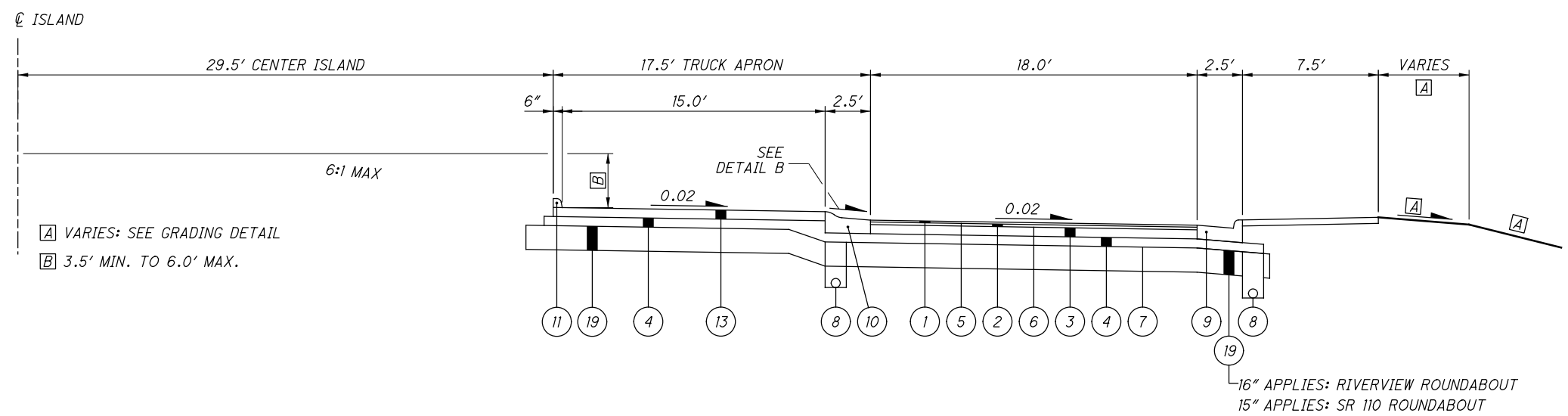
APPLIES:
 INDUSTRIAL DR.
 STA. 32+64.77 TO STA. 33+63.77
 STA. 49+27.51 TO STA. 49+81.05
 STA. 51+11.17 TO STA. 52+15.46

RIVERVIEW AVE.
 STA. 591+11.11 TO STA. 593+12.97
 STA. 594+42.37 TO STA. 596+34.91

S.R. 110
 STA. 103+78.89 TO STA. 105+79.01
 STA. 107+09.47 TO STA. 110+94.51



DETAIL B
 COMBINATION CURB AND GUTTER,
 TYPE 3, AS PER PLAN



NORMAL SECTION - ROUNDABOUT

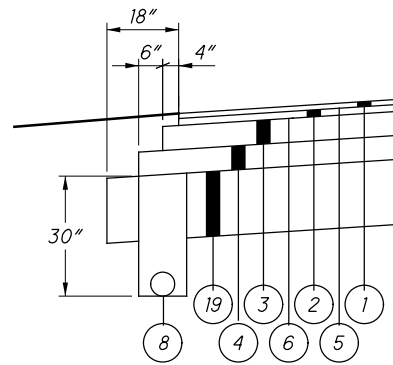
A VARIES: SEE GRADING DETAIL
B 3.5' MIN. TO 6.0' MAX.

FOR LEGEND SEE SHEET 7
 FOR SHOULDER DETAIL SEE SHEET
 FOR CURB AND GUTTER DETAIL SEE SHEET

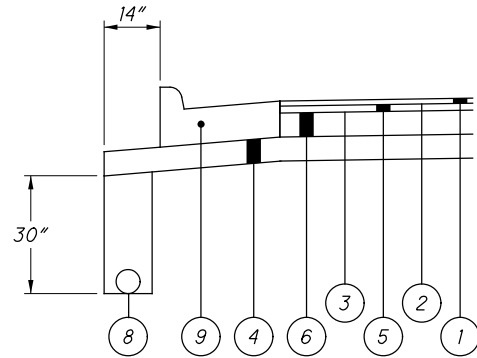
TYPICAL SECTIONS - ROUNDABOUT

HEN-NEW MAUMEE RIVER BRIDGE

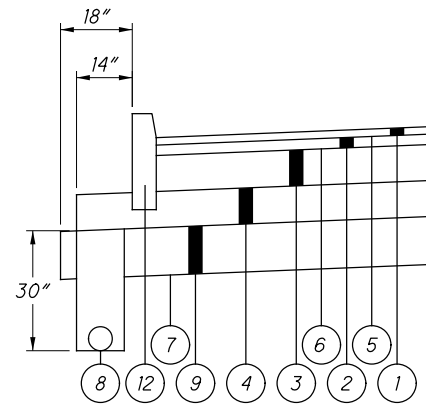
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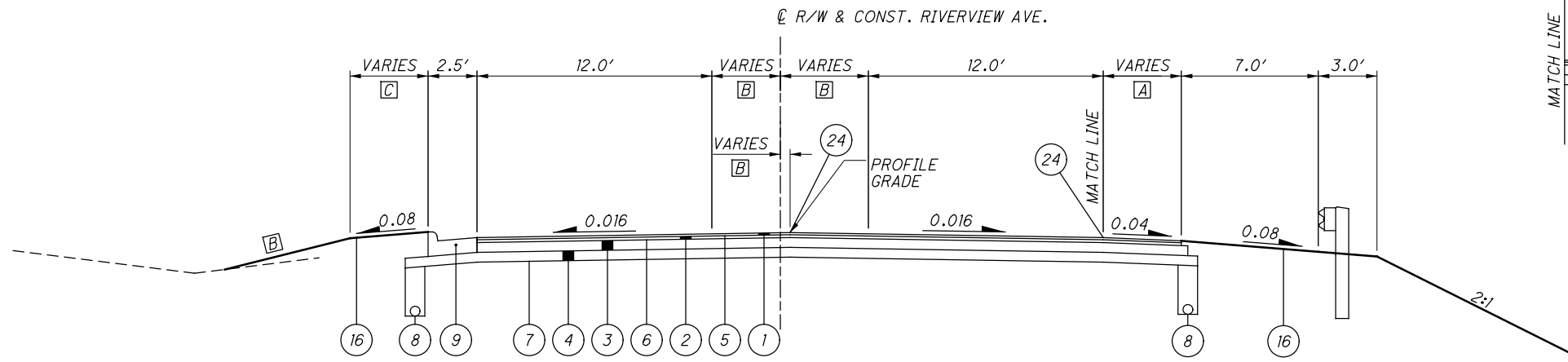
SHOULDER DETAIL



CURB & GUTTER DETAIL

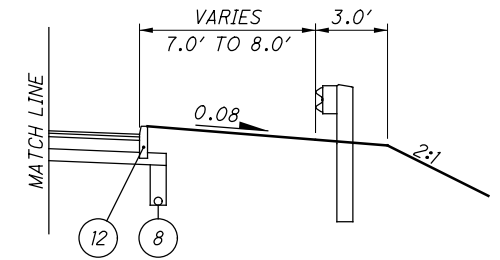


CURB DETAIL

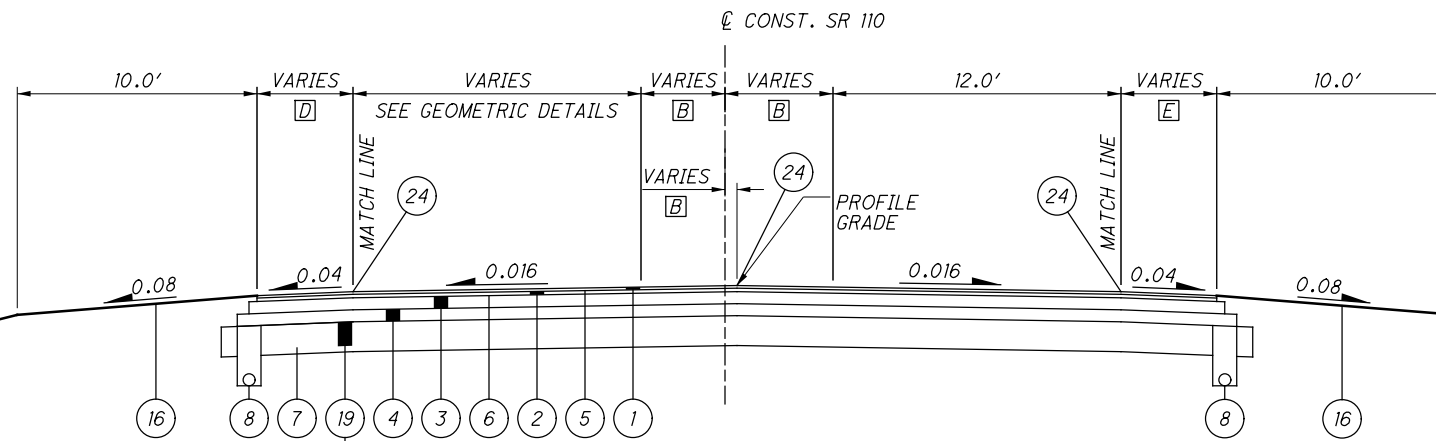


NORMAL SECTION - RIVERVIEW AVE.

- APPLIES:
 STA. 588+87.77 TO STA. 591+11.11
 STA. 596+34.91 TO STA. 598+95.61
- A 3.0' FROM STA. 588+87.77 TO STA. 591+06.20
 VARIES: 3.0' AT STA. 591+06.20 TO 2.0' AT STA. 591+10.33
 VARIES: 2.0' AT STA. 596+35.53 TO 3.0' AT STA. 596+39.63
 3.0' FROM STA. 596+39.63 TO STA. 598+95.61
 - B VARIES: SEE GEOMETRIC DETAILS
 - C VARIES: SEE GRADING DETAIL

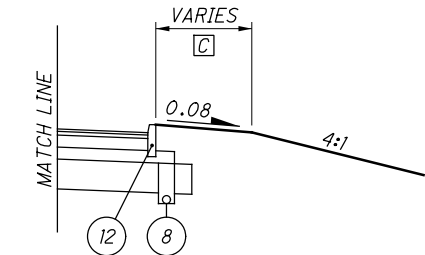


CURB SECTION
 APPLIES:
 STA. 591+06.20 TO STA. 591+10.33 RT.
 STA. 596+35.53 TO STA. 596+39.63 RT.



NORMAL SECTION - S.R. 110

- APPLIES:
 STA. 101+00.00 TO STA. 103+78.89
 STA. 110+94.51 TO STA. 113+44.83
- D 3.0' FROM STA. 101+00.00 TO STA. 103+74.75
 VARIES: 3.0' AT STA. 103+74.75 TO 2.0' AT STA. 103+78.86
 VARIES: 2.0' AT STA. 110+95.52 TO 3.0' AT STA. 110+99.70
 3.0' FROM STA. 110+99.70 TO STA. 113+44.83
 - E 3.0' FROM STA. 101+00.00 TO STA. 103+74.89
 VARIES: 3.0' AT STA. 103+74.89 TO 2.0' AT STA. 103+78.89
 VARIES: 2.0' AT STA. 110+94.51 TO 3.0' AT STA. 110+98.51
 3.0' FROM STA. 110+98.51 TO STA. 113+44.83

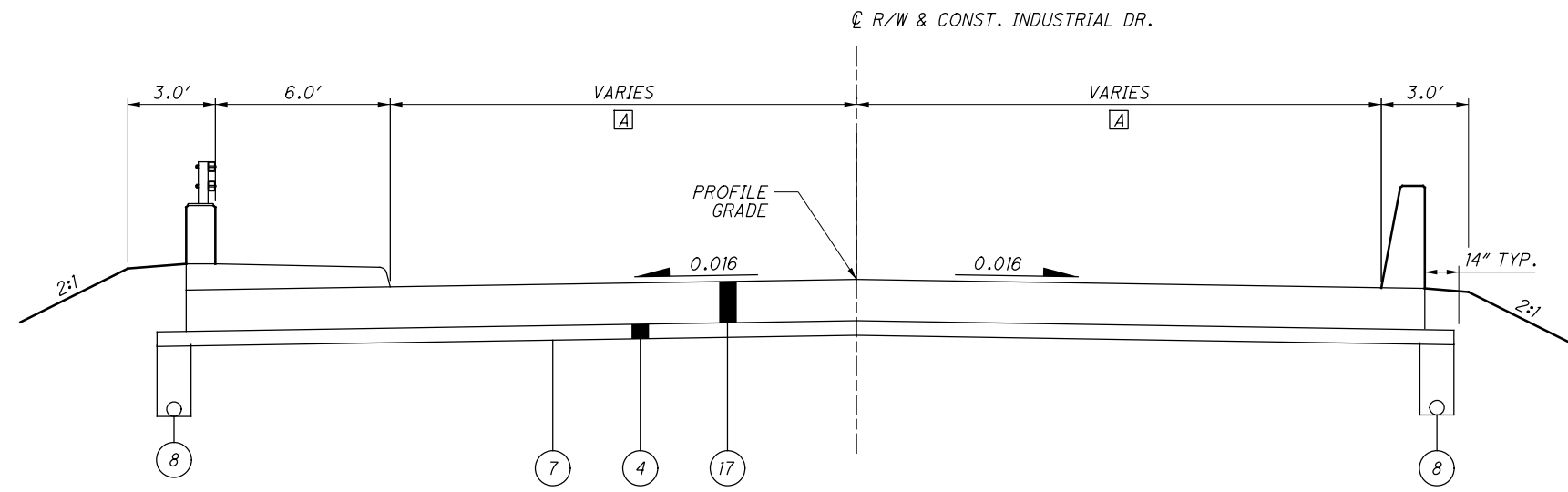


CURB SECTION
 APPLIES:
 STA. 33+64.83 TO STA. 33+73.11
 STA. 38+43.09 TO STA. 39+00.65

15" APPLIES:
 STA. 102+05.00 TO 103+78.89

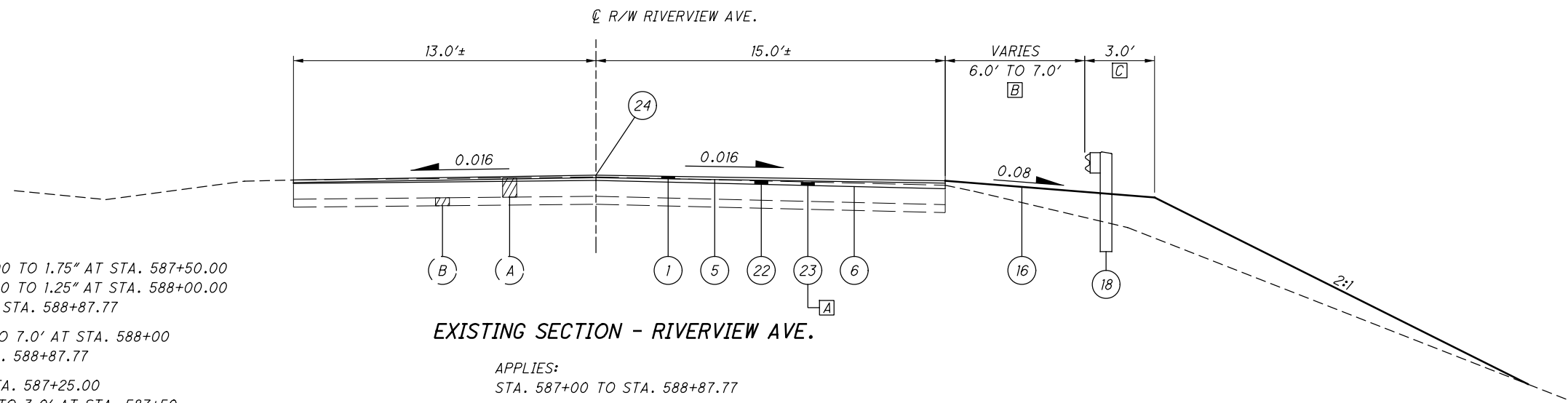
FOR LEGEND, SEE SHEET 7

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NORMAL SECTION - APPROACH SLAB

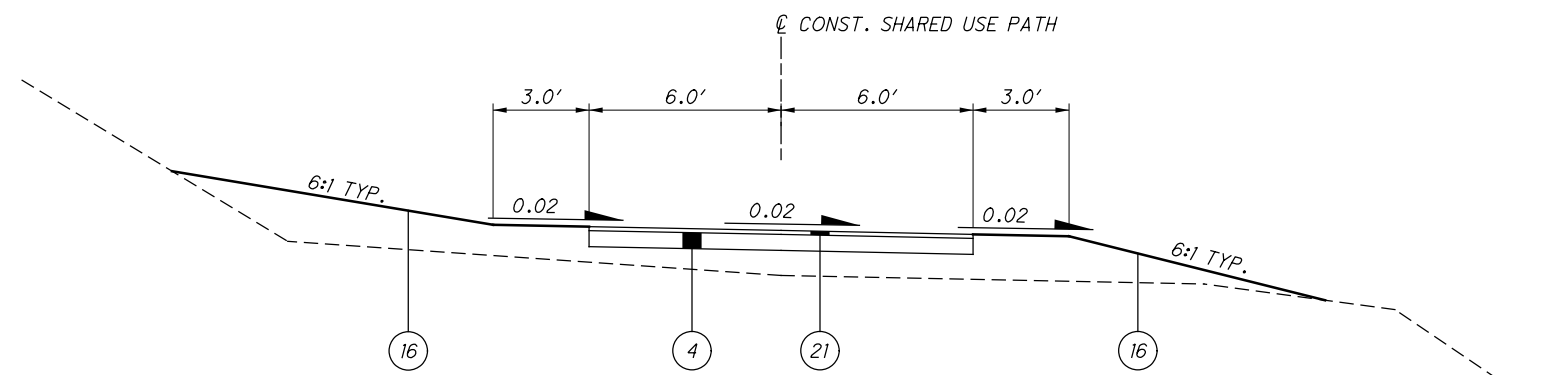
APPLIES:
STA. 39+06.32 TO STA. 39+31.32
STA. 48+76.82 TO STA. 49+08.32



EXISTING SECTION - RIVERVIEW AVE.

APPLIES:
STA. 587+00 TO STA. 588+87.77

- A** VARIES: 1.25" AT STA. 587+00.00 TO 1.75" AT STA. 587+50.00
VARIES: 1.75" AT STA. 587+50.00 TO 1.25" AT STA. 588+00.00
1.25" FROM STA. 588+00.00 TO STA. 588+87.77
- B** VARIES: 6.0' AT STA. 587+00 TO 7.0' AT STA. 588+00
7.0' FROM STA. 588+00 TO STA. 588+87.77
- C** 0' FROM STA. 587+00.00 TO STA. 587+25.00
VARIES: 0' AT STA. 587+25.00 TO 3.0' AT STA. 587+50
3.0' FROM STA. 587+50.00 TO STA. 588+87.77



NORMAL SECTION - SHARED USE PATH

FOR LEGEND, SEE SHEET 7

ROUNDING

THE ROUNDING AT SLOPE BREAKPOINTS SHOWN ON THE TYPICAL SECTIONS APPLIES TO ALL CROSS-SECTIONS EVEN THOUGH OTHERWISE SHOWN.

UTILITIES

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS AS REQUIRED BY SECTION 153.64 O.R.C.

UTILITY OWNERSHIP

LISTED BELOW ARE ALL UTILITIES LOCATED WITHIN THE PROJECT CONSTRUCTION LIMITS TOGETHER WITH THEIR RESPECTIVE OWNERS:

TIME WARNER CABLE

CITY OF NAPOLEON
255 WEST RIVERVIEW
NAPOLEON, OH 43545
(419) 592-4010

OHIO GAS
335 E. LEGGETT STREET
WASEON, OH 43567
(419) 825-5368

TOLEDO EDISON
300 MADISON AVENUE
TOLEDO, OH 43652
(419)249-5218

CENTURY LINK
812 DOLAN ST.
DEFIANCE, OH 43512

SURVEYING PARAMETERS

PRIMARY PROJECT CONTROL MONUMENTS GOVERN ALL POSITIONING ON ODOT PROJECTS. SEE SHEET ___ OF THE PLANS FOR A TABLE CONTAINING PROJECT CONTROL INFORMATION.

USE THE FOLLOWING PROJECT CONTROL, VERTICAL POSITIONING, AND HORIZONTAL POSITIONING PARAMETERS FOR ALL SURVEYING:

PROJECT CONTROL

POSITIONING METHOD: ODOT VRS
MONUMENT TYPE: CORS STATIONS

VERTICAL POSITIONING

ORTHOMETRIC HEIGHT DATUM: NAVD 88
GEOID: GEOID9

HORIZONTAL POSITIONING

REFERENCE FRAME: NAD83 (CORS96)
ELLIPSOID: GRS 80
MAP PROJECTION: LAMBERT CONFORMAL CONIC
COORDINATE SYSTEM: OHIO STATE PLANE - NORTH ZONE
COMBINED SCALE FACTOR: 1.0000675074
ORIGIN OF COORDINATE
SYSTEM: 0.00, 0.00

SURVEYING PARAMETERS (CONT.)

USE THE POSITIONING METHODS AND MONUMENT TYPE USED IN THE ORIGINAL SURVEY TO RESTORE ALL MONUMENTS RELATED TO PRIMARY PROJECT CONTROL THAT ARE DAMAGED OR DESTROYED BY CONSTRUCTION ACTIVITIES. RESTORE THE DAMAGED OR DESTROYED MONUMENTS IN ACCORDANCE WITH SUPPLEMENTAL SPECIFICATION 823.

UNITS ARE IN U.S. SURVEY FEET. USE THE FOLLOWING CONVERSION FACTOR: 1 METER = 3.280833333 U.S. SURVEY FEET.

ITEM 441, ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (446), AS PER PLAN

ITEM 441, ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (446), AS PER PLAN SHALL FOLLOW THE SPECIFICATIONS FOR 441 EXCEPT FOR SECTION 441.02 THAT THE BINDER SHALL BE PG88-22M FOR THE SURFACE COURSE AND A MAXIMUM OF 10% RAP BY DRY WEIGHT OF MIX CAN BE USED.

THE PG88-22M BINDER USED FOR THIS MIXTURE SHALL BE SAMPLED AT THE SUPPLIER TERMINAL AND TESTED BY THE DEPARTMENT PRIOR TO SHIPMENT TO THE ASPHALT PLANT. CONTACT THE DISTRICT FOR SAMPLING. IF THE SUPPLIER IS CERTIFIED FOR THE BINDER DO NOT SAMPLE AT THE TERMINAL. OBTAIN FOUR 1-QUART SAMPLES OF THE BINDER FROM THE ASPHALT PLANT STORAGE TANK AND HOLD FOR THE DISTRICT.

THE USE OF WARM MIX IS NOT PERMITTED FOR THIS MIXTURE.

ITEM 441, ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 1, (446), AS PER PLAN

ITEM 441, ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 1, (446), AS PER PLAN SHALL FOLLOW THE SPECIFICATIONS FOR 441 EXCEPT FOR SECTION 441.02 THAT THE BINDER SHALL BE PG88-22M FOR THE INTERMEDIATE COURSE AND A MAXIMUM OF 10% RAP BY DRY WEIGHT OF MIX CAN BE USED.

THE PG88-22M BINDER USED FOR THIS MIXTURE SHALL BE SAMPLED AT THE SUPPLIER TERMINAL AND TESTED BY THE DEPARTMENT PRIOR TO SHIPMENT TO THE ASPHALT PLANT. CONTACT THE DISTRICT FOR SAMPLING. IF THE SUPPLIER IS CERTIFIED FOR THE BINDER DO NOT SAMPLE AT THE TERMINAL. OBTAIN FOUR 1-QUART SAMPLES OF THE BINDER FROM THE ASPHALT PLANT STORAGE TANK AND HOLD FOR THE DISTRICT.

THE USE OF WARM MIX IS NOT PERMITTED FOR THIS MIXTURE.

UNCONTROLLED FILL AREA

THE UNCONTROLLED FILL IDENTIFIED IN THE PLANS CONSISTS OF UNCONTROLLED AND OTHER DELETERIOUS MATERIALS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING THE EXISTING MATERIAL THAT IS WITHIN THE PROJECT CONSTRUCTION LIMITS TO A DEPTH OF 7 FEET BELOW THE EXISTING GRADE AND REPLACING WITH GRANULAR EMBANKMENT.

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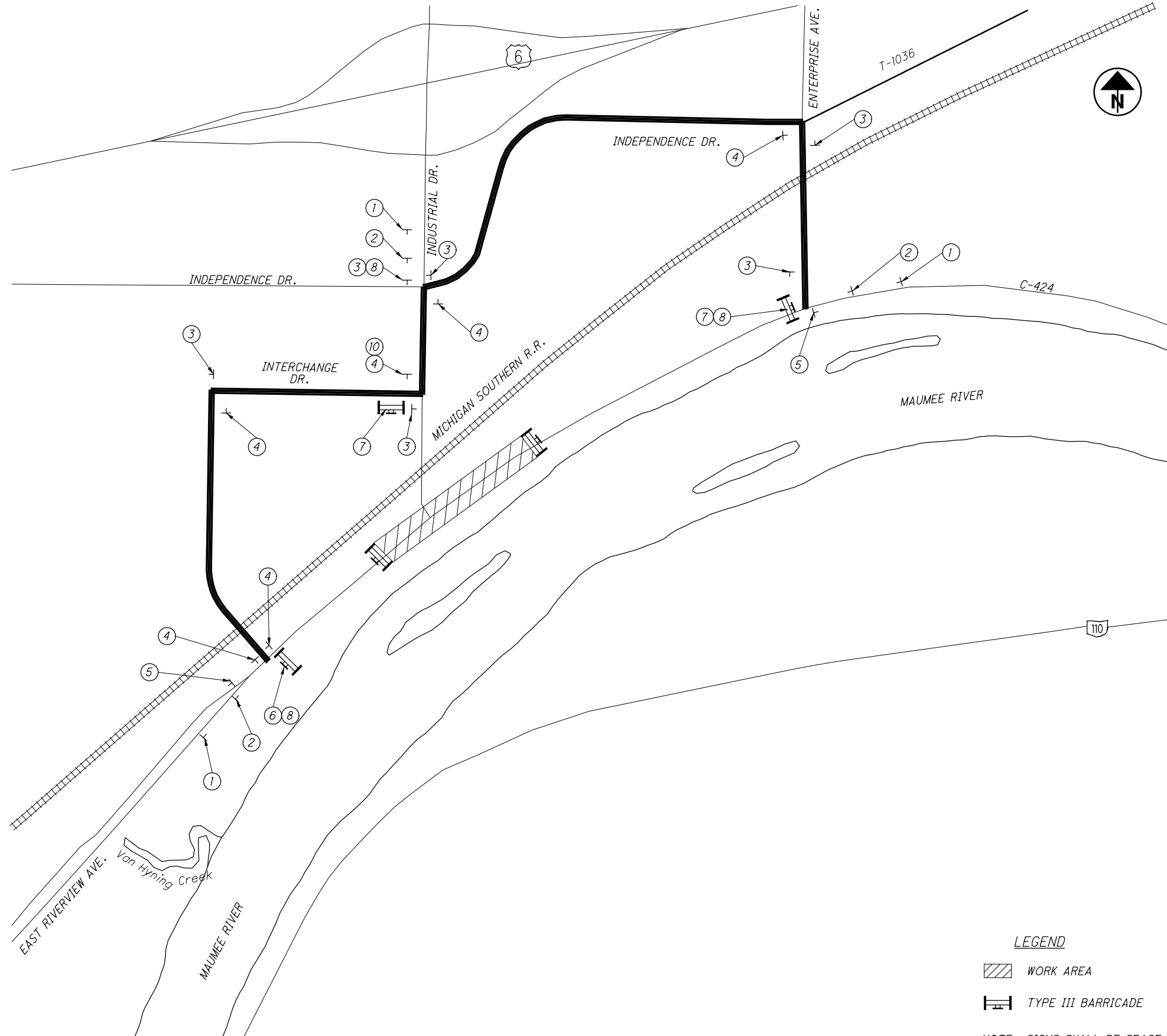
CALCULATED
CHECKED

GENERAL NOTES

HEN-NEW MAUMEE RIVER BRIDGE

12
180

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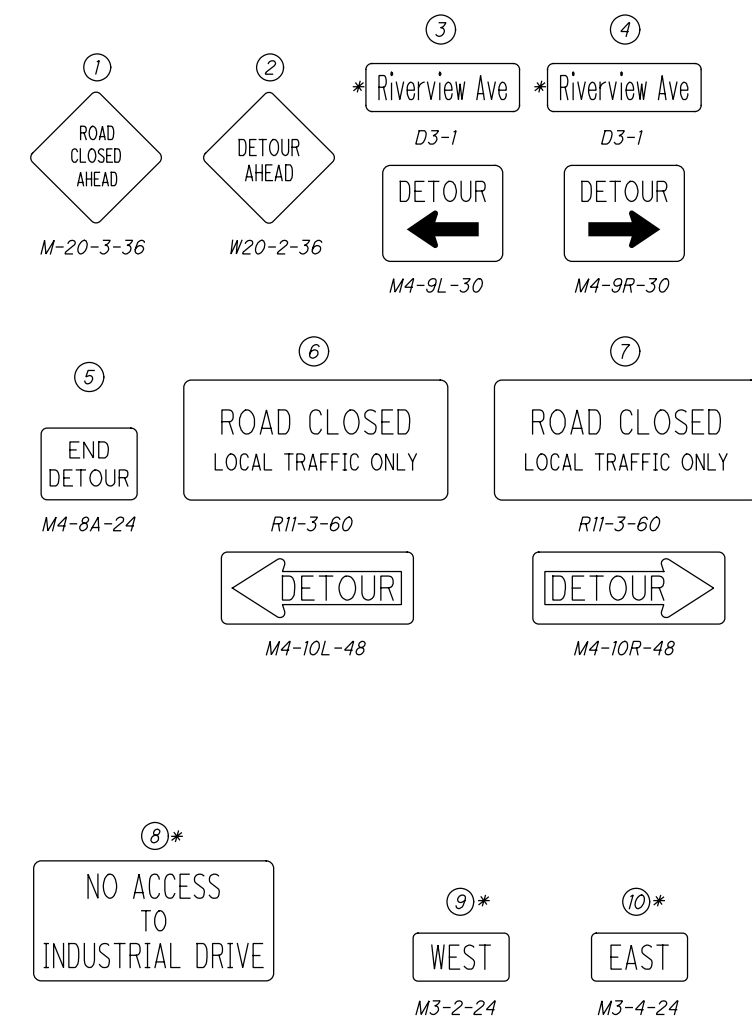


LEGEND

WORK AREA

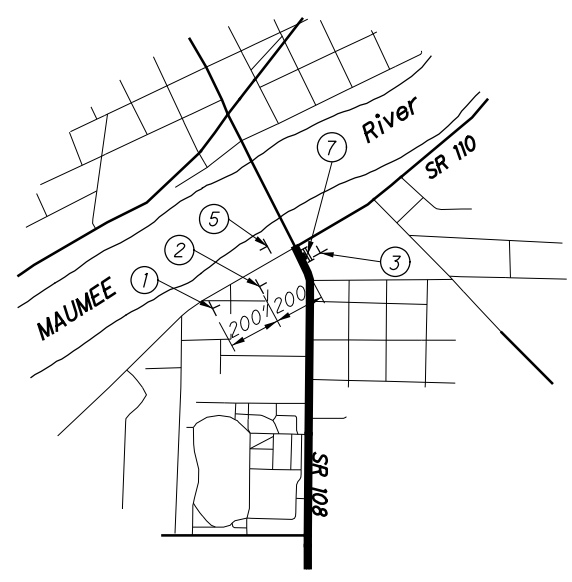
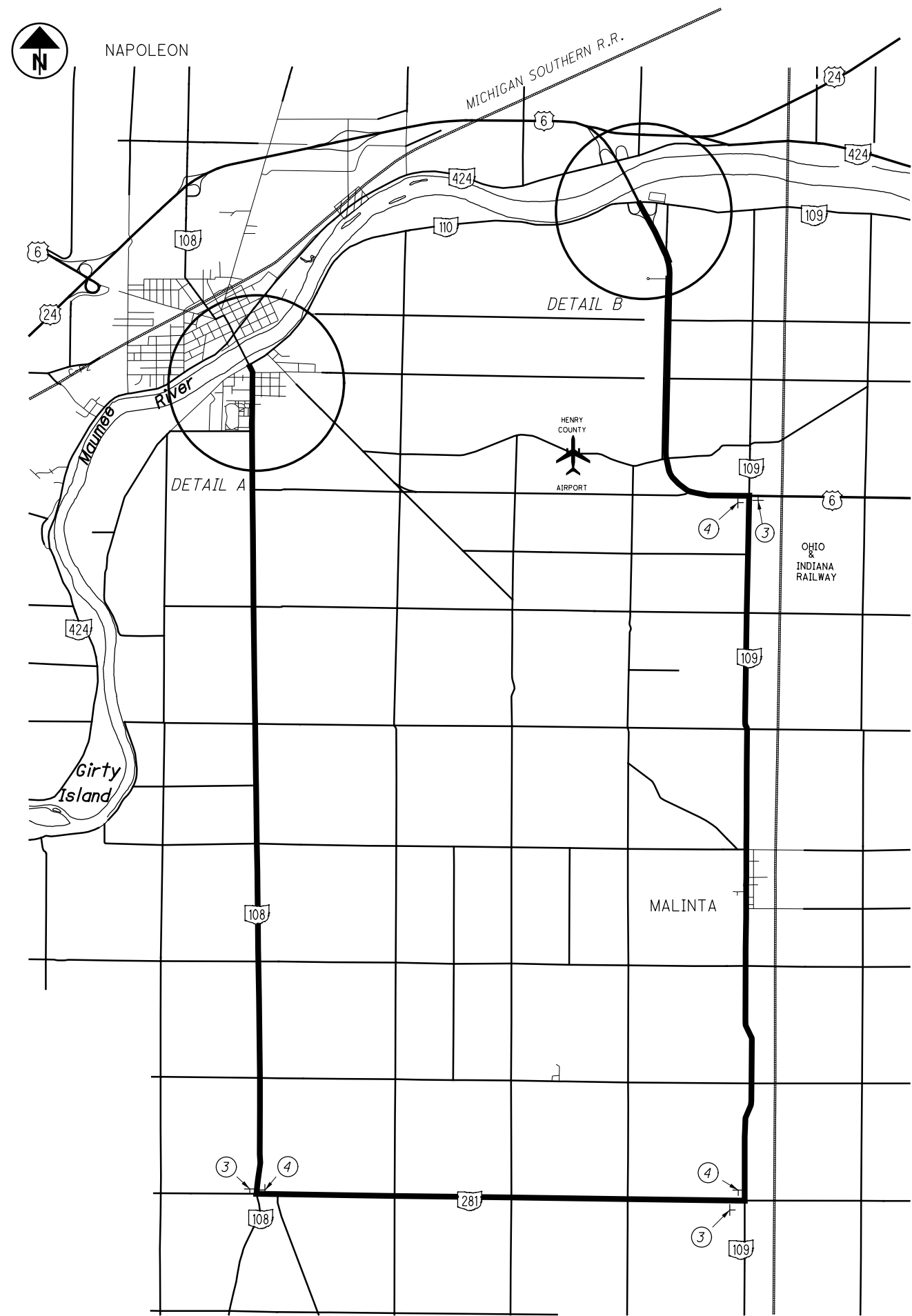
TYPE III BARRICADE

NOTE: SIGNS SHALL BE SPACE AT 200' FOR INDUSTRIAL DR. AND 350' FOR RIVERVIEW AVE.
 CONTRACTOR SHALL CLOSE RIVERVIEW AVE AND INDUSTRIAL DR AT THE PROJECT WORK LIMITS WHILE MAINTAINING ACCESS TO ALL PROPERTIES.

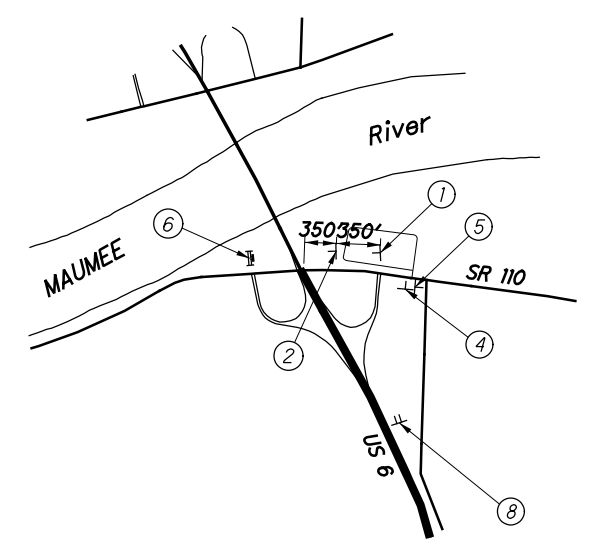


* SIGN SHALL BE BLACK ON ORANGE

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DETAIL A
S.R. 108 AND S.R. 110



DETAIL B
U.S. 6 AND S.R. 110

LEGEND

- WORK AREA
- TYPE III BARRICADE

① ROAD CLOSED AHEAD M-20-3-36	② DETOUR AHEAD W20-2-36	③ DETOUR M4-8-30 110 MI-5-30-3 ← M6-1-30	④ DETOUR M4-8-30 110 MI-5-30-3 → M6-1-30
⑤ END DETOUR M4-8A-24	⑥ ROAD CLOSED 2 MILES AHEAD LOCAL TRAFFIC ONLY R11-3a ← DETOUR M4-10L-48	⑦ ROAD CLOSED 2 MILES AHEAD LOCAL TRAFFIC ONLY R11-3a DETOUR → M4-10R-48	⑧ DETOUR M4-9R-30 110 NAPOLEON GRAND RAPIDS EXIT MILES

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ITEM 614, MAINTAINING TRAFFIC

A MINIMUM OF ONE LANE OF TRAFFIC IN EACH DIRECTION SHALL BE MAINTAINED AT ALL TIMES, EXCEPT FOR A PERIOD NOT TO EXCEED CONSECUTIVE CALENDAR DAYS, WHEN THROUGH TRAFFIC MAY BE DETOURED AS SHOWN ON SHEET OR THE USE OF ONE WAY SIGNAL OPERATION. DISINCENTIVE SHALL BE ASSESSED IN THE AMOUNT OF \$ FOR EACH CALENDAR DAY THE ROADWAY REMAINS CLOSED TO TRAFFIC BEYOND THE SPECIFIED LIMIT. RIVERVIEW DRIVE AND SR 110 SHALL NOT BE CLOSED AT THE SAME TIME.

LENGTH AND DURATION OF LANE CLOSURES AND RESTRICTIONS SHALL BE AT THE APPROVAL OF THE ENGINEER. IT IS THE INTENT TO MINIMIZE THE IMPACT TO THE TRAVELING PUBLIC. LANE CLOSURES OR RESTRICTIONS OVER SEGMENTS OF THE PROJECT IN WHICH NO WORK IS ANTICIPATED WITHIN A REASONABLE TIME FRAME, AS DETERMINED BY THE ENGINEER, SHALL NOT BE PERMITTED. THE LEVEL OF UTILIZATION OF MAINTENANCE OF TRAFFIC DEVICES SHALL BE COMMENSURATE WITH THE WORK IN PROGRESS.

NOTICE OF CLOSURE SIGNS, AS DETAILED IN THESE PLANS, SHALL BE ERECTED BY THE CONTRACTOR AT LEAST ONE WEEK IN ADVANCE OF THE SCHEDULED ROAD CLOSURE. THE SIGNS SHALL BE ERECTED ON THE RIGHT-HAND SIDE OF THE ROAD FACING TRAFFIC. THEY SHALL BE PLACED SO AS NOT TO INTERFERE WITH THE VISIBILITY OF ANY OTHER TRAFFIC CONTROL SIGNS. ON ROADWAYS, THEY SHOULD BE ERECTED AT THE POINT OF CLOSURE. THE SIGNS MAY BE ERECTED ANYWHERE ON RAMPS AS LONG AS THEY ARE VISIBLE TO THE MOTORISTS USING THE RAMP.

THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN INCLUDED IN THE GENERAL SUMMARY FOR USE AS DETERMINED BY THE ENGINEER FOR THE MAINTENANCE OF TRAFFIC.

ITEM 410, TRAFFIC COMPACTED SURFACE, TYPE A OR B	----- CU. YD.
ITEM 410, TRAFFIC COMPACTED SURFACE, TYPE C	----- CU. YD.
ITEM 614, ASPHALT CONCRETE FOR MAINTAINING TRAFFIC	----- CU. YD.
ITEM 616, WATER	----- M. GAL.

ALL WORK AND TRAFFIC CONTROL DEVICES SHALL BE IN ACCORDANCE WITH CMS 614 AND OTHER APPLICABLE PORTIONS OF THE SPECIFICATIONS, AS WELL AS THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. PAYMENT FOR ALL LABOR, EQUIPMENT AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR ITEM 614, MAINTAINING TRAFFIC, UNLESS SEPARATELY ITEMIZED IN THE PLAN.

OVERNIGHT TRENCH CLOSING

THE BASE WIDENING SHALL BE COMPLETED TO A DEPTH OF NO MORE THAN INCHES BELOW THE EXISTING PAVEMENT BY THE END OF EACH WORK DAY. NO TRENCH SHALL BE LEFT OPEN OVERNIGHT EXCEPT FOR A SHORT LENGTH (25 FEET OR LESS) OF A WORK SECTION AT THE END OF THE TRENCH. IN CASE WORK MUST BE SUSPENDED BECAUSE OF INCLEMENT WEATHER OR OTHER REASONS, THE TRENCH FOR THE UN-COMPLETED BASE WIDENING SHALL BE BACKFILLED AT THE DIRECTION OF THE ENGINEER.

DUST CONTROL

THE CONTRACTOR SHALL FURNISH AND APPLY WATER FOR DUST CONTROL AS DIRECTED BY THE ENGINEER. THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN INCLUDED FOR DUST CONTROL PURPOSES:

ITEM 616, WATER ----- M. GAL

ITEM 614, REPLACEMENT SIGN

FLATSHEET SIGNS FURNISHED BY THE CONTRACTOR IN ACCORDANCE WITH THE REQUIREMENTS OF THE PLANS, SPECIFICATIONS AND PROPOSAL WHICH BECOME DAMAGED BY TRAFFIC FOR REASONS BEYOND THE CONTROL OF THE CONTRACTOR SHALL BE REPLACED IN KIND WHEN ORDERED BY THE ENGINEER. REPLACEMENT SIGNS SHALL BE NEW. OTHER MATERIALS MAY BE IN USED, BUT GOOD, CONDITION SUBJECT TO APPROVAL BY THE ENGINEER.

PAYMENT FOR THE NEW SIGNS SHALL BE MADE AT THE CONTRACT PRICE PER EACH FOR ITEM 614, REPLACEMENT SIGN, AND SHALL INCLUDE THE COST OF REMOVING AND DISPOSING OF DAMAGED SIGNS, HARDWARE AND SUPPORTS, AND PROVIDING THE NECESSARY REPLACEMENT HARDWARE, SUPPORTS, ETC.

AN ESTIMATED QUANTITY OF EACH HAS BEEN PROVIDED IN THE GENERAL SUMMARY.

ITEM 614, REPLACEMENT DRUM

DRUMS FURNISHED BY THE CONTRACTOR IN ACCORDANCE WITH THE REQUIREMENTS OF THE PLANS, SPECIFICATIONS AND PROPOSAL WHICH BECOME DAMAGED BY TRAFFIC FOR REASONS BEYOND THE CONTROL OF THE CONTRACTOR SHALL BE REPLACED IN KIND WHEN ORDERED BY THE ENGINEER. REPLACEMENT DRUMS SHALL BE NEW.

PAYMENT FOR THE NEW DRUMS SHALL BE MADE AT THE CONTRACT PRICE PER EACH FOR ITEM 614, REPLACEMENT DRUM, AND SHALL INCLUDE THE COST OF REMOVING AND DISPOSING OF THE DAMAGED DRUM, AND PROVIDING AND MAINTAINING THE REPLACEMENT DRUM IN ACCORDANCE WITH THE CONTRACT REQUIREMENTS FOR THE ORIGINAL DRUM.

AN ESTIMATED QUANTITY OF EACH HAS BEEN PROVIDED IN THE GENERAL SUMMARY.

ITEM 614, BARRIER REFLECTORS AND/OR OBJECT MARKERS

BARRIER REFLECTORS AND/OR OBJECT MARKERS SHALL BE INSTALLED ON ALL PORTABLE BARRIER (PB) USED FOR TRAFFIC CONTROL. BARRIER REFLECTORS, OBJECT MARKERS AND THEIR INSTALLATION SHALL CONFORM TO CMS 626, EXCEPT THAT THE SPACING SHALL BE 50 FEET. AN ESTIMATED QUANTITY OF EACH OF ITEM 614 BARRIER REFLECTOR, TYPE B AND EACH OF ITEM 614 OBJECT MARKER, -WAY HAVE BEEN PROVIDED AND CARRIED TO THE GENERAL SUMMARY.

FULLY-ACTUATED OPERATION OF WORK ZONE TRAFFIC SIGNAL

THE WORK ZONE SIGNAL CONTROL REQUIRED FOR THIS PROJECT AND SHOWN ON SHEETS AND SCDS MT-96.11, 96.20 AND 96.26 SHALL BE FULLY TRAFFIC-ACTUATED AND OPERATE IN A MANNER SIMILAR TO THAT DESCRIBED IN SECTION 733.02 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS.

THE INITIAL CONTROLLER TIMING SHALL BE AS FOLLOWS:

RIVER AVE PHASE *				
	1 (All Red) Dummy Phase	2 Mainline Direction	3 (All Red) Dummy Phase	4 Mainline Direction
MIN. GREEN		10		10
EXTENSION		4		4
MAX. GREEN		30		30
YELLOW		3.5		3.5
ALL RED	30		30	
RECALL	MAX.	MIN.	MAX.	MIN.

SR 110 PHASE *				
	1 (All Red) Dummy Phase	2 Mainline Direction	3 (All Red) Dummy Phase	4 Mainline Direction
MIN. GREEN		10		10
EXTENSION		4		4
MAX. GREEN		30		30
YELLOW		3.5		3.5
ALL RED	30		30	
RECALL	MAX.	MIN.	MAX.	MIN.

*PHASES AS SHOWN ON SCD MT-96.20 FOR ACTUATED CONTROL. ADD MORE PHASES AS NEEDED TO ACCOMMODATE SIDE STREETS, DRIVEWAYS, ETC.
+/- PROVIDE TIMING FOR THE SIGNAL LOCATION UNDER CONSIDERATION.

1 INDICATE DIRECTION OF GREEN.

THE CONTRACTOR SHALL ALSO DESIGN, FURNISH, INSTALL AND MAINTAIN A TRAFFIC DETECTOR ON EACH TRAFFIC APPROACH WHICH WILL RELIABLY DETECT ALL LEGAL TRAFFIC APPROACHING (BUT NOT LEAVING) THE SIGNAL AS IT PASSES OR WAITS IN THE DESIGNATED DETECTOR ZONE SHOWN IN THE PLANS. DETECTOR DESIGNS WHICH DO NOT PROVIDE RELIABLE DETECTION, FREE FROM FALSE CALLS, SHALL BE IMMEDIATELY REPLACED BY THE CONTRACTOR.

SEQUENCE OF CONSTRUCTION

GENERAL

THE CONSTRUCTION OF THE NEW STRUCTURE MAY BE PERFORMED AT ANY TIME DURING THE CONSTRUCTION SEQUENCE SO LONG AS ALL REQUIREMENTS OF THESE PLANS AND THE PROJECT SCHEDULE ARE MET.

STAGE 1

SETUP THE TEMPORARY TRAFFIC CONTROL AS DETAILED ON SHEETS XX-XX AND PERFORM THE FOLLOWING WORK. TRAFFIC SHALL BE MAINTAINED ON EXISTING PAVEMENT OF SR 110 AND RIVERVIEW AVENUE.

-CONSTRUCT THE NEW INDUSTRIAL DRIVE ALIGNMENT OUTSIDE OF THE EXISTING PAVEMENT TO THE PROPOSED SUBGRADE.

STAGE 2

UPON THE COMPLETION OF STAGE 1 INSTALL TEMPORARY SIGNALS AT RIVERVIEW AVENUE AND SR 110. UPON THE INSTALLATION OF THE TEMPORARY SIGNALS SETUP THE TEMPORARY TRAFFIC CONTROL AS DETAILED ON SHEETS XX-XX AND PERFORM THE FOLLOWING WORK.

-CONSTRUCT EB SIDE OF RIVERVIEW AVENUE TO THE PROPOSED SUBGRADE.

-CONSTRUCT WB SIDE OF SR 110 TO THE PROPOSED SUBGRADE.

STAGE 3

UPON COMPLETION OF STAGE 2 CLOSE INDUSTRIAL DRIVE, RIVERVIEW AVE., AND SR 110 TO THROUGH TRAFFIC AND DETOUR TRAFFIC AS SHOWN ON SHEETS XX-XX AND PERFORM THE FOLLOWING WORK. AT THE COMPLETION OF PHASE 3 THE CONTRACTOR SHALL OPEN THE BRIDGE TO TRAFFIC.

-CONSTRUCT INDUSTRIAL DRIVE NORTH OF RIVERVIEW AVENUE AVE AND SR 110 TO THE FINAL SURFACE COURSE.

-CONSTRUCT THE RIVERVIEW AVENUE AND SR 110 ROUNDABOUTS TO THE FINAL SURFACE COURSE.

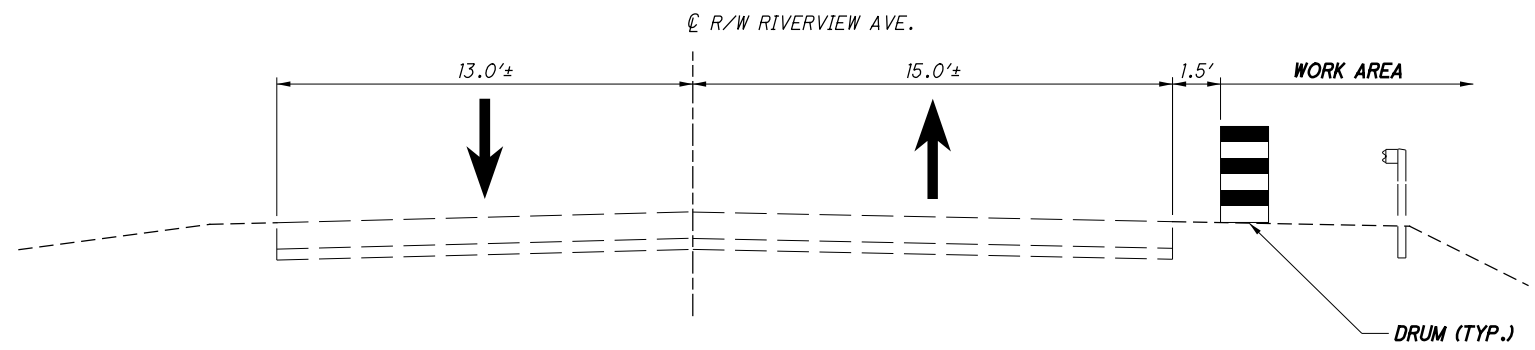
-INSTALL PAVEMENT MARKINGS AND SIGNS. MARKINGS AND SIGNS FOR THE ROUNDABOUT SHALL BE INSTALLED PRIOR TO OPENING THE ROUNDABOUT TO TRAFFIC.

CALCULATED
GJM
CHECKED
ALT

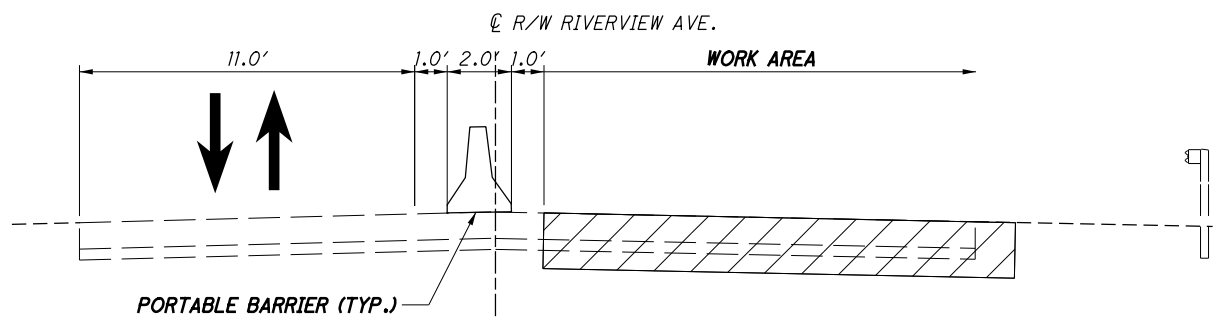
MAINTENANCE OF TRAFFIC GENERAL NOTES

HEN-NEW MAUMEE RIVER BRIDGE

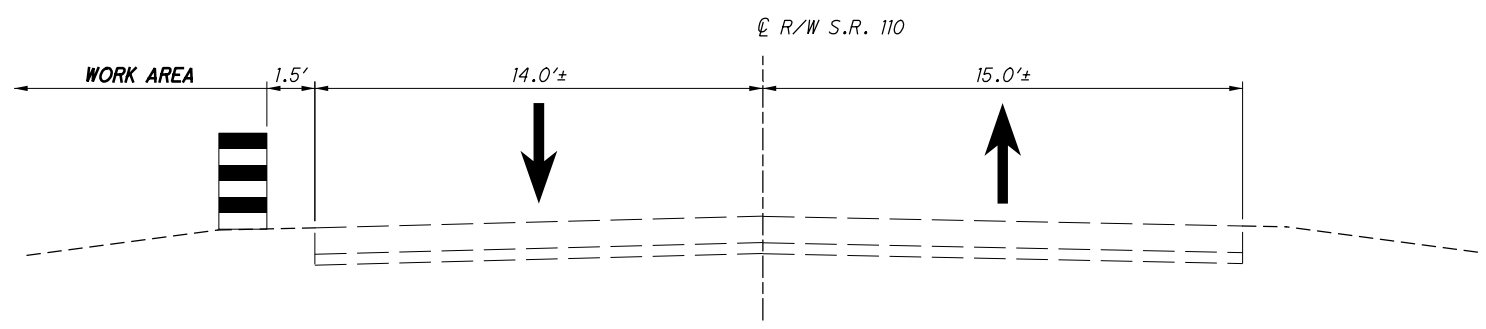
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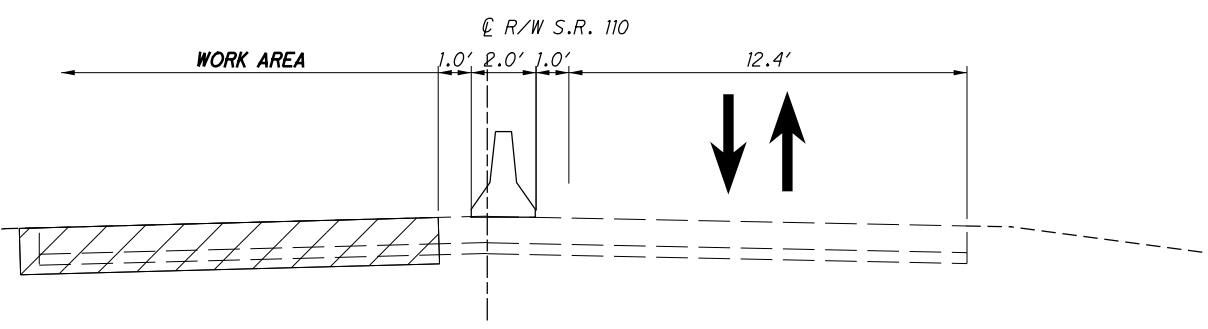
TYPICAL SECTION - STAGE 1
RIVERVIEW AVE.



TYPICAL SECTION - STAGE 2
RIVERVIEW AVE.



TYPICAL SECTION - STAGE 1
S.R. 110






TYPICAL SECTION - STAGE 2
S.R. 110

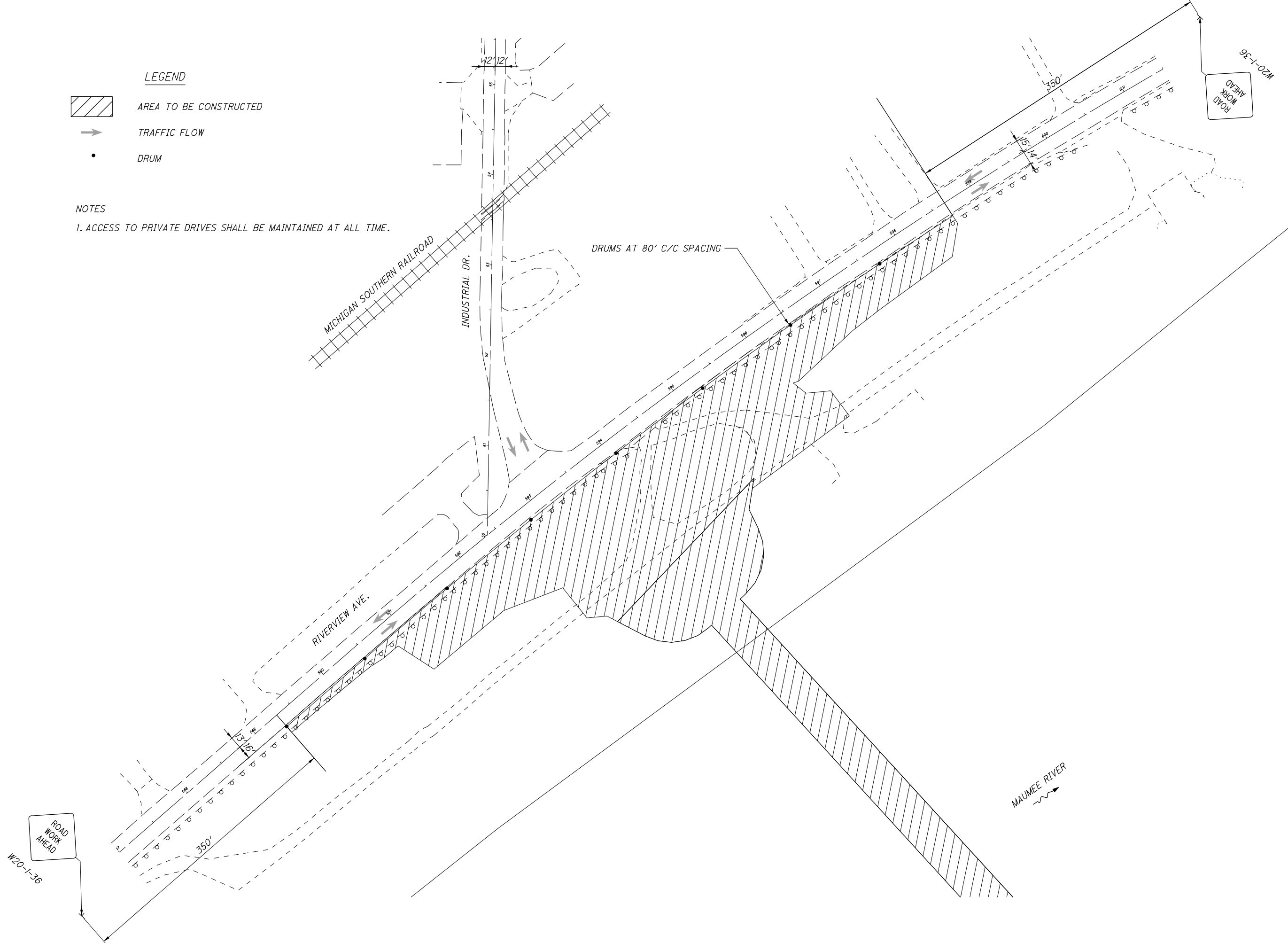
MAINTENANCE OF TRAFFIC
TYPICAL SECTIONS

HEN-NEW MAUMEE
RIVER BRIDGE

\\msgfilesrv\MSGData\Projects\Projects F-J\H2530002\22984\mot\sheet\22984MP001.dgn 5/22/2015 4:03:49 PM SValentin

- LEGEND**
-  AREA TO BE CONSTRUCTED
 -  TRAFFIC FLOW
 -  DRUM

NOTES
 1. ACCESS TO PRIVATE DRIVES SHALL BE MAINTAINED AT ALL TIME.



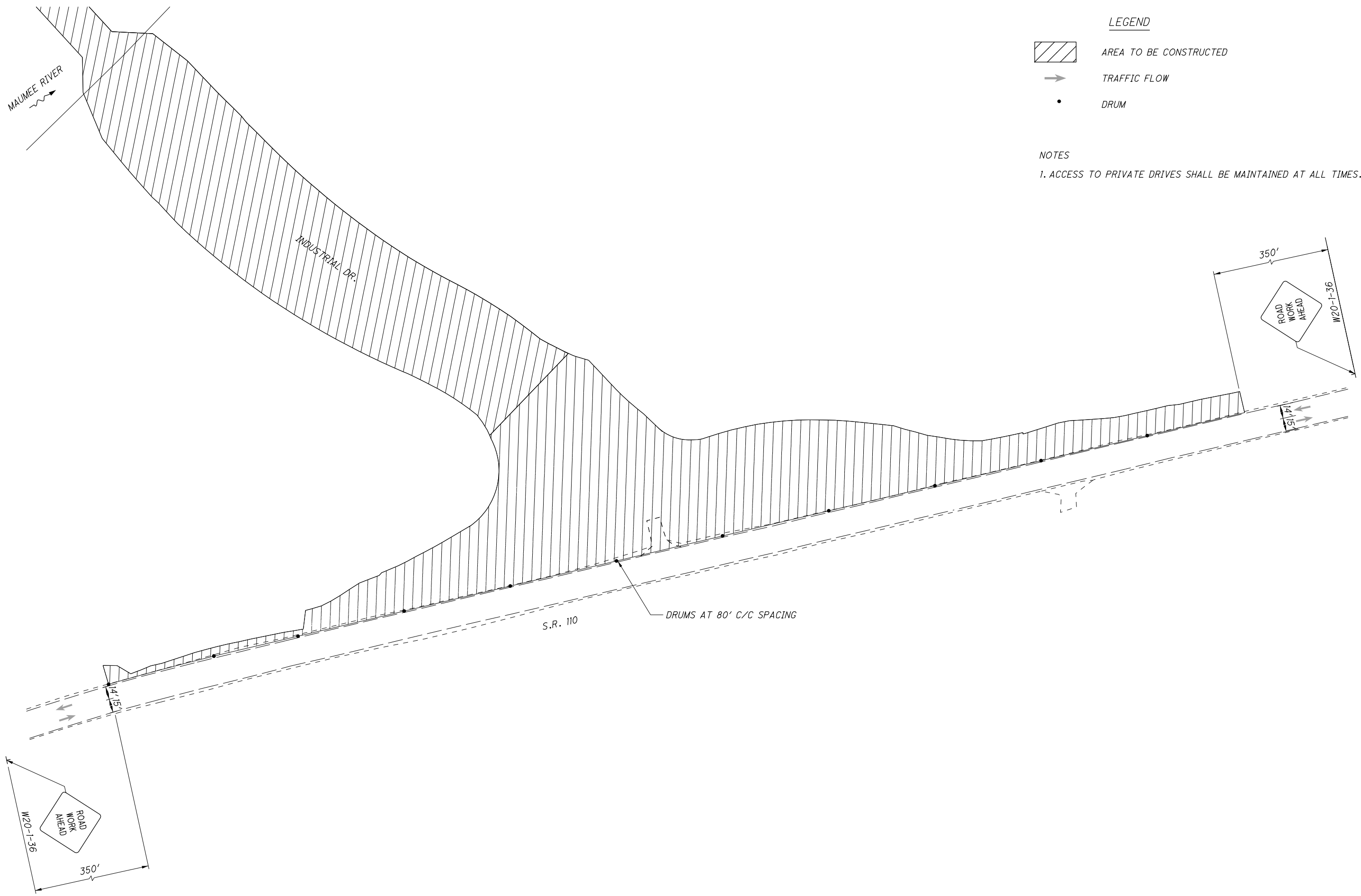
CALCULATED
 CHECKED

0 50 100
 25
 HORIZONTAL
 SCALE IN FEET




**MAINTENANCE OF TRAFFIC
 STAGE 1**

**HEN-NEW MAUMEE
 RIVER BRIDGE**

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LEGEND

-  AREA TO BE CONSTRUCTED
-  TRAFFIC FLOW
-  DRUM

NOTES

1. ACCESS TO PRIVATE DRIVES SHALL BE MAINTAINED AT ALL TIMES.




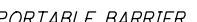


CALCULATED

CHECKED

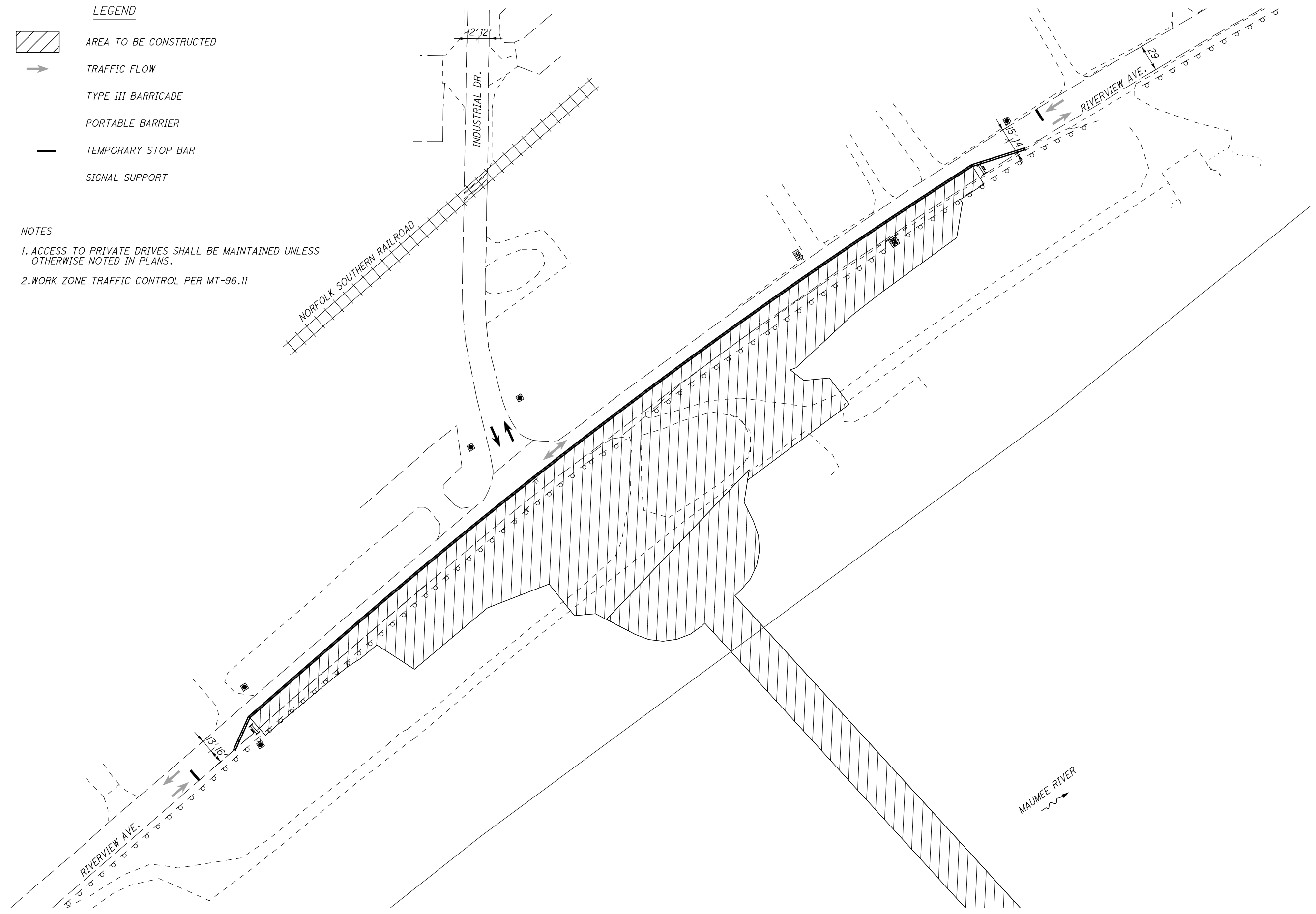
0 50 100
HORIZONTAL SCALE IN FEET

MAINTENANCE OF TRAFFIC
STAGE 1

HEN-NEW MAUMEE
RIVER BRIDGE

- LEGEND**
-  AREA TO BE CONSTRUCTED
 -  TRAFFIC FLOW
 -  TYPE III BARRICADE
 -  PORTABLE BARRIER
 -  TEMPORARY STOP BAR
 -  SIGNAL SUPPORT

- NOTES**
1. ACCESS TO PRIVATE DRIVES SHALL BE MAINTAINED UNLESS OTHERWISE NOTED IN PLANS.
 2. WORK ZONE TRAFFIC CONTROL PER MT-96.11



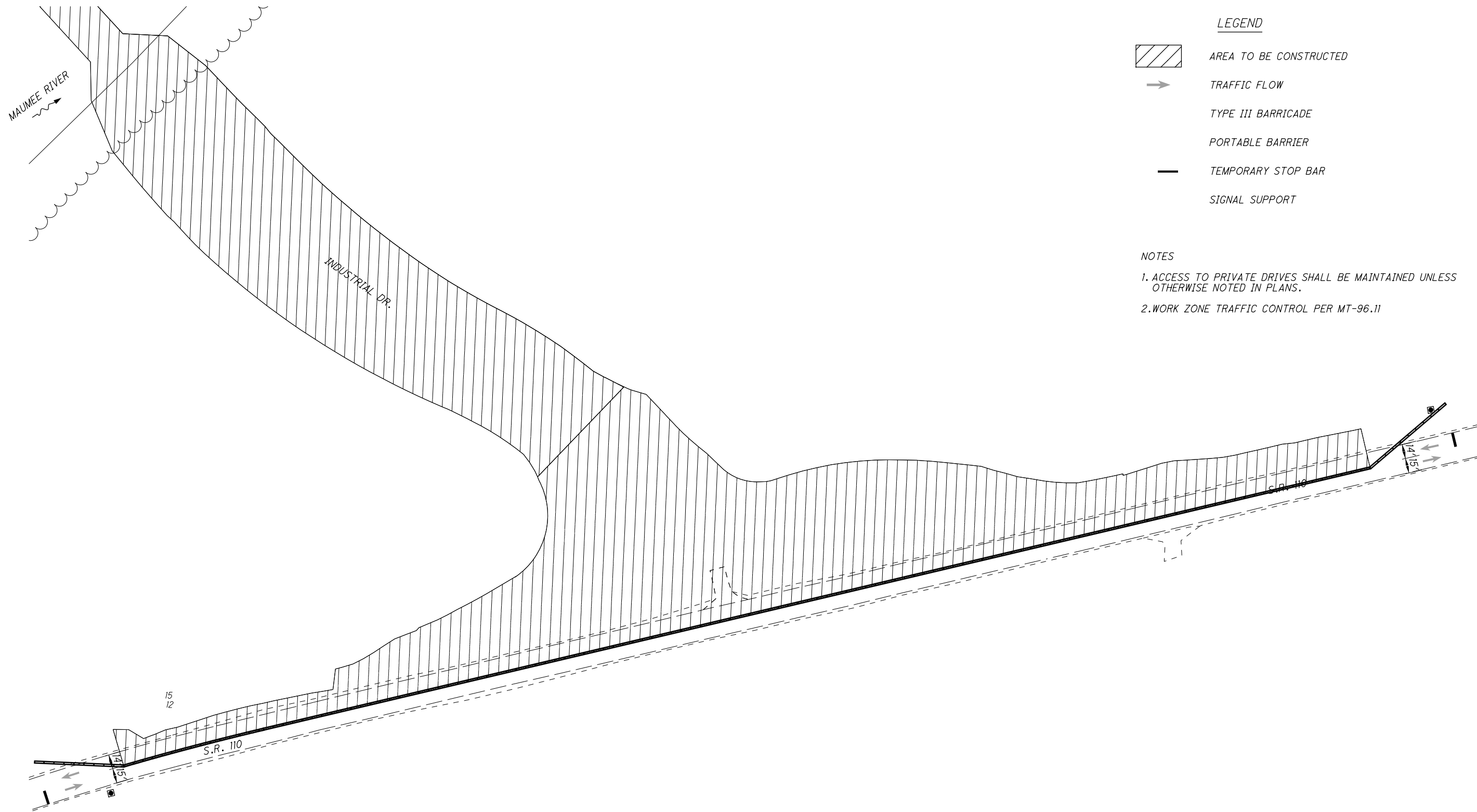
CALCULATED
CHECKED

**MAINTENANCE OF TRAFFIC
STAGE 2**







**HEN-NEW MAUMEE
RIVER BRIDGE**

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LEGEND

-  AREA TO BE CONSTRUCTED
-  TRAFFIC FLOW
-  TYPE III BARRICADE
-  PORTABLE BARRIER
-  TEMPORARY STOP BAR
-  SIGNAL SUPPORT

NOTES

1. ACCESS TO PRIVATE DRIVES SHALL BE MAINTAINED UNLESS OTHERWISE NOTED IN PLANS.
2. WORK ZONE TRAFFIC CONTROL PER MT-96.11

CALCULATED

CHECKED

0 50 100

HORIZONTAL SCALE IN FEET

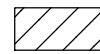

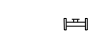


MAINTENANCE OF TRAFFIC
STAGE 2

HEN-NEW MAUMEE
RIVER BRIDGE

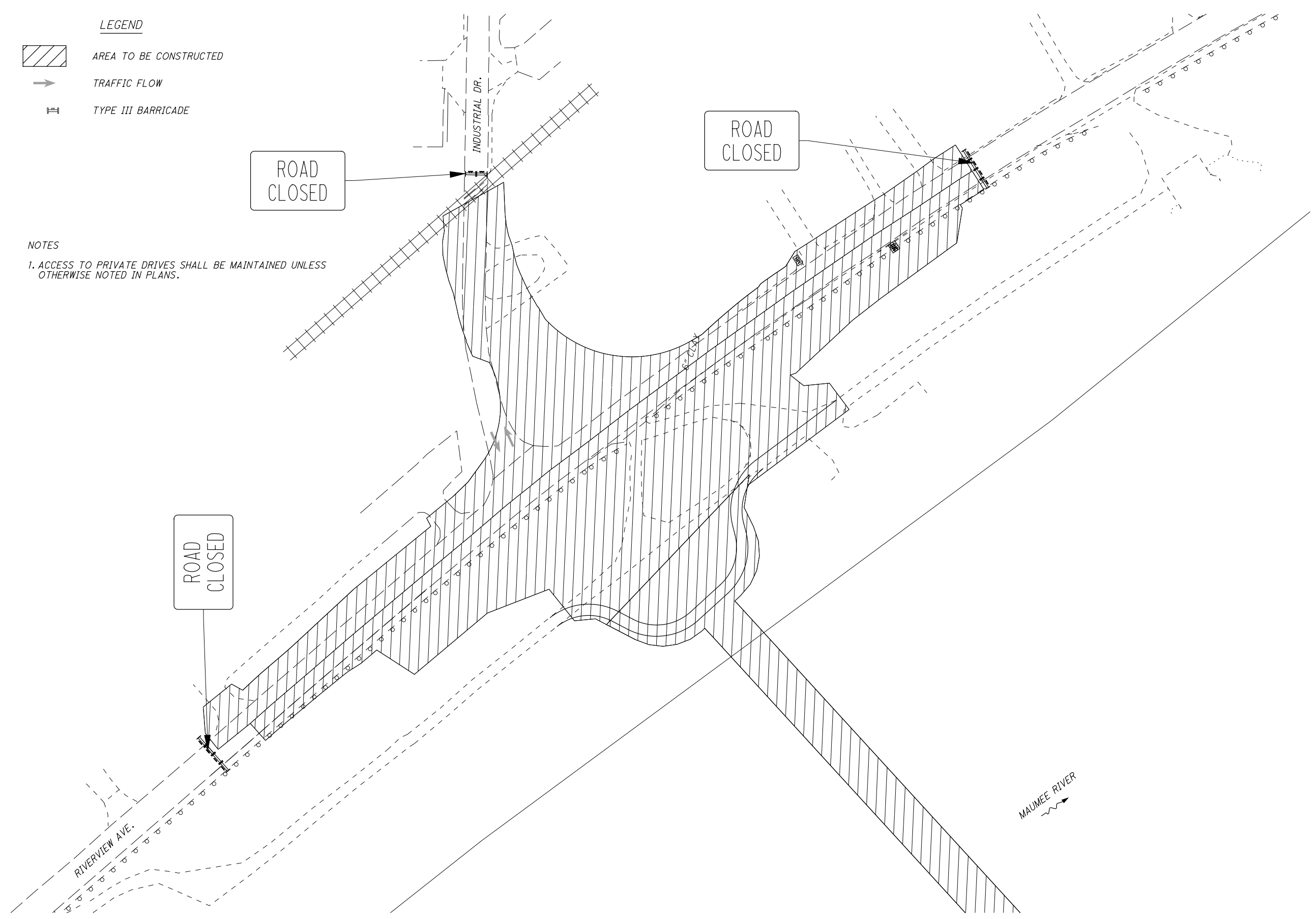
\\msgfilesrv\MSGData\Projects\Projects F-J\H2530002\22984\mot\sheet\22984MP005.dgn 5/22/2015 4:03:51 PM SValentin

LEGEND

-  AREA TO BE CONSTRUCTED
-  TRAFFIC FLOW
-  TYPE III BARRICADE

NOTES

1. ACCESS TO PRIVATE DRIVES SHALL BE MAINTAINED UNLESS OTHERWISE NOTED IN PLANS.



CALCULATED

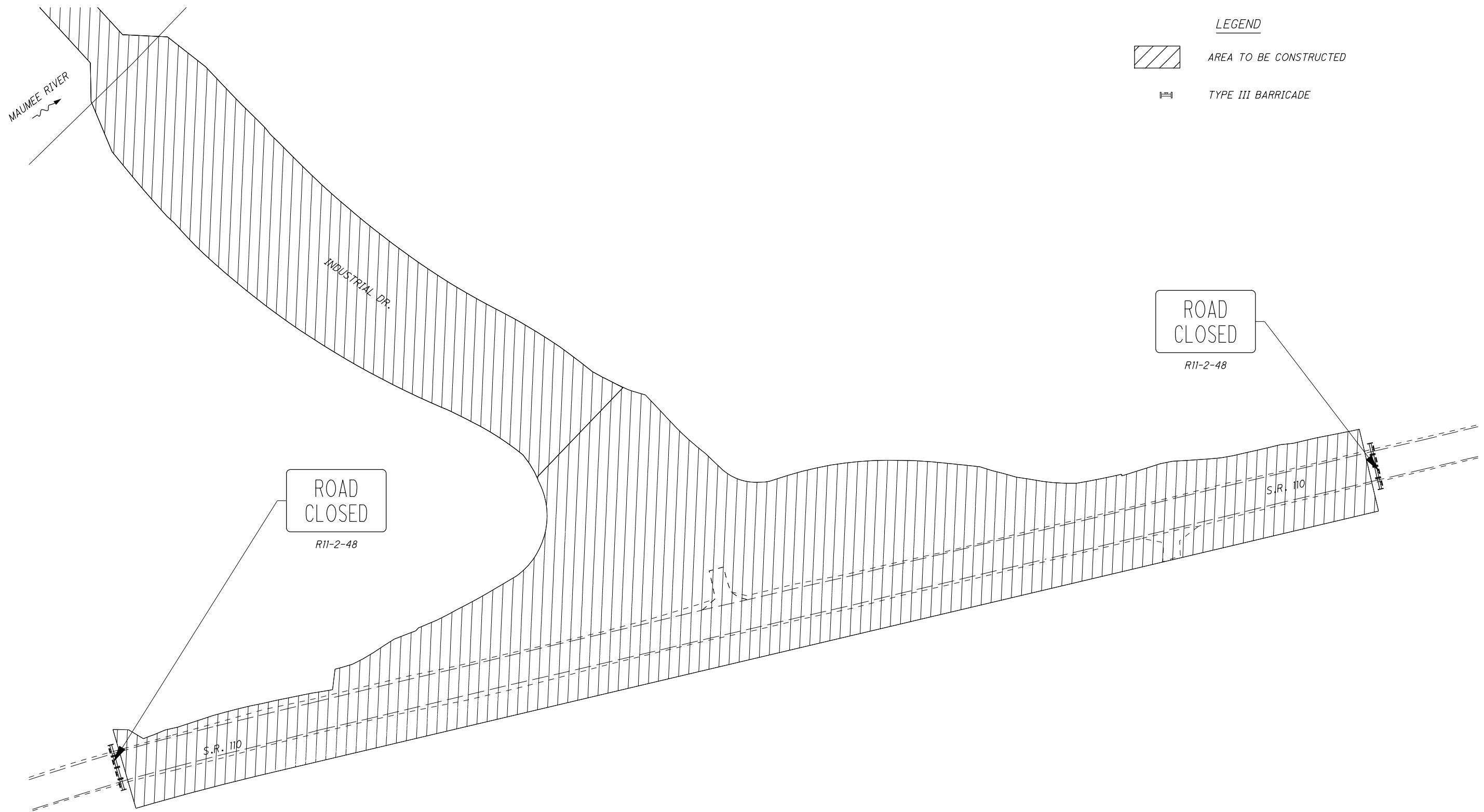
CHECKED

0 25 50 100
HORIZONTAL SCALE IN FEET



**MAINTENANCE OF TRAFFIC
STAGE 3**

**HEN-NEW MAUMEE
RIVER BRIDGE**

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LEGEND

-  AREA TO BE CONSTRUCTED
-  TYPE III BARRICADE

CALCULATED
CHECKED

0 50 100
HORIZONTAL
SCALE IN FEET

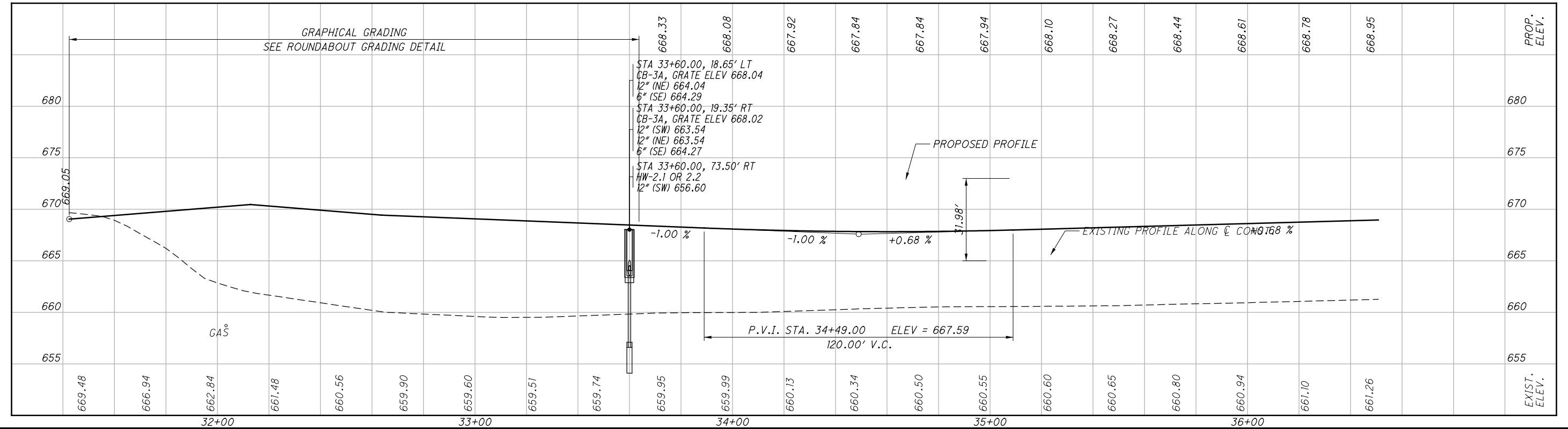
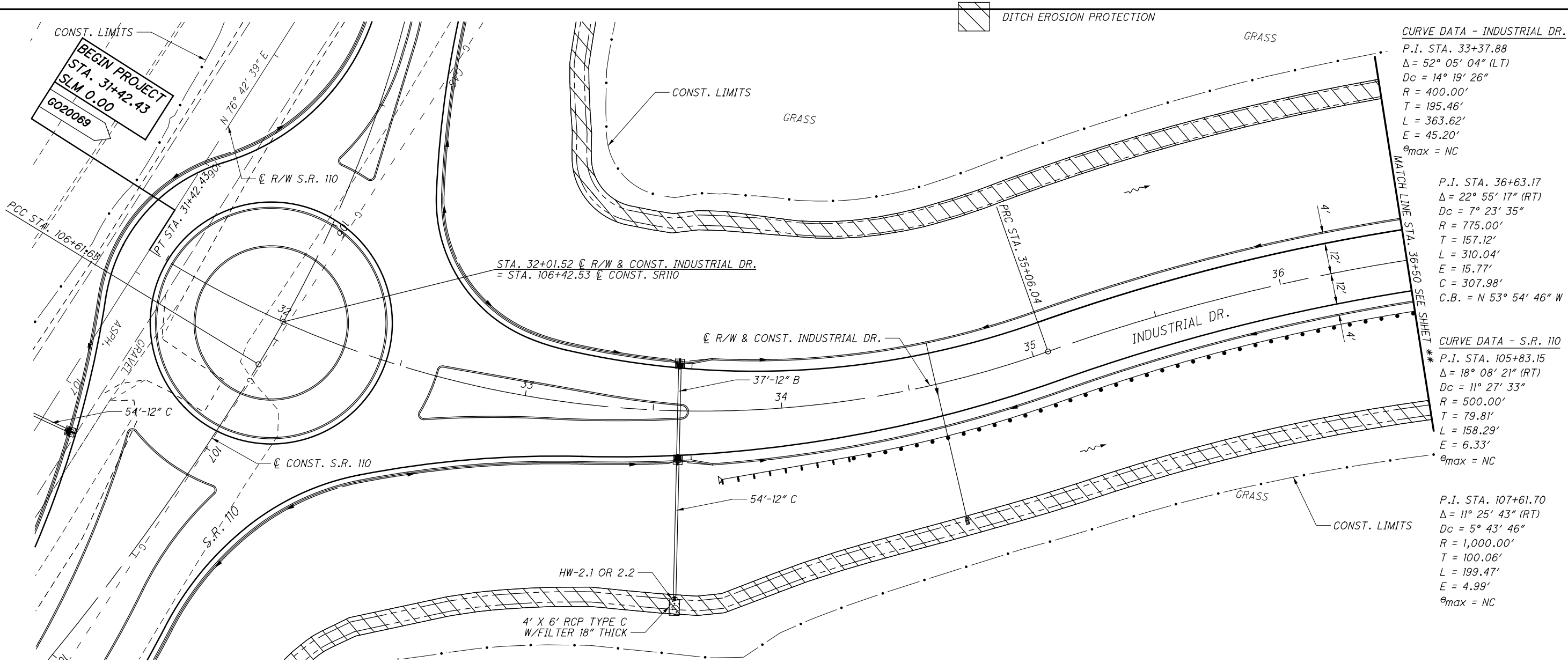
MAINTENANCE OF TRAFFIC
STAGE 3

HEN-NEW MAUMEE
RIVER BRIDGE

NOTES

1. ACCESS TO PRIVATE DRIVES SHALL BE MAINTAINED UNLESS OTHERWISE NOTED IN PLANS.

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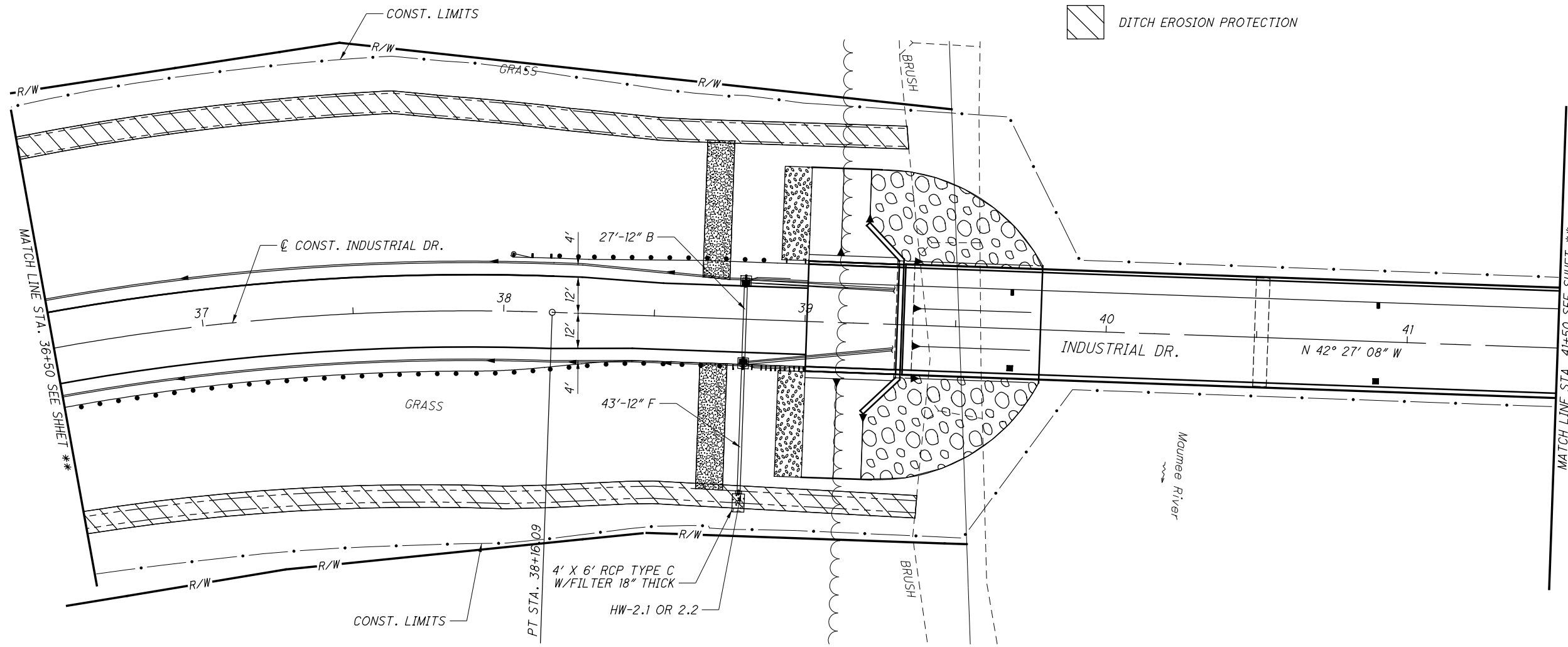
PLAN AND PROFILE - INDUSTRIAL DR., STA. 31+42.43 TO STA. 36+50.00

HEN-NEW MAUMEE RIVER BRIDGE

CALCULATED
CHECKED

23
180

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DITCH EROSION PROTECTION

CURVE DATA - INDUSTRIAL DR.

P.I. STA. 36+63.17

$\Delta = 22^\circ 55' 17''$ (RT)

$D_c = 7^\circ 23' 35''$

$R = 775.00'$

$T = 157.12'$

$L = 310.04'$

$E = 15.77'$

$C = 307.98'$

C.B. = N 53° 54' 46" W

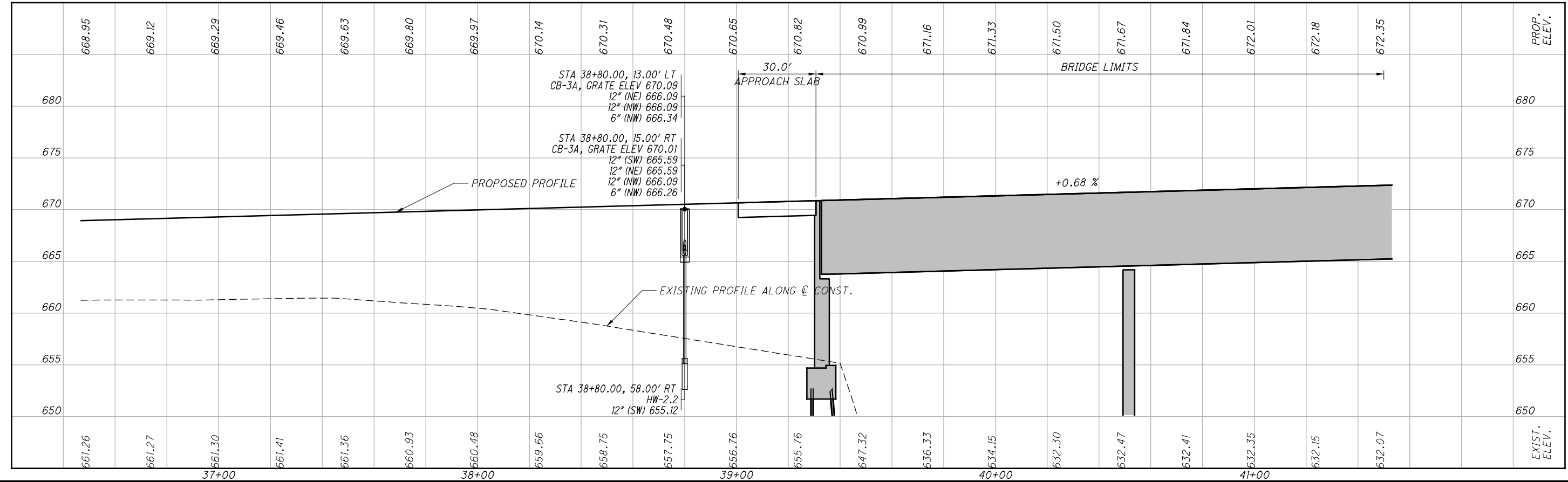
CALCULATED

CHECKED

0 20 40

10

HORIZONTAL SCALE IN FEET



PLAN AND PROFILE - INDUSTRIAL DR.
STA. 36+50.00 TO STA. 41+50.00

HEN-NEW MAUMEE RIVER BRIDGE

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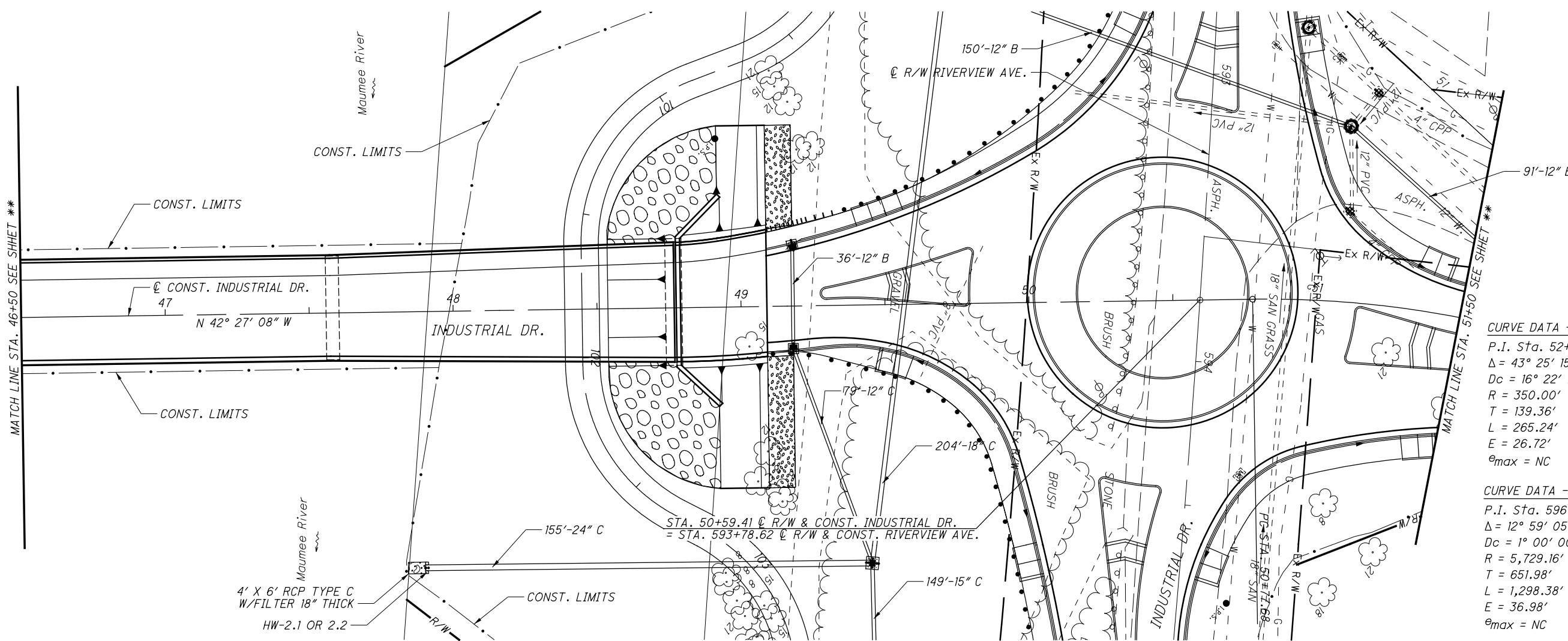


PLAN AND PROFILE - INDUSTRIAL DR.
STA. 46+50.00 TO STA. 51+50.00

HEN-NEW MAUMEE RIVER BRIDGE

25
180

CALCULATED
CHECKED

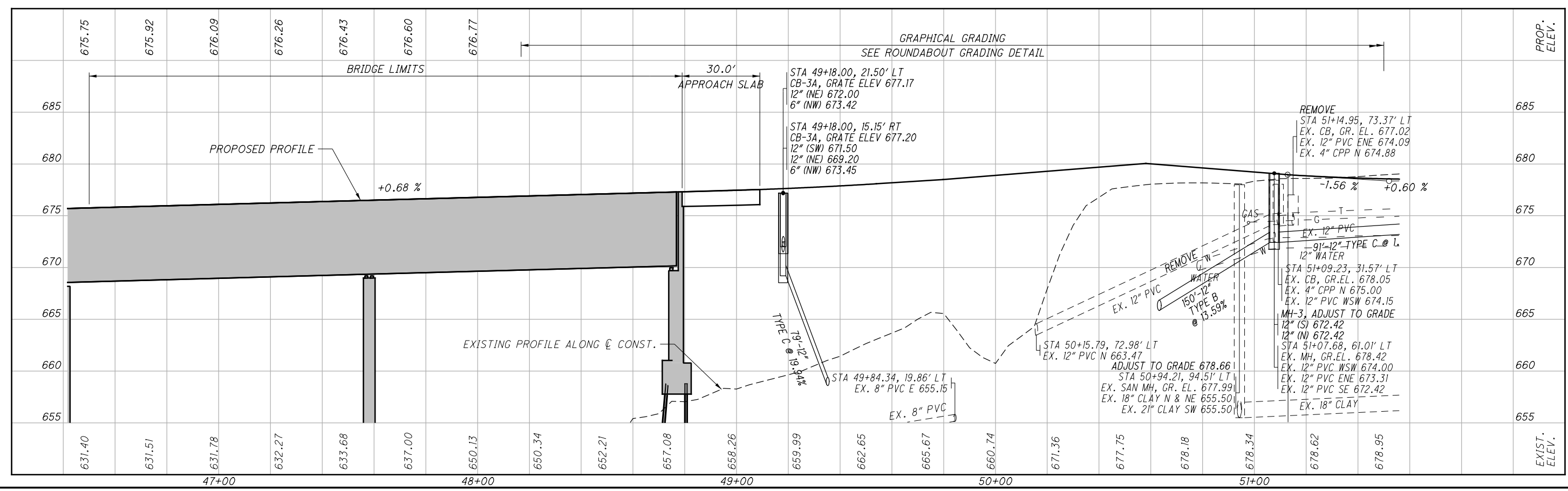


CURVE DATA - INDUSTRIAL DR.

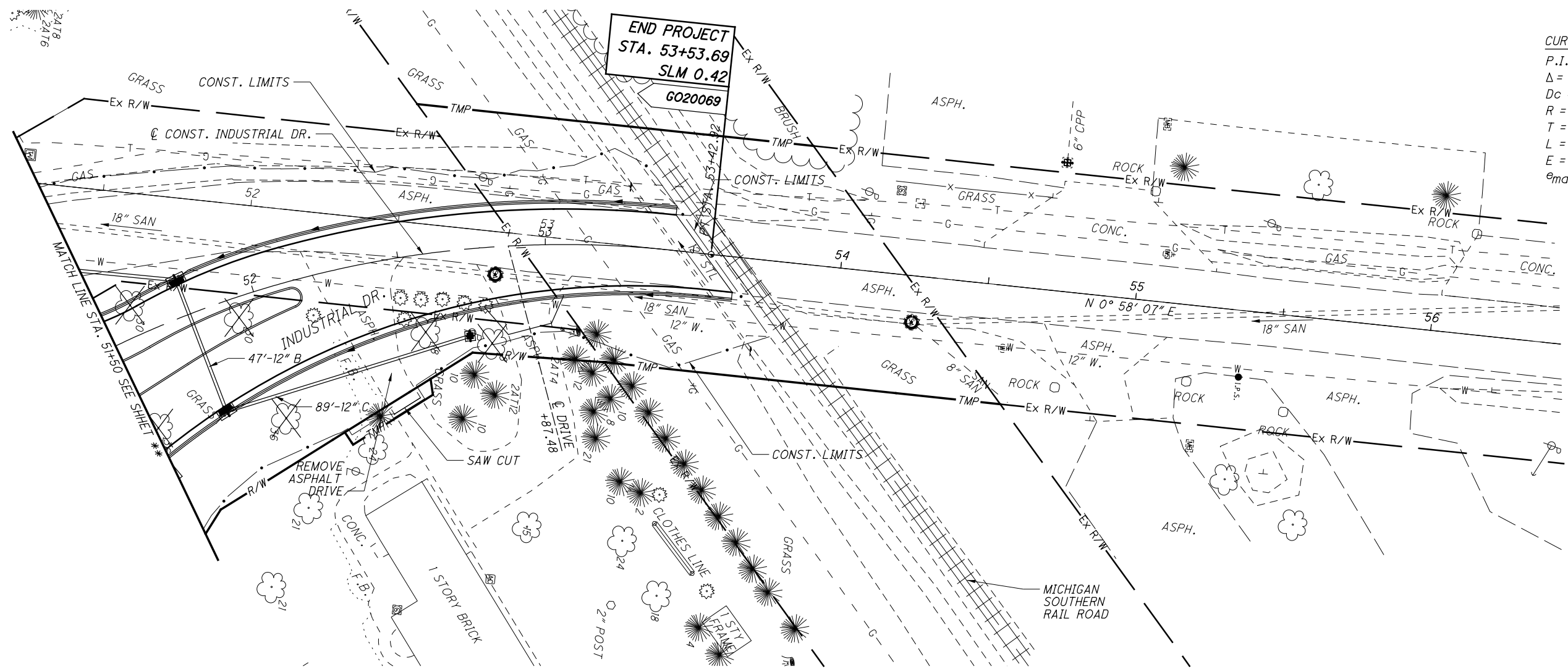
P.I. Sta. 52+17.04
 $\Delta = 43^\circ 25' 15''$ (RT)
 $D_c = 16^\circ 22' 13''$
 $R = 350.00'$
 $T = 139.36'$
 $L = 265.24'$
 $E = 26.72'$
 $\theta_{max} = NC$

CURVE DATA - RIVERVIEW AVE.

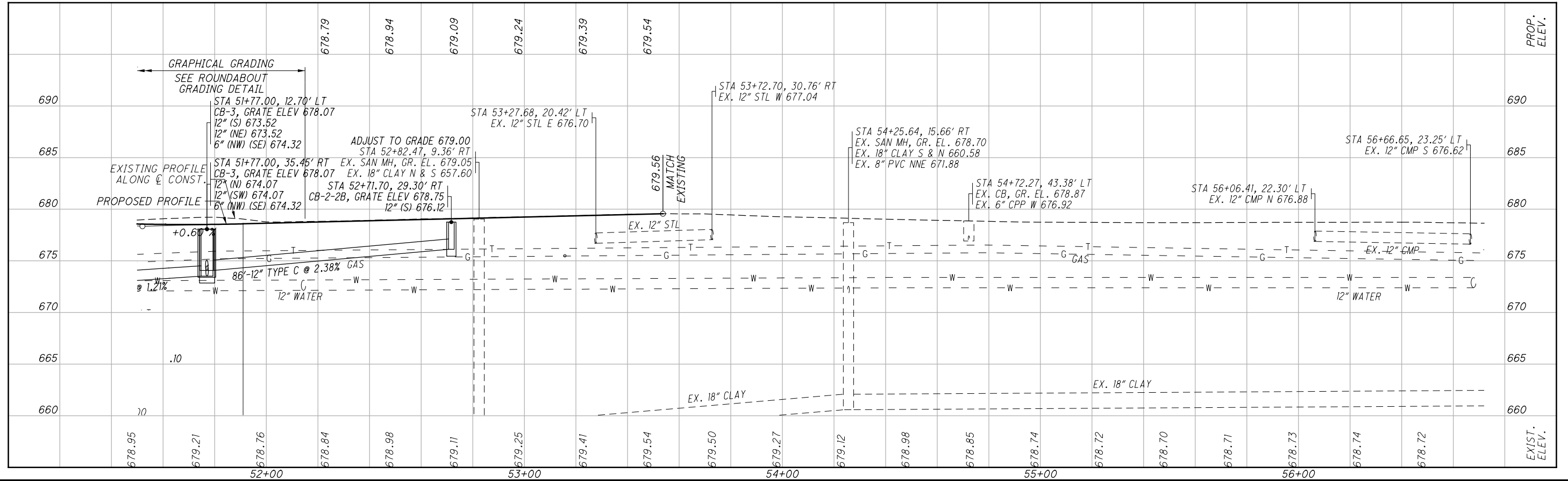
P.I. Sta. 596+98.18
 $\Delta = 12^\circ 59' 05''$ (RT)
 $D_c = 1^\circ 00' 00''$
 $R = 5,729.16'$
 $T = 651.98'$
 $L = 1,298.38'$
 $E = 36.98'$
 $\theta_{max} = NC$



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CURVE DATA - INDUSTRIAL DR.
 P.I. Sta. 52+17.04
 $\Delta = 43^\circ 25' 15''$ (RT)
 $Dc = 16^\circ 22' 13''$
 $R = 350.00'$
 $T = 139.36'$
 $L = 265.24'$
 $E = 26.72'$
 $e_{max} = NC$



PLAN AND PROFILE - INDUSTRIAL DR.
STA. 51+50.00 TO STA. 56+50.00

HEN-NEW MAUMEE RIVER BRIDGE

26
180



0 20 40
HORIZONTAL SCALE IN FEET

CALCULATED
CHECKED

PLAN AND PROFILE - RIVERVIEW AVE.
STA. 586+50.00 TO STA. 591+50.00

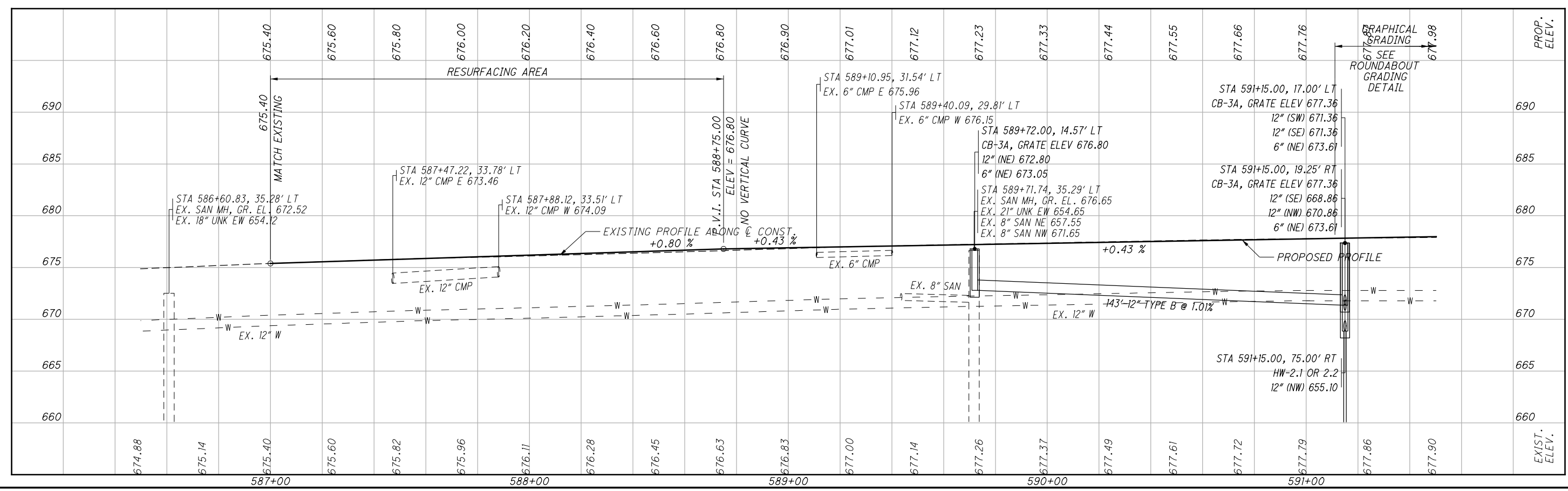
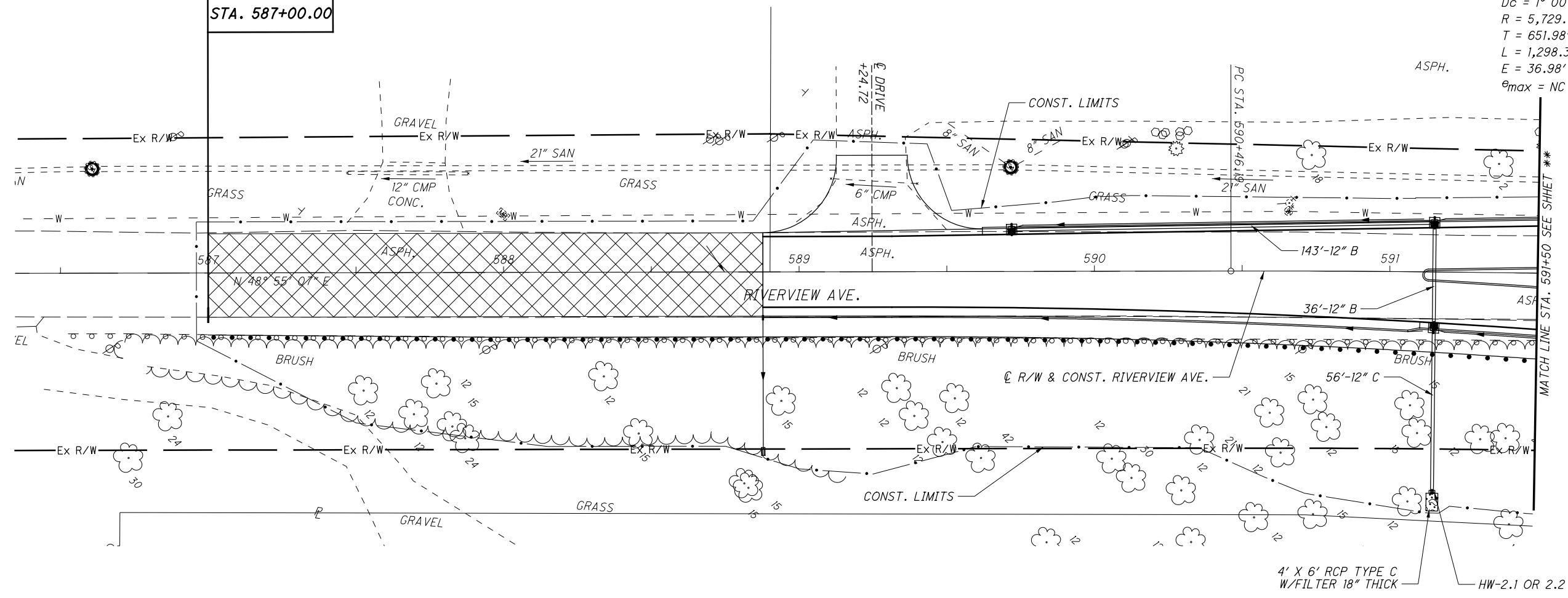
HEN-NEW MAUMEE RIVER BRIDGE

27
180

CURVE DATA - RIVERVIEW AVE.
P.I. Sta. 596+98.18
 $\Delta = 12^\circ 59' 05''$ (RT)
 $Dc = 1^\circ 00' 00''$
 $R = 5,729.16'$
 $T = 651.98'$
 $L = 1,298.38'$
 $E = 36.98'$
 $\theta_{max} = NC$

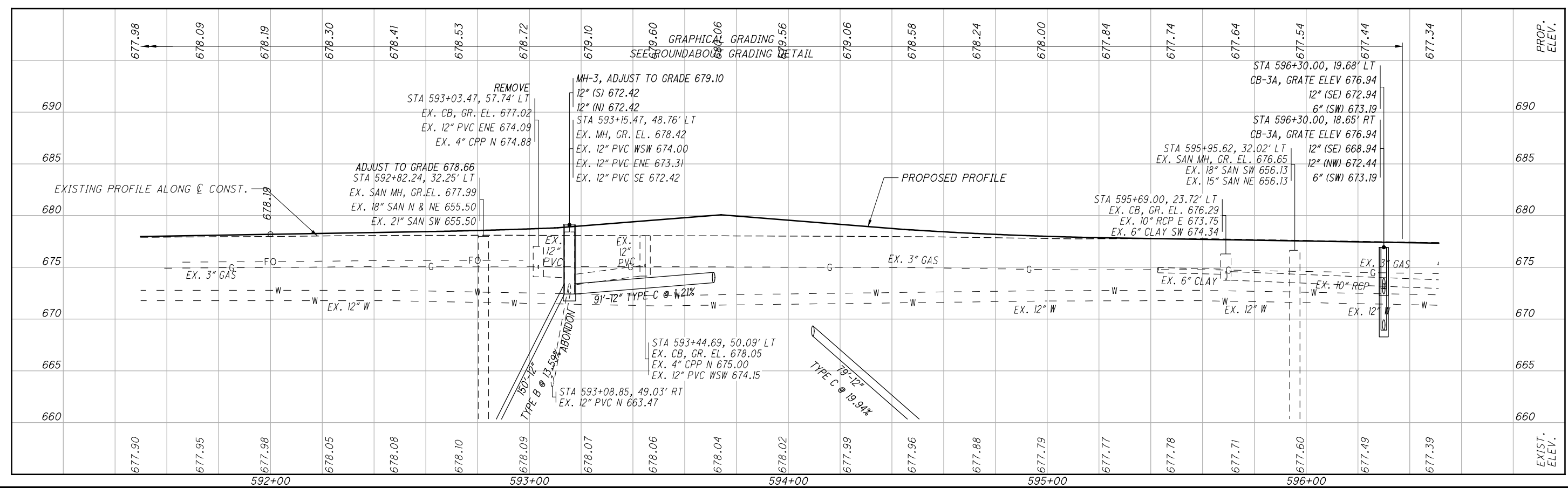
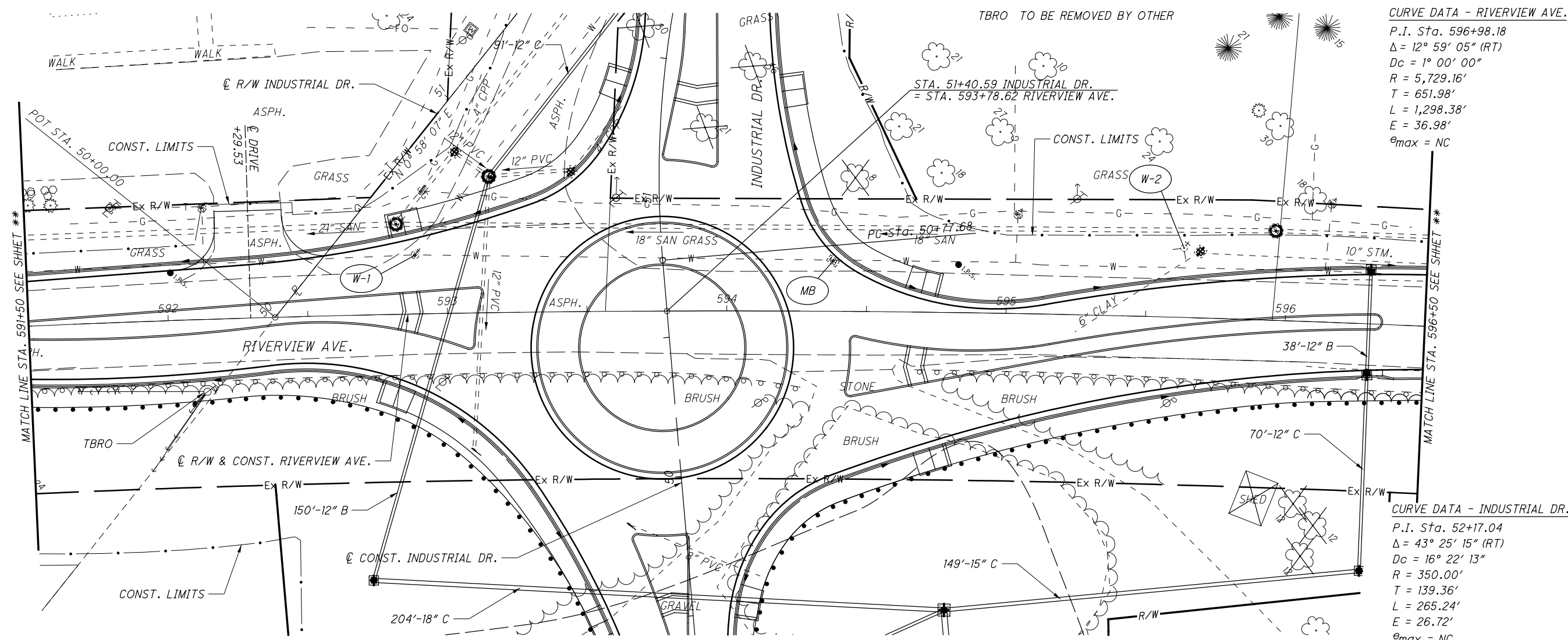
RESURFACING AREA

BEGIN WORK
STA. 587+00.00



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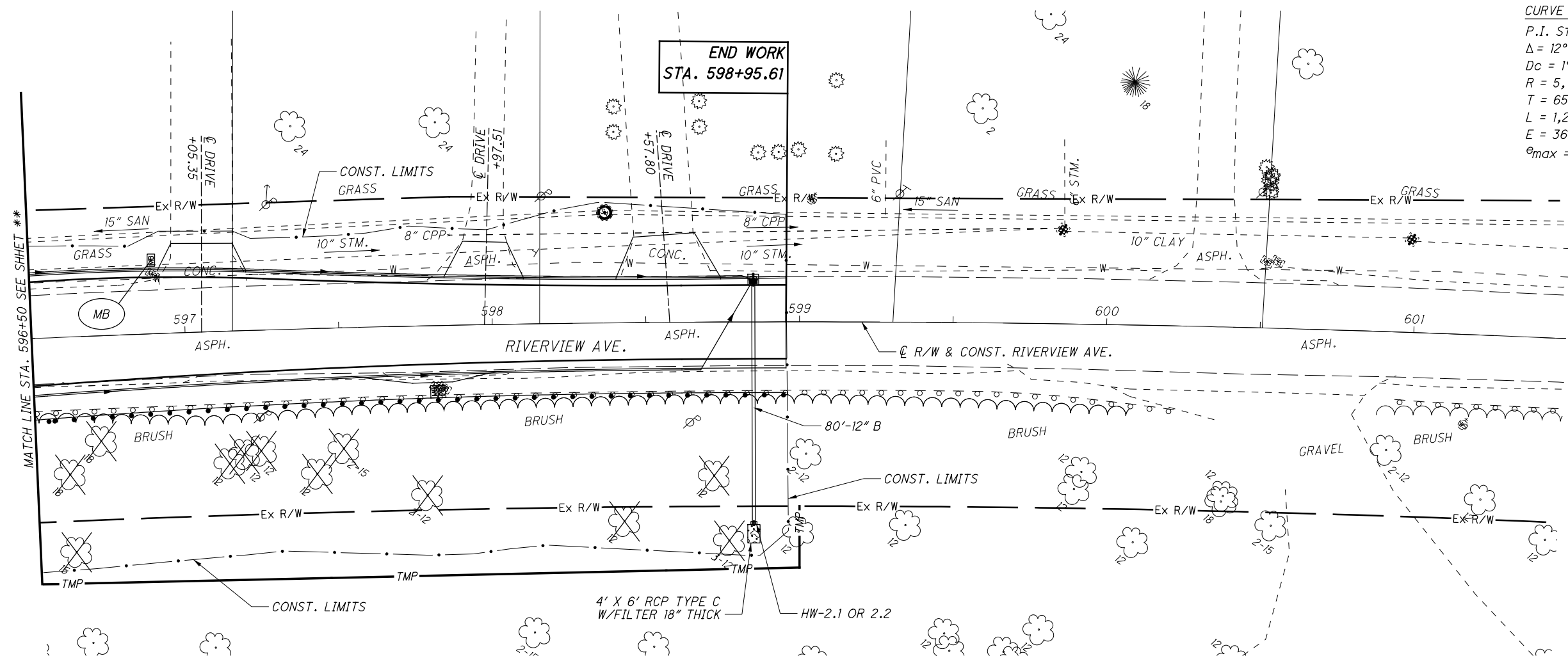
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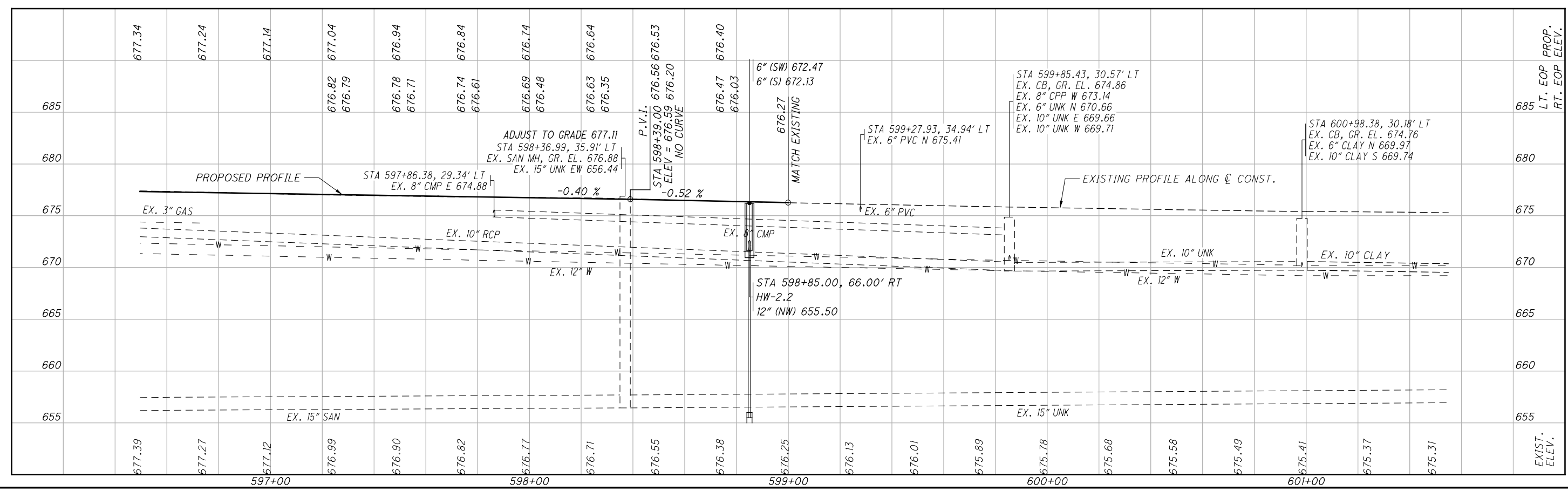
PLAN AND PROFILE - RIVERVIEW AVE:
STA. 591+50.00 TO STA. 596+50.00

HEN-NEW MAUMEE RIVER BRIDGE

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CURVE DATA - RIVERVIEW AVE.
 P.I. Sta. 596+98.18
 $\Delta = 12^\circ 59' 05''$ (RT)
 $D_c = 1^\circ 00' 00''$
 $R = 5,729.16'$
 $T = 651.98'$
 $L = 1,298.38'$
 $E = 36.98'$
 $e_{max} = NC$

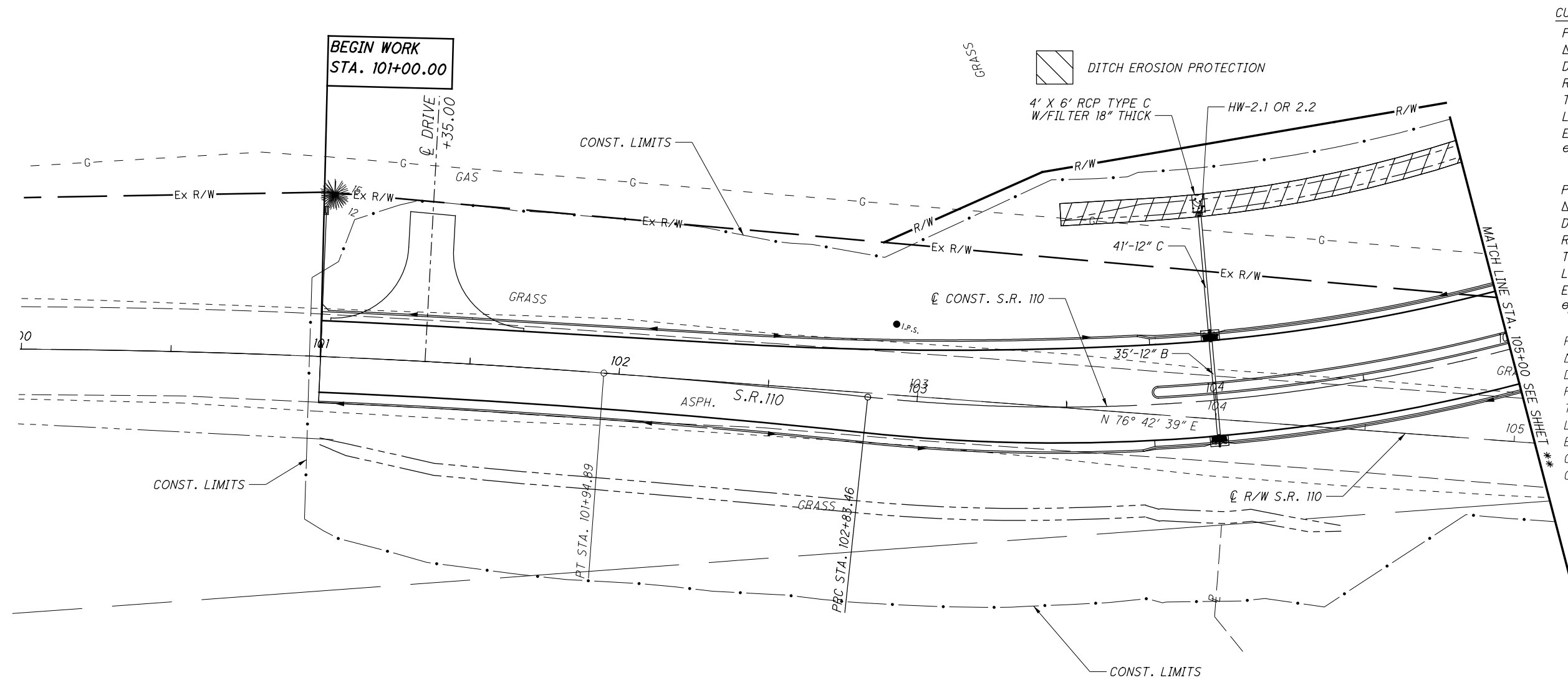


PLAN AND PROFILE - RIVERVIEW AVE:
STA. 596+50 TO STA. 601+50.00

HEN-NEW MAUMEE RIVER BRIDGE



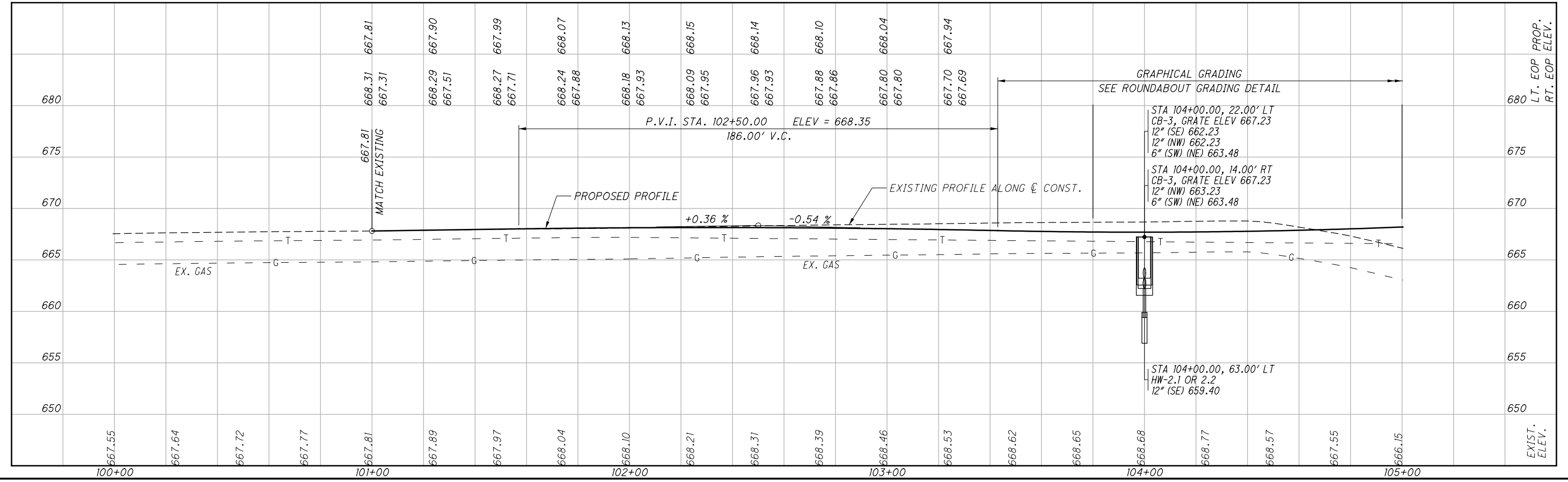
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CURVE DATA - S.R. 110
 P.I. STA. 103+94.64
 $\Delta = 20^\circ 59' 50''$ (LT)
 $Dc = 9^\circ 32' 57''$
 $R = 600.00'$
 $T = 111.19'$
 $L = 219.88'$
 $E = 10.22'$
 $\theta_{max} = NC$

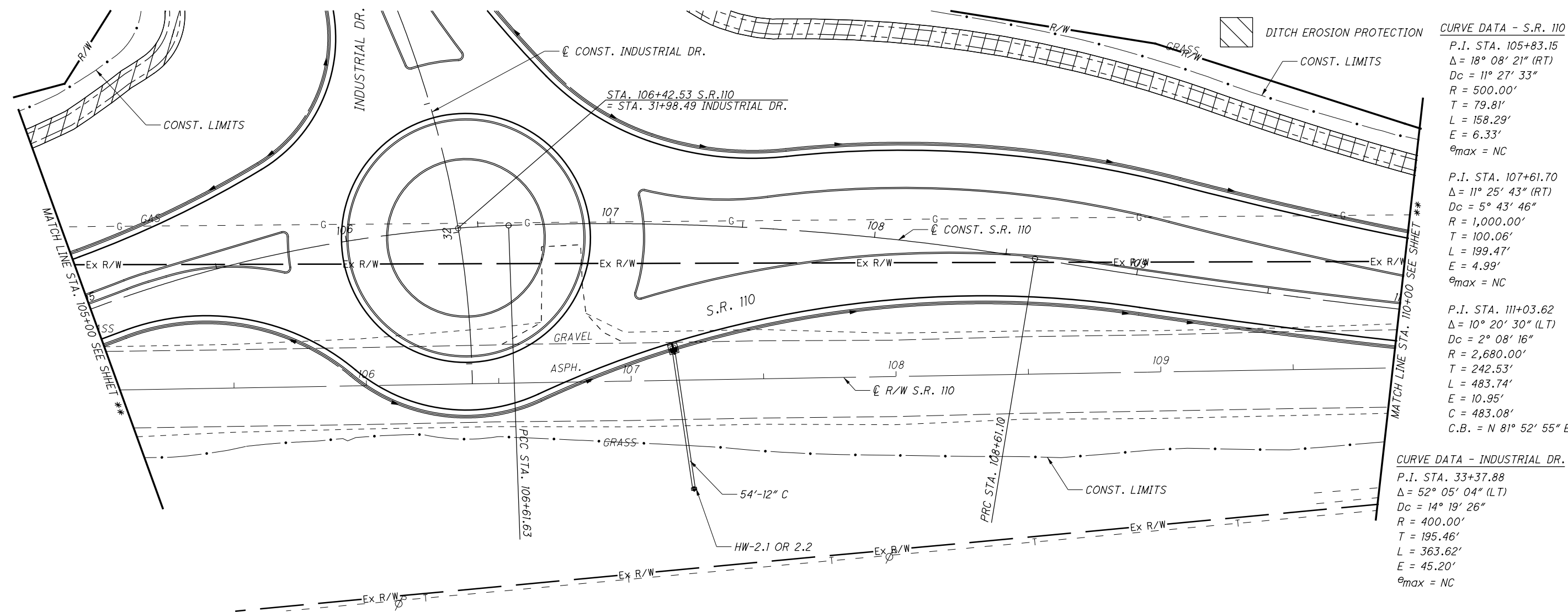
P.I. STA. 97+48.28
 $\Delta = 21^\circ 40' 09''$ (RT)
 $Dc = 2^\circ 00' 00''$
 $R = 2,864.80'$
 $T = 548.28'$
 $L = 1,083.46'$
 $E = 51.99'$
 $\theta_{max} = NC$

P.I. STA. 90+33.37
 $\Delta = 49^\circ 52' 00''$ (RT)
 $Dc = 2^\circ 00' 00''$
 $R = 2,864.80'$
 $T = 1,331.82'$
 $L = 2,493.34'$
 $E = 294.45'$
 $C = 2,415.39'$
 $C.B. = N 51^\circ 46' 39'' E$



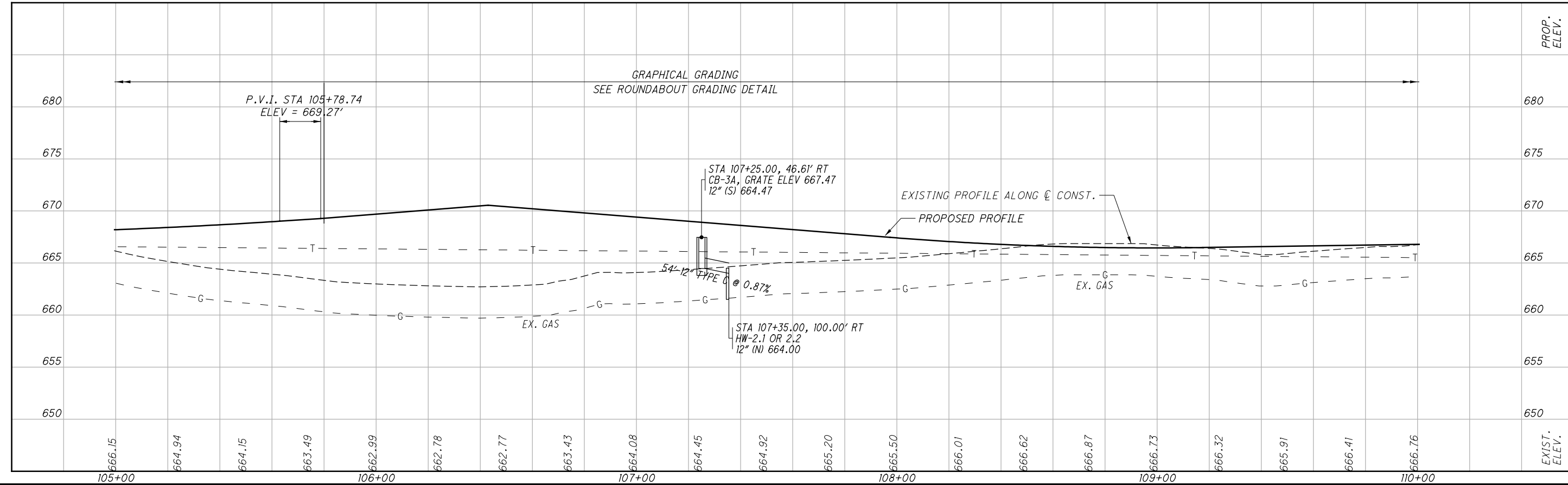
HEN-NEW MAUMEE RIVER BRIDGE
PLAN AND PROFILE - S.R.110
STA. 100+00.00 TO STA. 105+00.00
 30
 180

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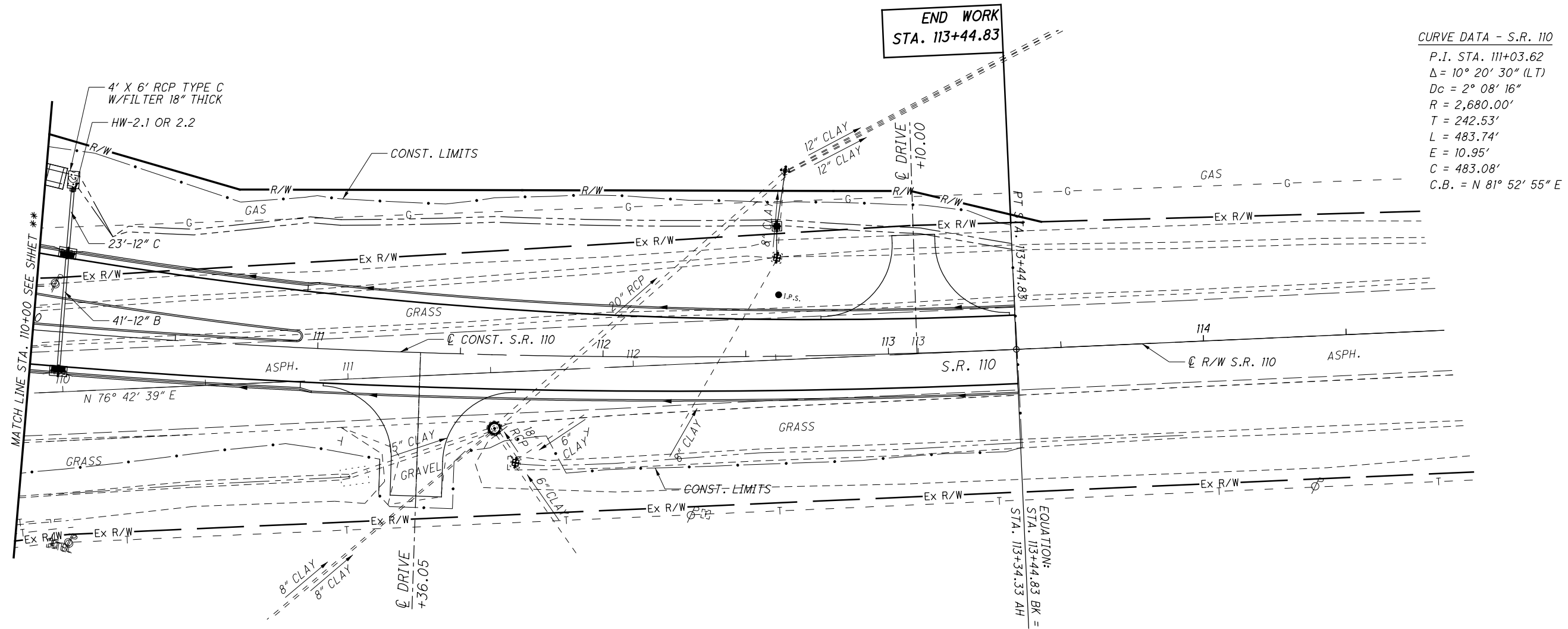


CALCULATED
 CHECKED

**PLAN AND PROFILE - S.R.110
 STA. 105+00.00 TO STA. 110+00.00**



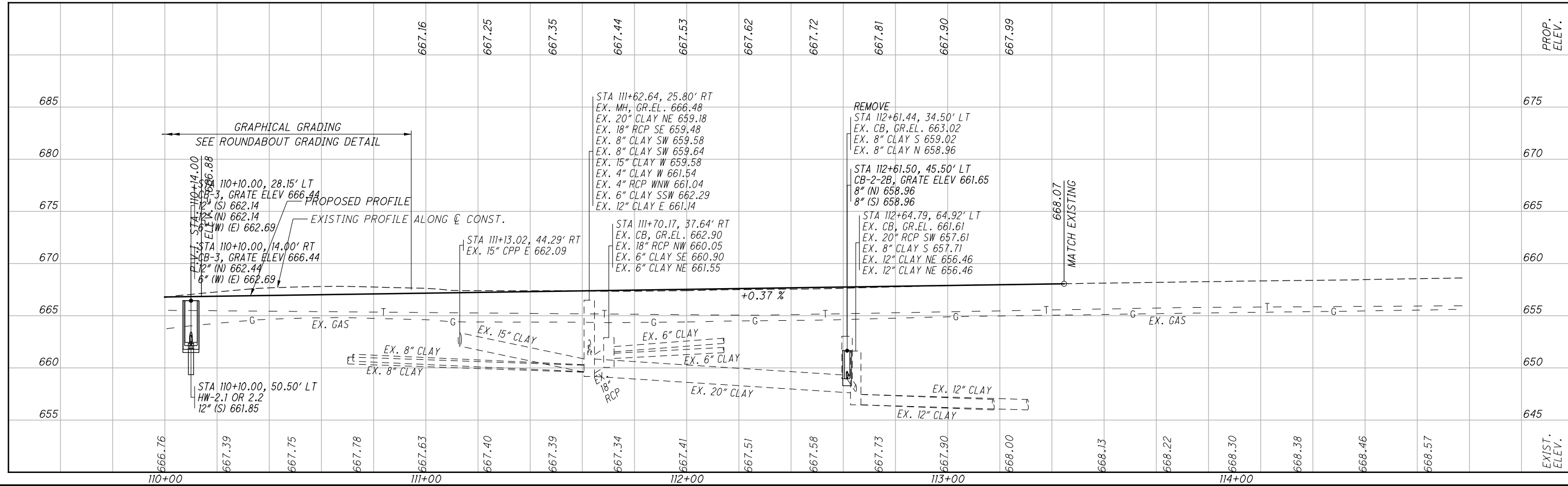
**HEN-NEW MAUMEE
 RIVER BRIDGE**



CURVE DATA - S.R. 110
 P.I. STA. 111+03.62
 $\Delta = 10^\circ 20' 30''$ (LT)
 $Dc = 2^\circ 08' 16''$
 $R = 2,680.00'$
 $T = 242.53'$
 $L = 483.74'$
 $E = 10.95'$
 $C = 483.08'$
 $C.B. = N 81^\circ 52' 55'' E$

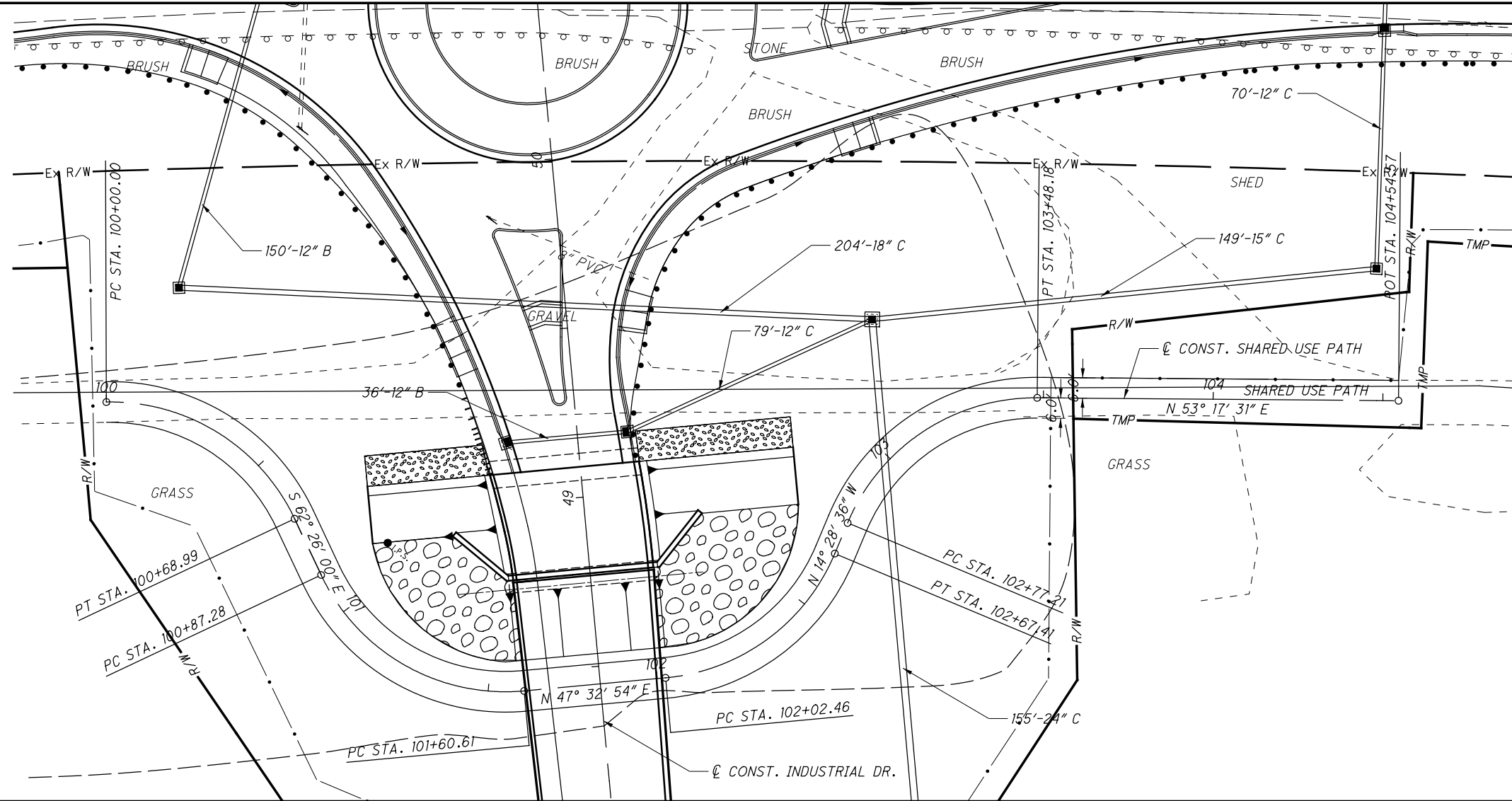


PLAN AND PROFILE - S.R.110
STA. 110+00.00 TO STA. 115+00.00



HEN-NEW MAUMEE RIVER BRIDGE

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CURVE DATA - SHARED USE PATH

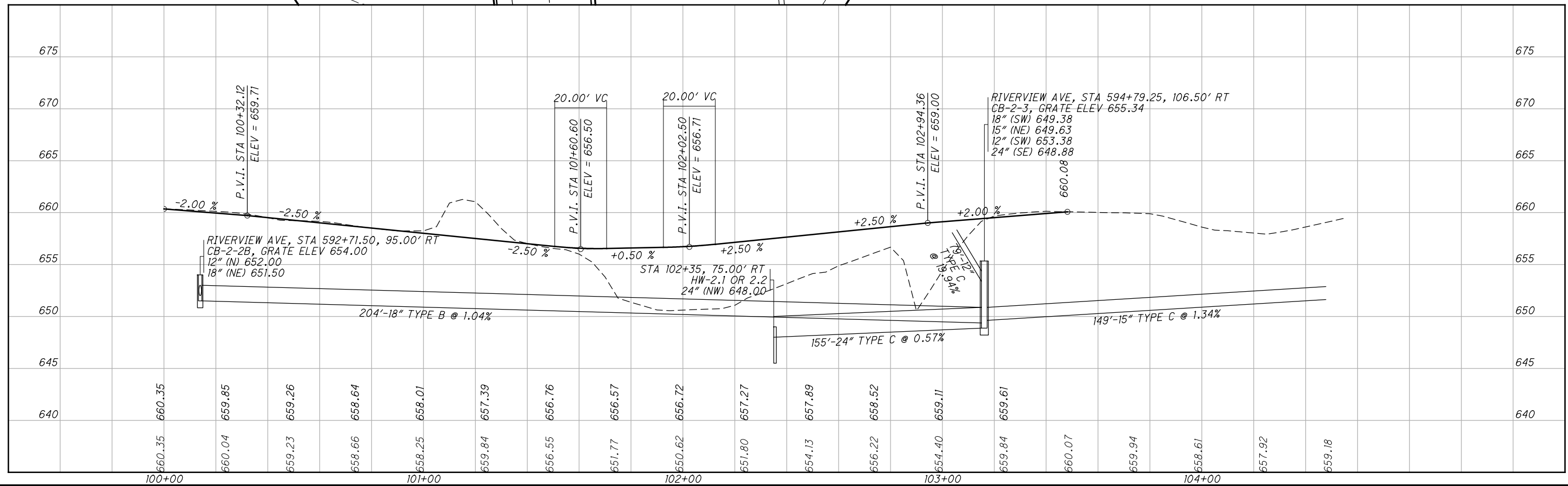
Station	Delta	Dc	R	T	L	E	Emax
P.I. Sta. 100+38.87	Δ = 65° 52' 36" (RT)	Dc = 95° 29' 35"	R = 60.00'	T = 38.87'	L = 68.99'	E = 11.49'	Emax = NC
P.I. Sta. 101+29.31	Δ = 70° 01' 06" (LT)	Dc = 95° 29' 35"	R = 60.00'	T = 42.03'	L = 73.32'	E = 13.25'	Emax = NC
P.I. Sta. 102+38.53	Δ = 62° 01' 29" (LT)	Dc = 95° 29' 35"	R = 60.00'	T = 36.07'	L = 64.95'	E = 10.01'	Emax = NC
P.I. Sta. 103+17.50	Δ = 67° 46' 06" (RT)	Dc = 95° 29' 35"	R = 60.00'	T = 40.29'	L = 70.97'	E = 12.27'	Emax = NC

VERTICAL CLEARANCE

MINIMUM VERTICAL CLEARANCE IS 12.82' AT STA. 102+01.43, 6.0' LT.

DESIGN SPEED

DESIGN SPEED IS 18 MPH



CALCULATED
CHECKED

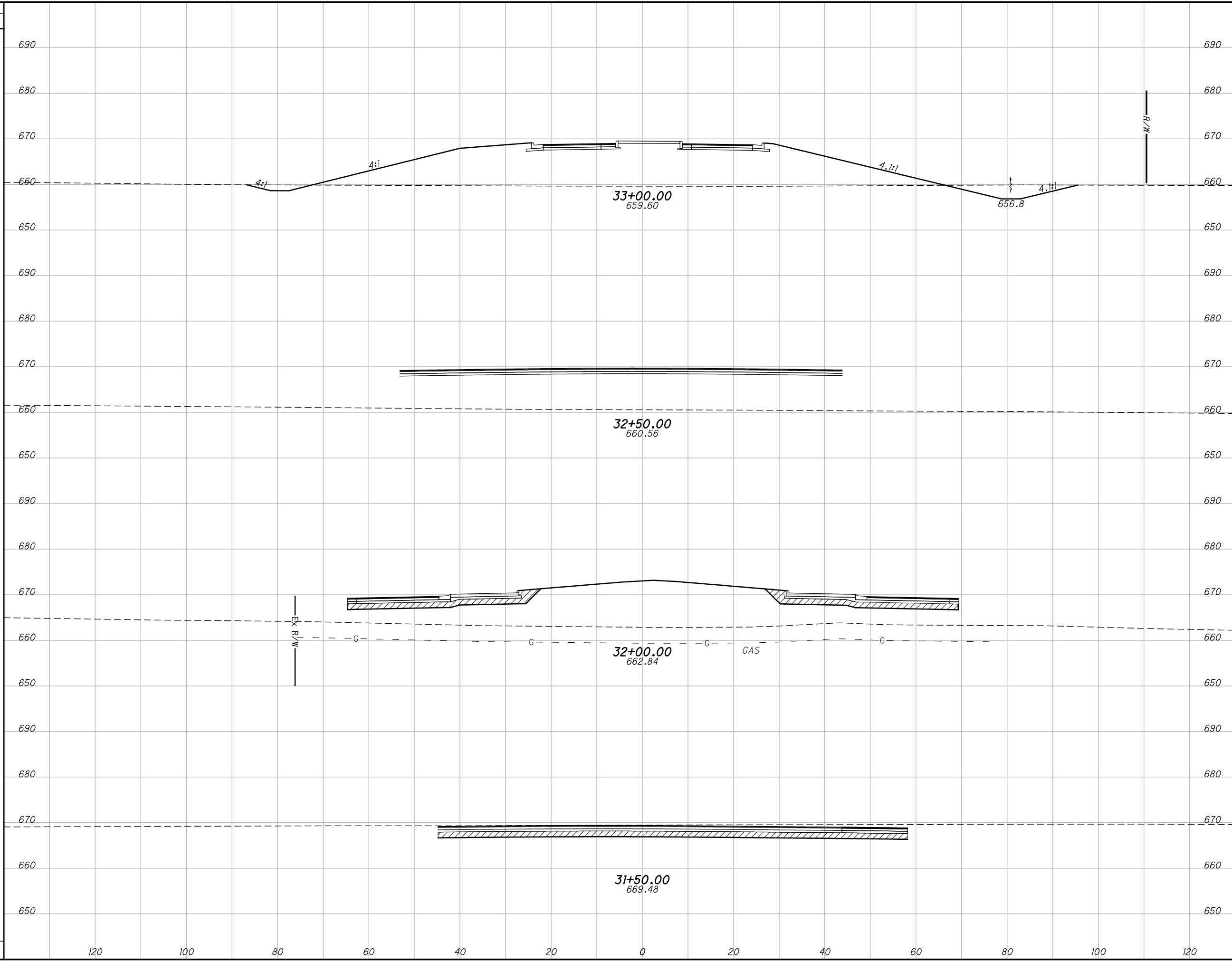
**PLAN AND PROFILE - SHARED USE PATH
STA. 100+00.00 TO 104+54.57**

HEN-NEW MAUMEE RIVER BRIDGE

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SEEDING

END WIDTH	SO. YDS.



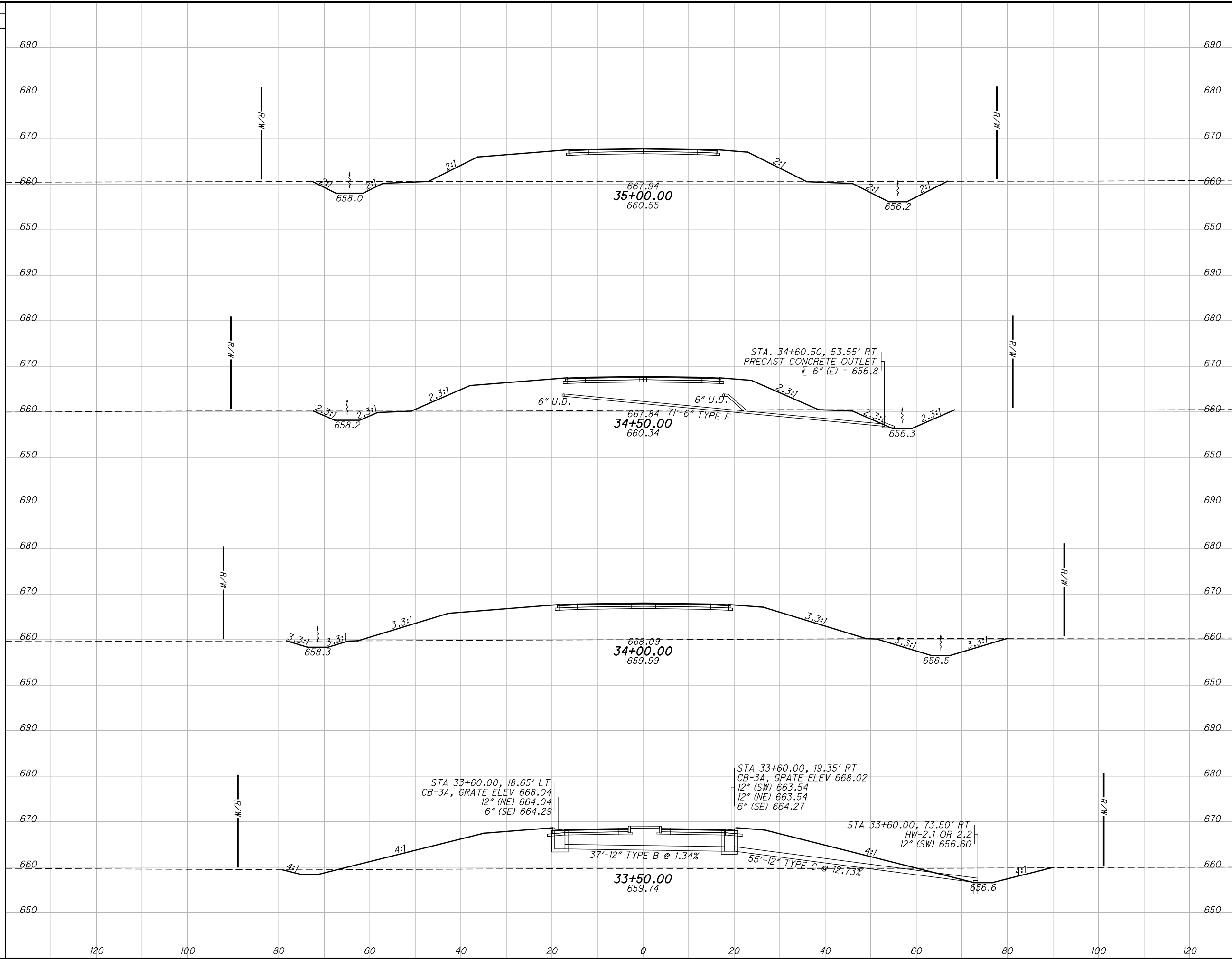
END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

CROSS SECTIONS INDUSTRIAL DR.
STA. 31+50.00 TO STA. 33+00.00

HEN-NEW MAUMEE RIVER BRIDGE

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SEEDING
END SO.
WIDTH YDS.



END AREA	VOLUME	CALCULATED	CHECKED				
				CUT	FILL	CUT	FILL

HEN-NEW MAUMEE RIVER BRIDGE

CROSS SECTIONS INDUSTRIAL DR.

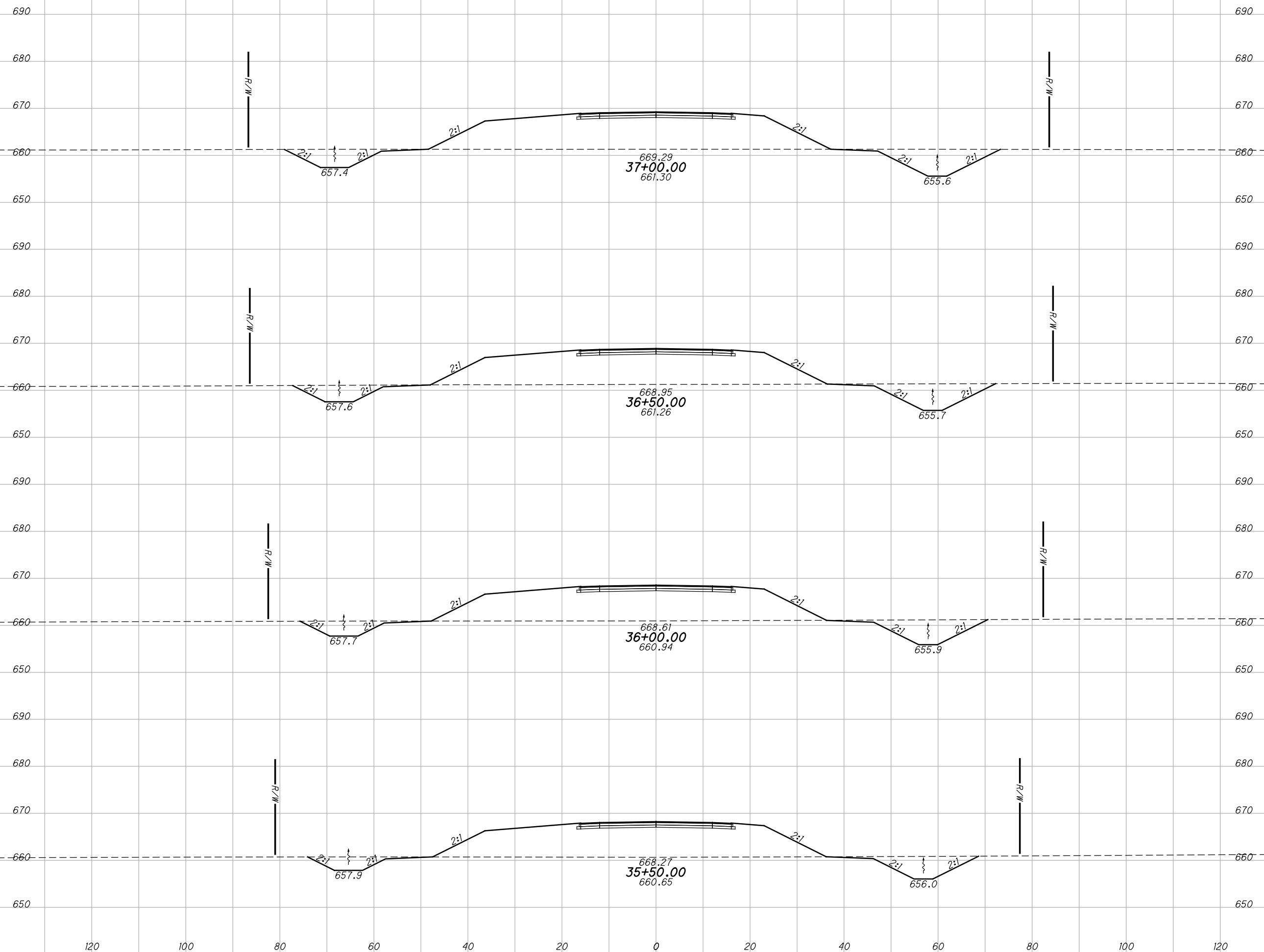
STA. 33+50.00 TO STA. 35+00.00

35
180

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SEEDING
END SO.
WIDTH YDS.

END AREA
CUT FILL
VOLUME
CUT FILL
CALCULATED
CHECKED



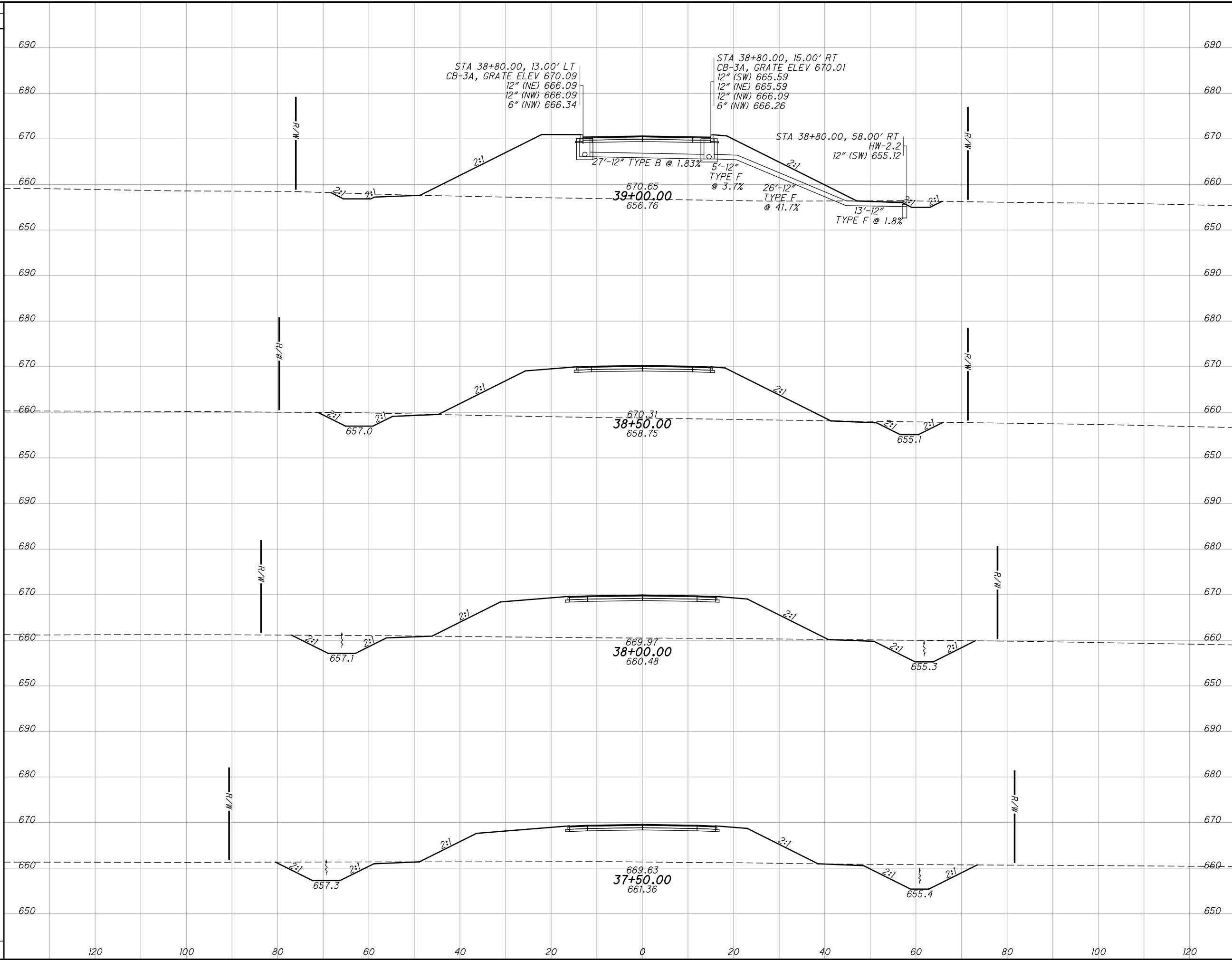
CROSS SECTIONS INDUSTRIAL DR.
STA. 35+50.00 TO STA. 37+00.00

HEN-NEW MAUMEE
RIVER BRIDGE

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SEEDING

END WIDTH	SO. YDS.



END AREA	VOLUME	CALCULATED	CHECKED

HEN-NEW MAUMEE RIVER BRIDGE

CROSS SECTIONS INDUSTRIAL DR.

STA. 37+50.00 TO STA. 39+00.00

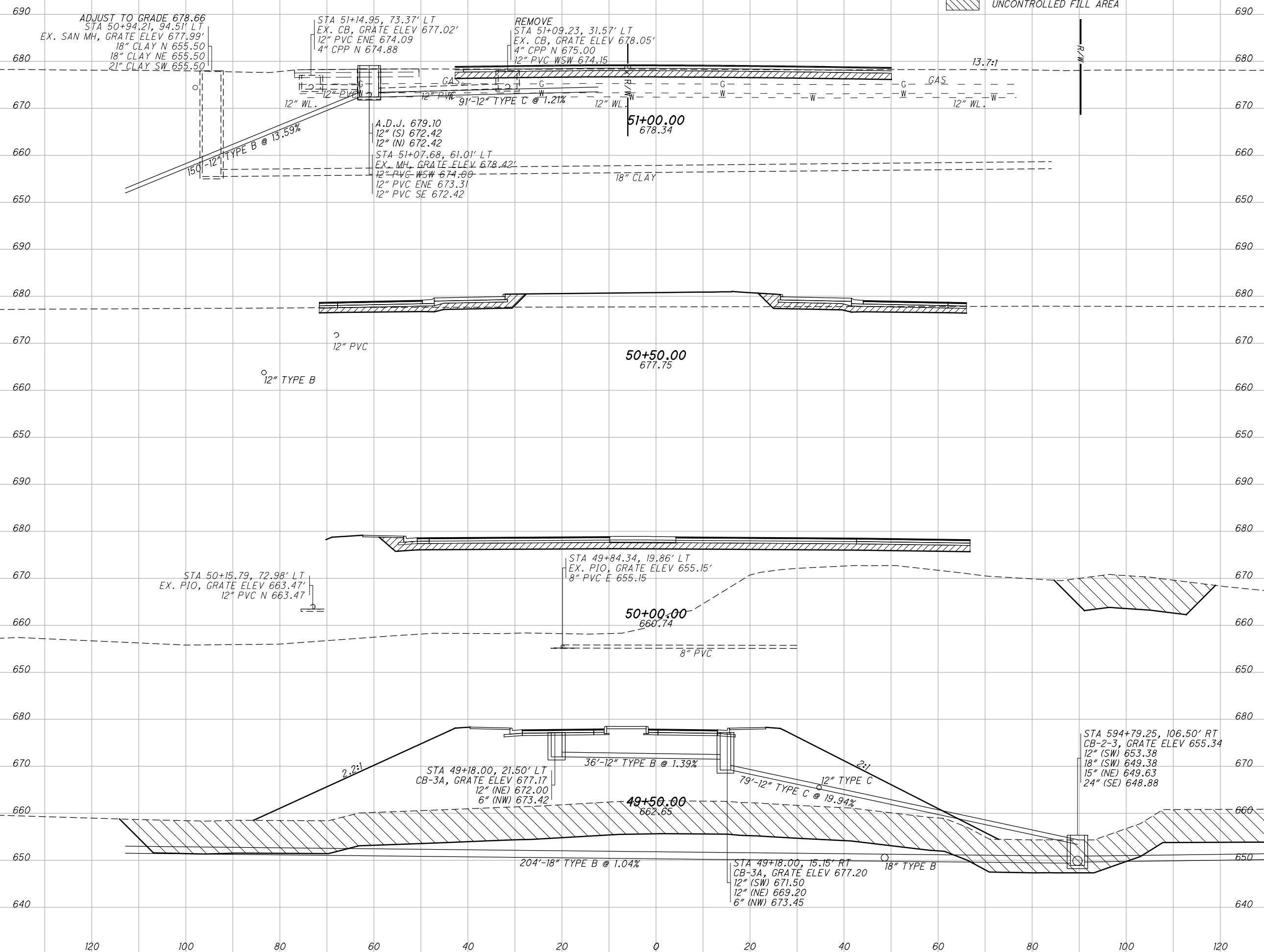
37
180

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SEEDING
END SO.
WIDTH YDS.

 ITEM 204 - EXCAVATION AND
 ITEM 204 - GRANULAR MATERIAL, TYPE B
UNCONTROLLED FILL AREA

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



**CROSS SECTIONS INDUSTRIAL DR.
STA. 49+50.00 TO STA. 51+00.00**

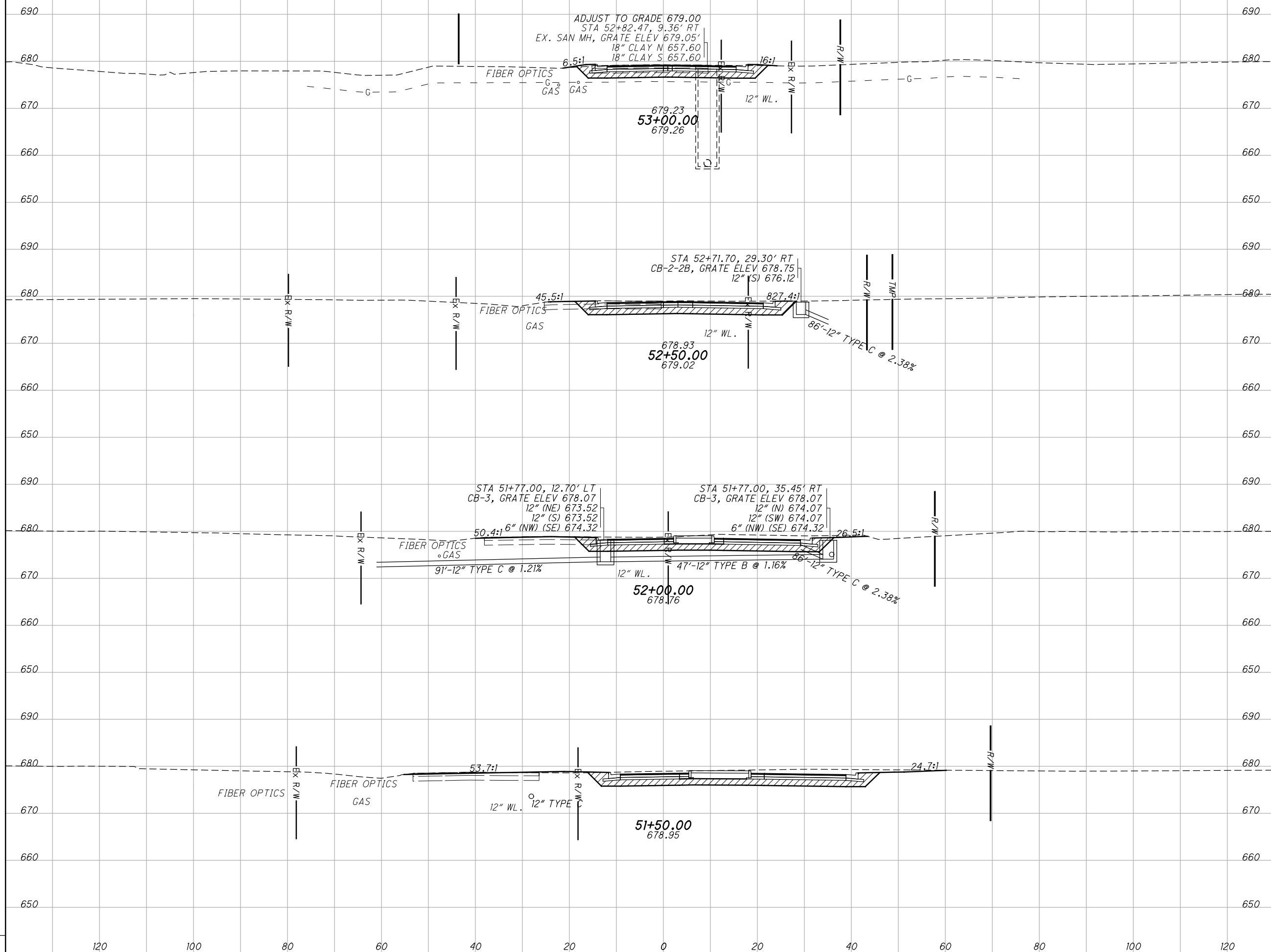
**HEN-NEW MAUMEE
RIVER BRIDGE**

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SEEDING
END SO.
WIDTH YDS.

ITEM 204 - EXCAVATION AND
ITEM 204 - GRANULAR MATERIAL, TYPE B

END AREA VOLUME
CUT FILL CUT FILL
CALCULATED CHECKED



CROSS SECTIONS INDUSTRIAL DR.
STA. 51+50.00 TO STA. 53+00.00

HEN-NEW MAUMEE
RIVER BRIDGE

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SEEDING
END SO.
WIDTH YDS.

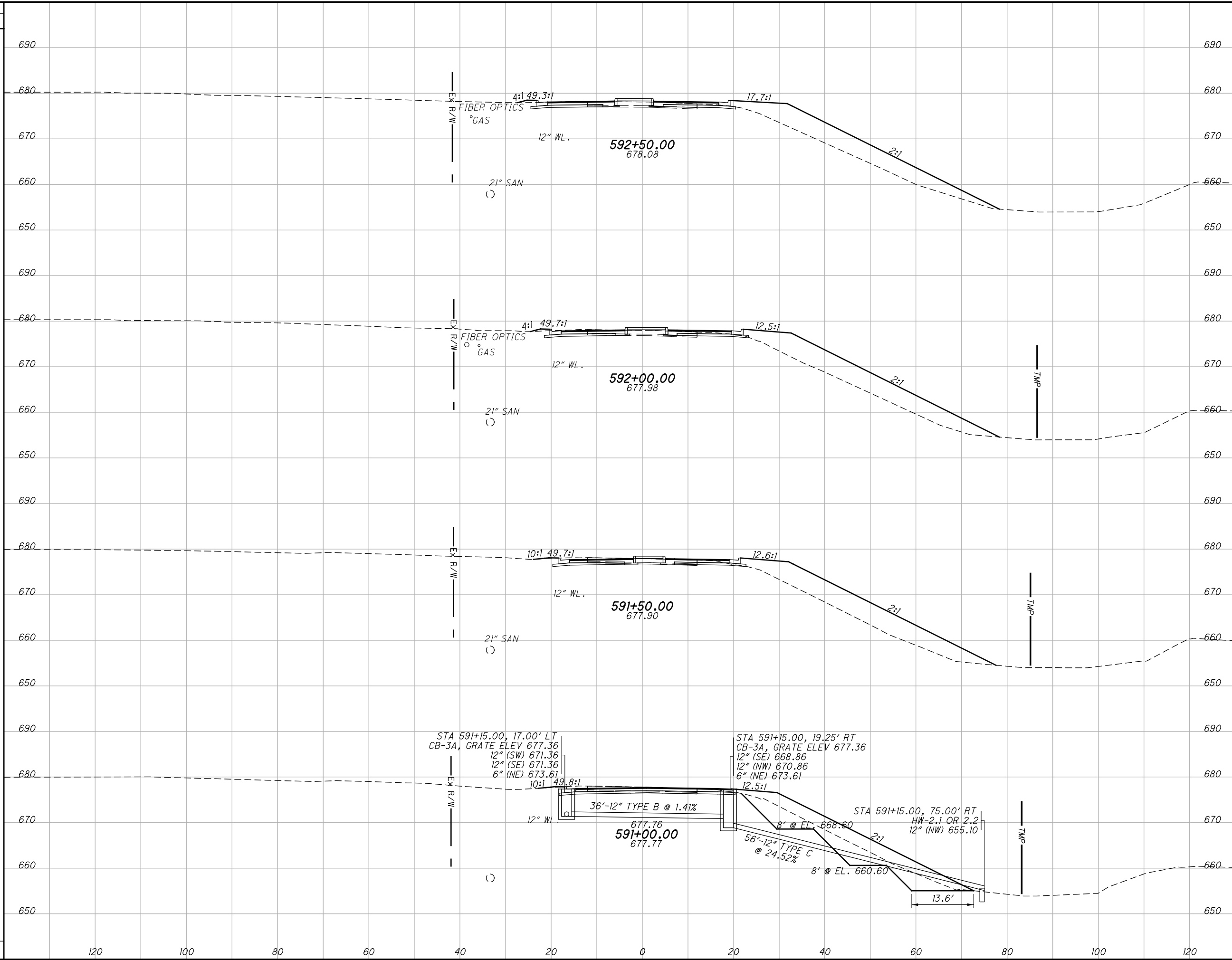


END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

HEN-NEW MAUMEE RIVER BRIDGE
CROSS SECTIONS RIVERVIEW AVE.
STA. 589+00.00 TO STA. 590+50.00

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SEEDING
END SO.
WIDTH YDS.

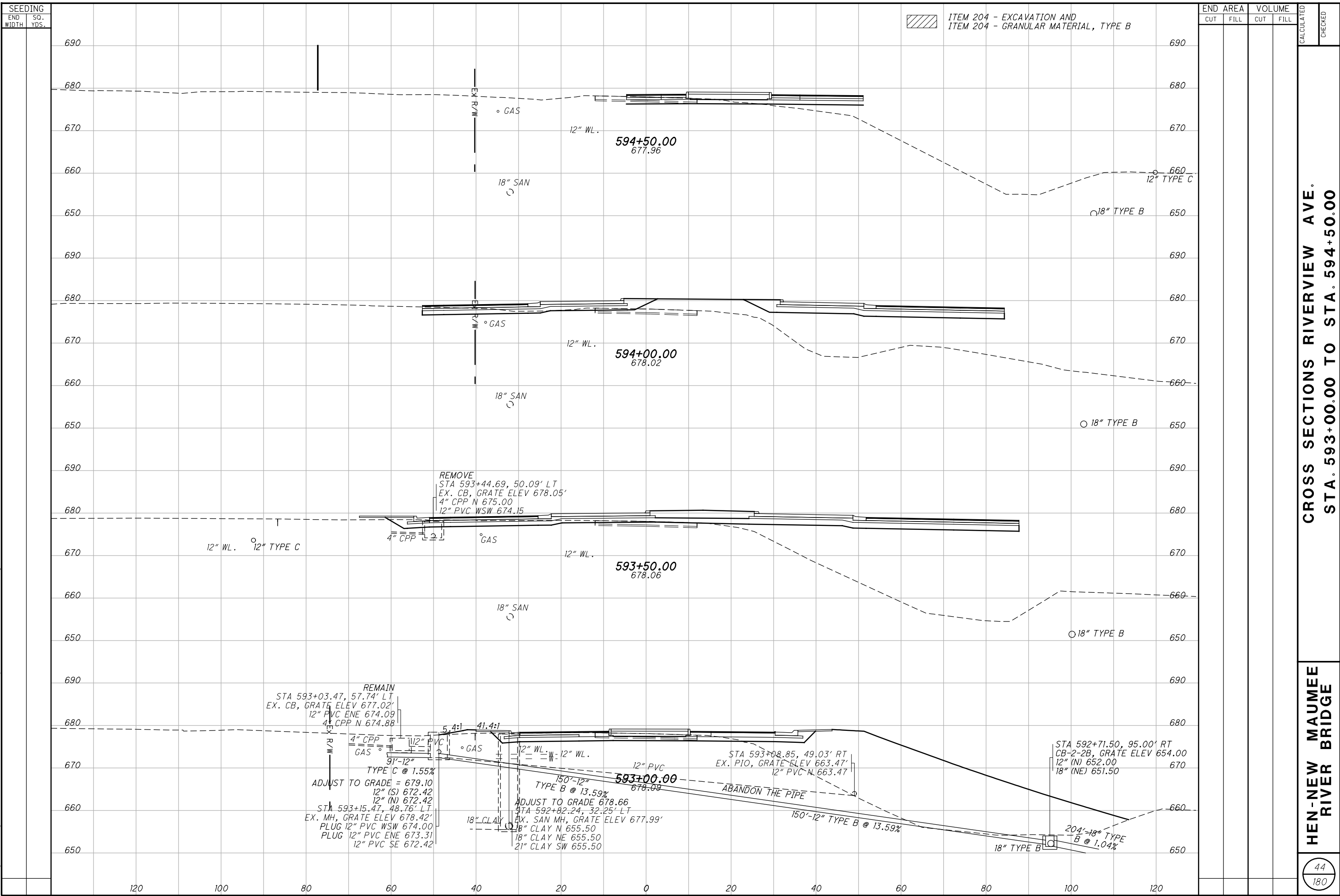


END	AREA		VOLUME		CALCULATED	CHECKED
	CUT	FILL	CUT	FILL		

HEN-NEW MAUMEE RIVER BRIDGE
CROSS SECTIONS RIVERVIEW AVE.
STA. 591+00.00 TO STA. 592+50.00

43
180

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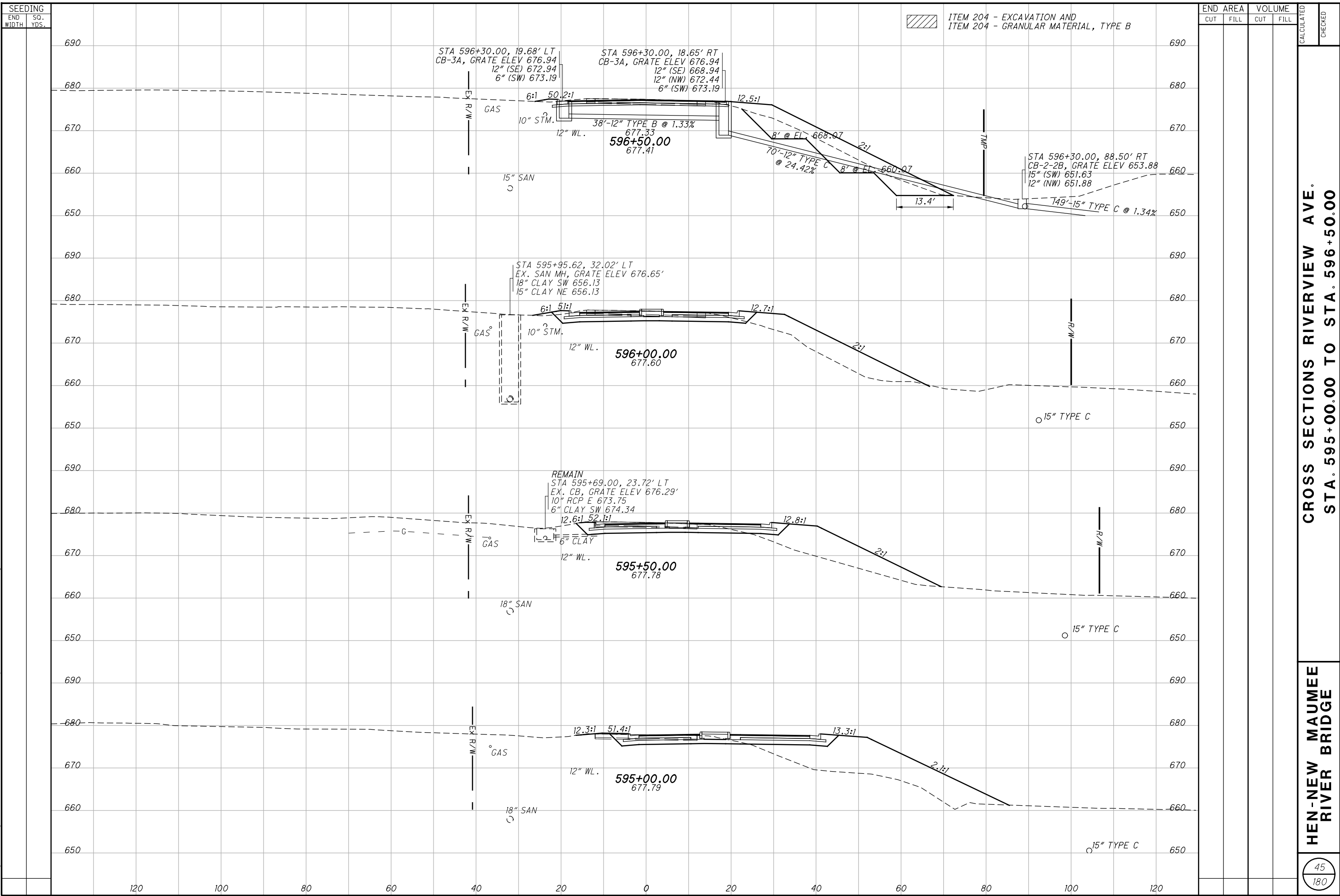
ITEM 204 - EXCAVATION AND
ITEM 204 - GRANULAR MATERIAL, TYPE B

SEEDING		END AREA		VOLUME		CALCULATED	CHECKED
END WIDTH	SO. YDS.	CUT	FILL	CUT	FILL		

**CROSS SECTIONS RIVERVIEW AVE.
STA. 593+00.00 TO STA. 594+50.00**

**HEN-NEW MAUMEE
RIVER BRIDGE**

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ITEM 204 - EXCAVATION AND
ITEM 204 - GRANULAR MATERIAL, TYPE B

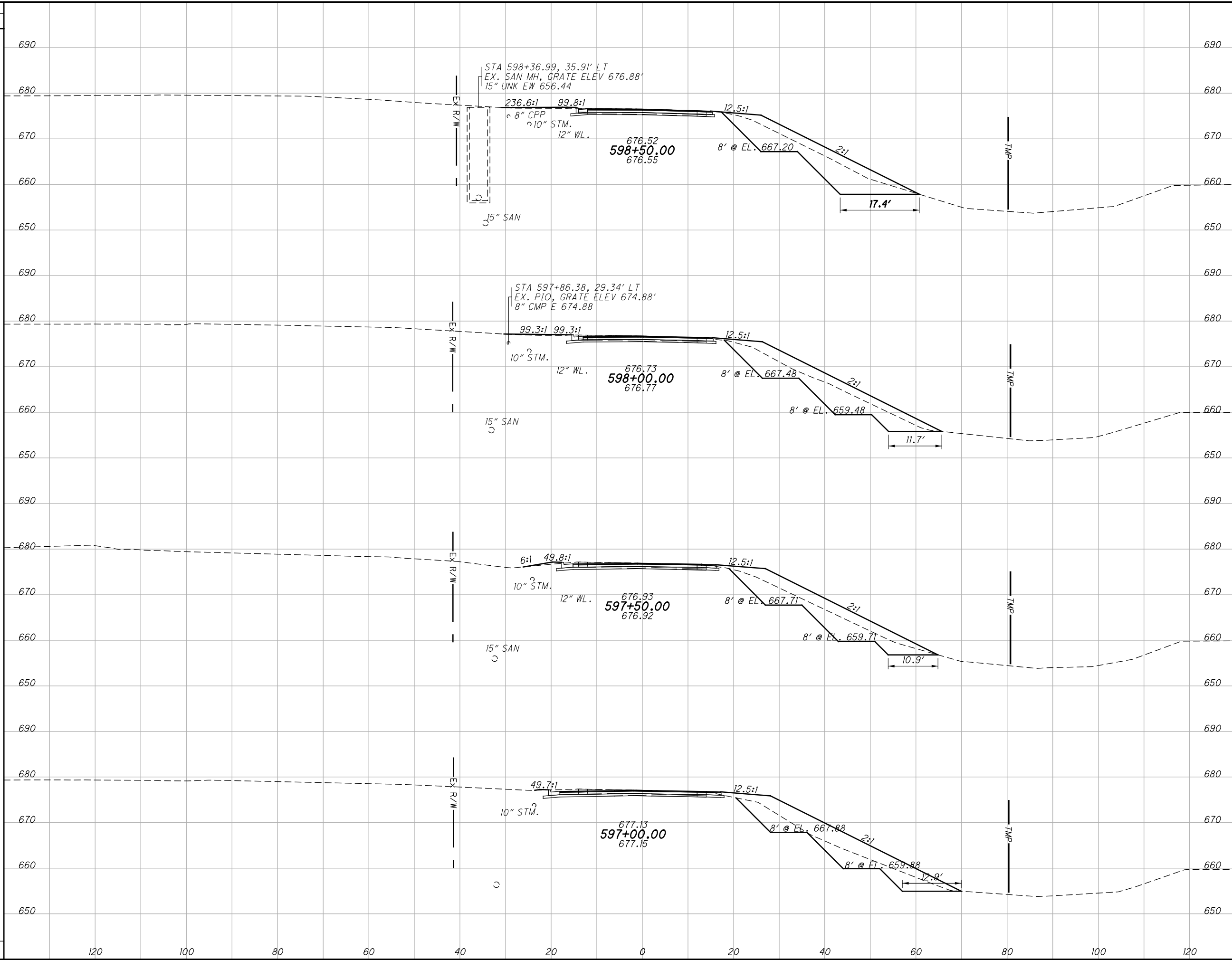
END	AREA		VOLUME		CALCULATED	CHECKED
	CUT	FILL	CUT	FILL		
690						
680						
670						
660						
650						
690						
680						
670						
660						
650						
690						
680						
670						
660						
650						
690						
680						
670						
660						
650						
690						
680						
670						
660						
650						

CROSS SECTIONS RIVERVIEW AVE.
STA. 595+00.00 TO STA. 596+50.00

HEN-NEW MAUMEE
RIVER BRIDGE

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SEEDING	
END WIDTH	SO. YDS.



END	AREA		VOLUME		CALCULATED	CHECKED
	CUT	FILL	CUT	FILL		
690						
680						
670						
660						
650						
690						
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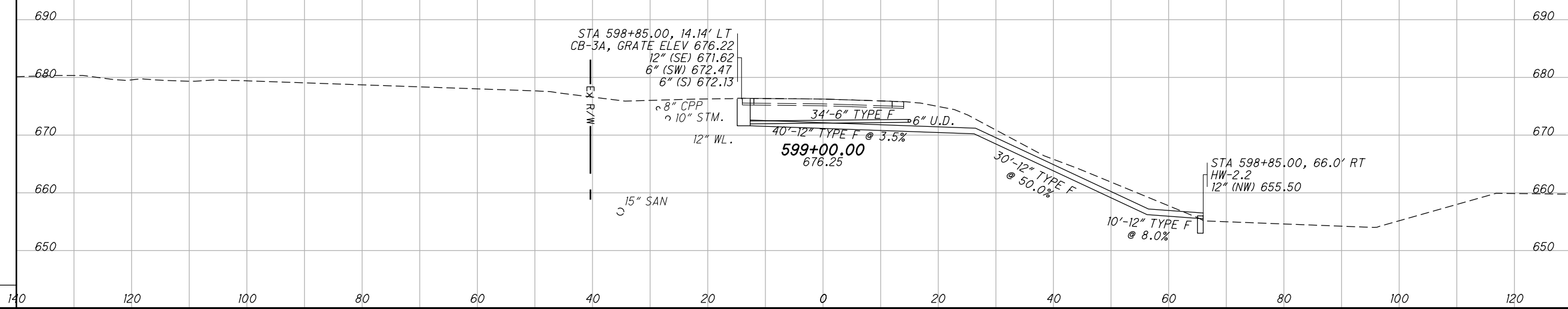
HEN-NEW MAUMEE RIVER BRIDGE
CROSS SECTIONS RIVERVIEW AVE.
STA. 597+00.00 TO STA. 598+50.00

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SEEDING

END WIDTH	SO. YDS.

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



CROSS SECTIONS RIVERVIEW AVE.
STA. 599+00.00

HEN-NEW MAUMEE RIVER BRIDGE

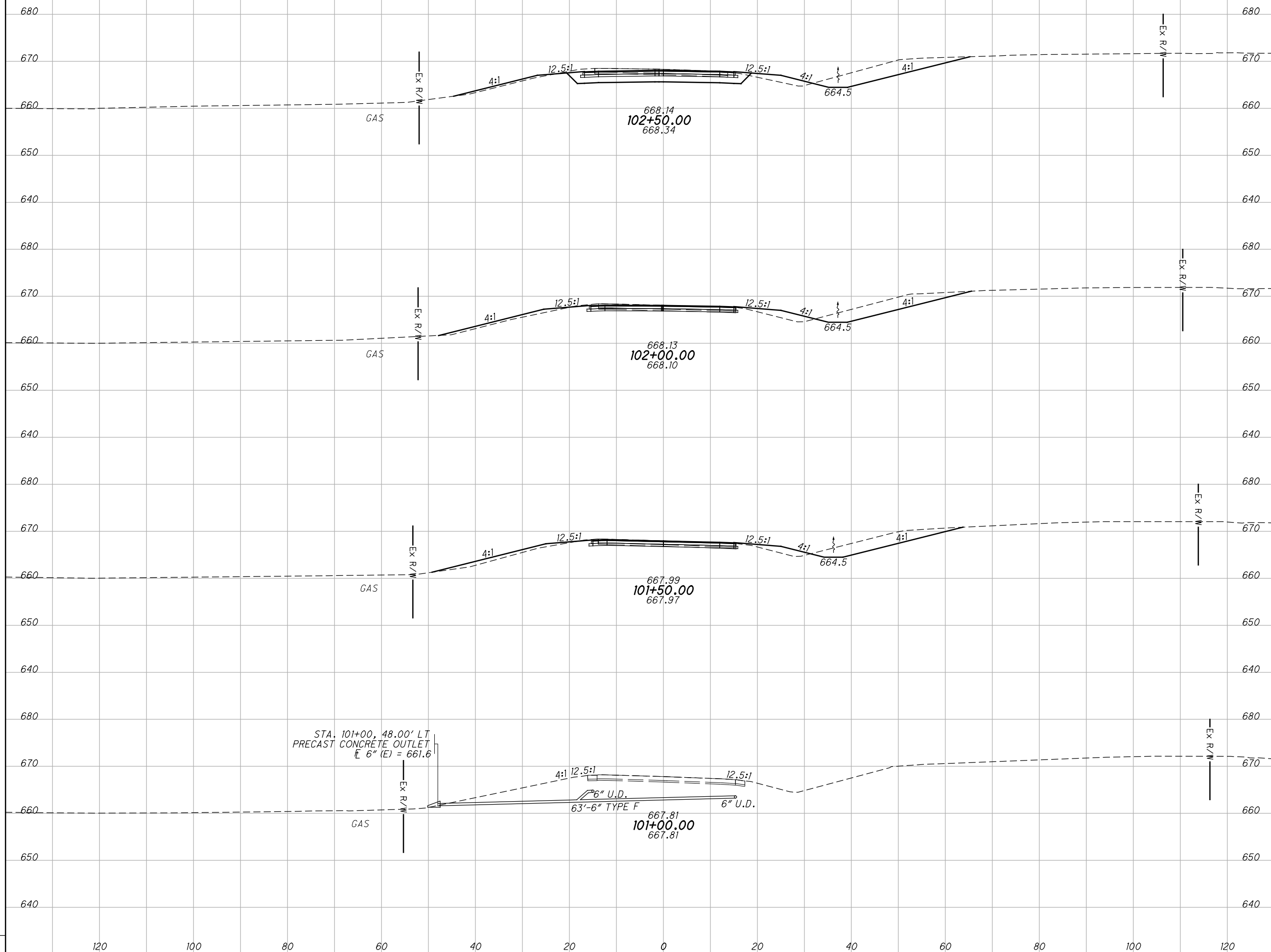
47
180

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SEEDING	
END WIDTH	SO. YDS.

ITEM 204 - EXCAVATION AND
ITEM 204 - GRANULAR MATERIAL, TYPE B

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



CROSS SECTIONS S.R. 110
STA. 101+00.00 TO STA. 102+50.00

HEN-NEW MAUMEE
RIVER BRIDGE

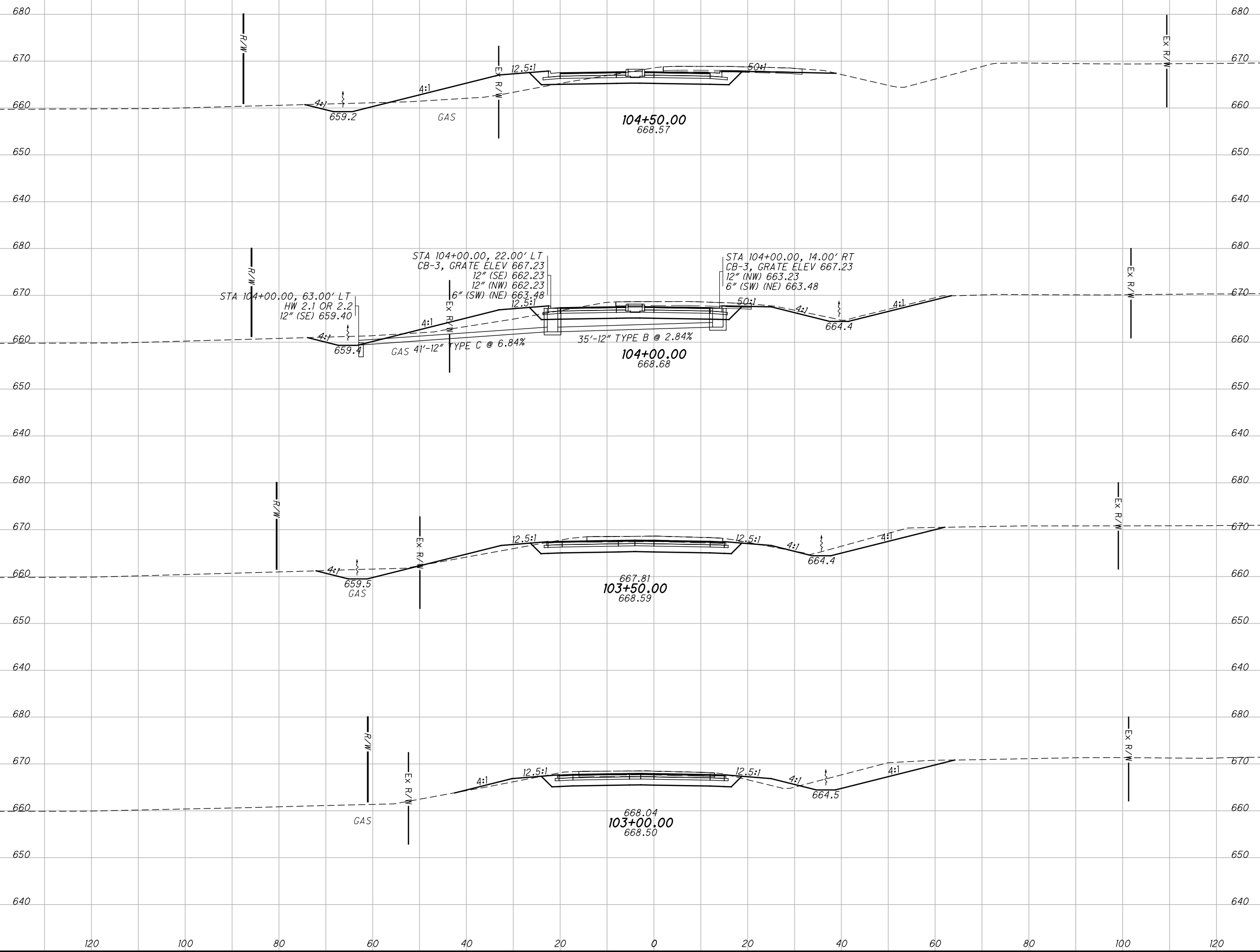
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SEEDING

END WIDTH	SO. YDS.

ITEM 204 - EXCAVATION AND
ITEM 204 - GRANULAR MATERIAL, TYPE B

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



CROSS SECTIONS S.R. 110
STA. 103+00.00 TO STA. 104+50.00

HEN-NEW MAUMEE
RIVER BRIDGE

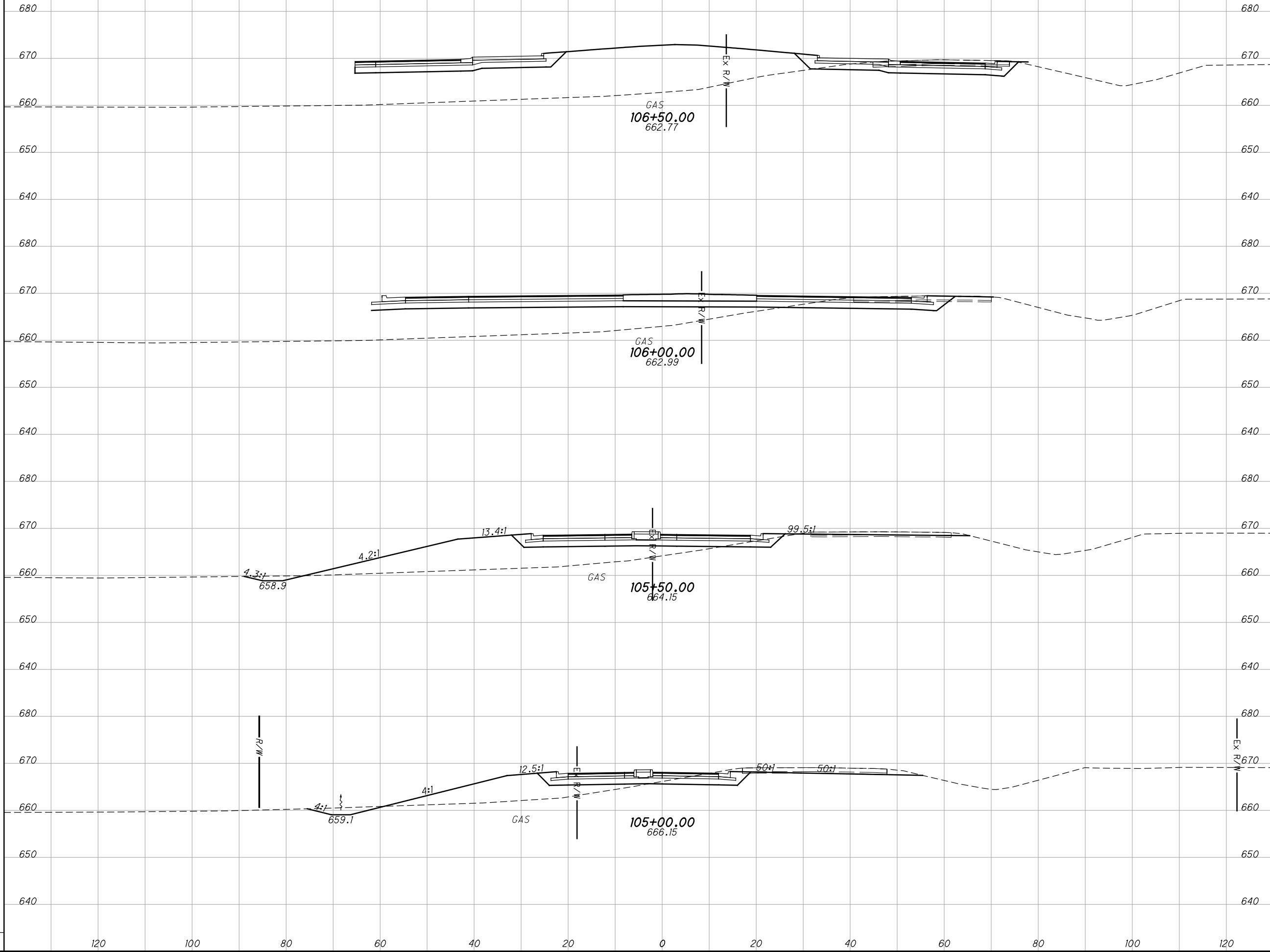
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SEEDING

END WIDTH	SO. YDS.
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ITEM 204 - EXCAVATION AND
ITEM 204 - GRANULAR MATERIAL, TYPE B

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

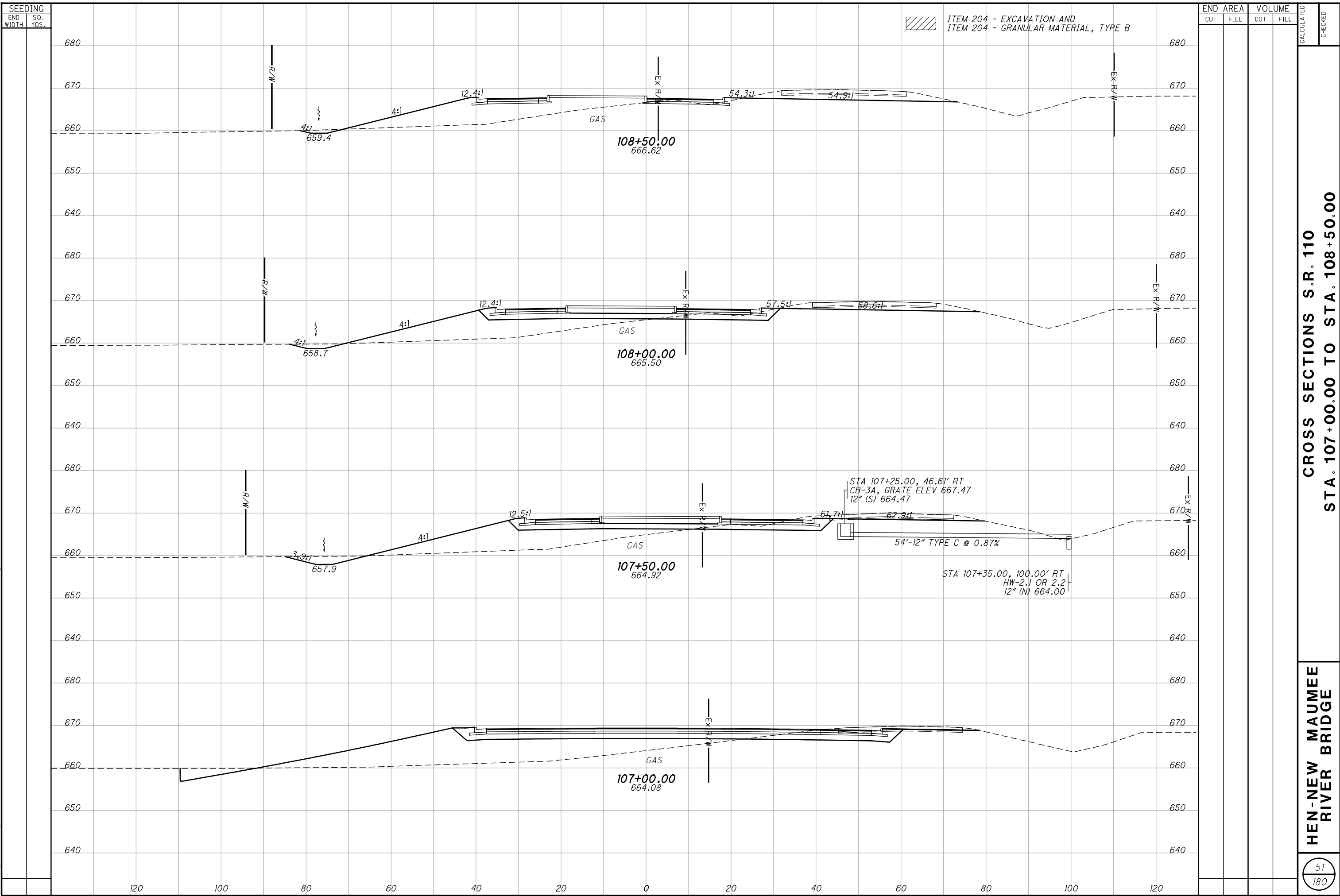


CROSS SECTIONS S.R. 110
STA. 105+00.00 TO STA. 106+50.00

HEN-NEW MAUMEE
RIVER BRIDGE

50
180

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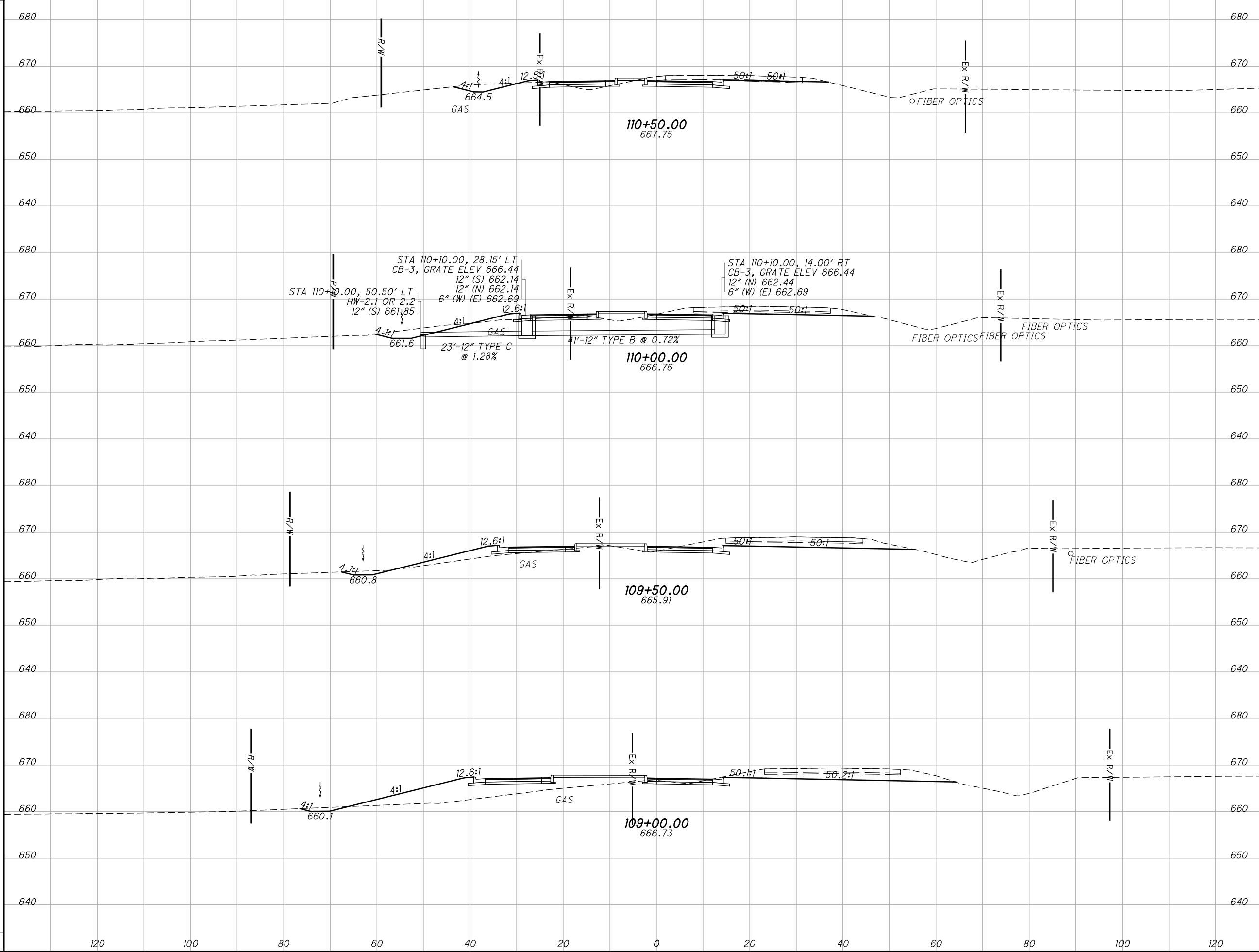
**CROSS SECTIONS S.R. 110
STA. 107+00.00 TO STA. 108+50.00**

**HEN-NEW MAUMEE
RIVER BRIDGE**

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SEEDING	
END WIDTH	SO. YDS.

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



CROSS SECTIONS S.R. 110
STA. 109+00.00 TO STA. 110+50.00

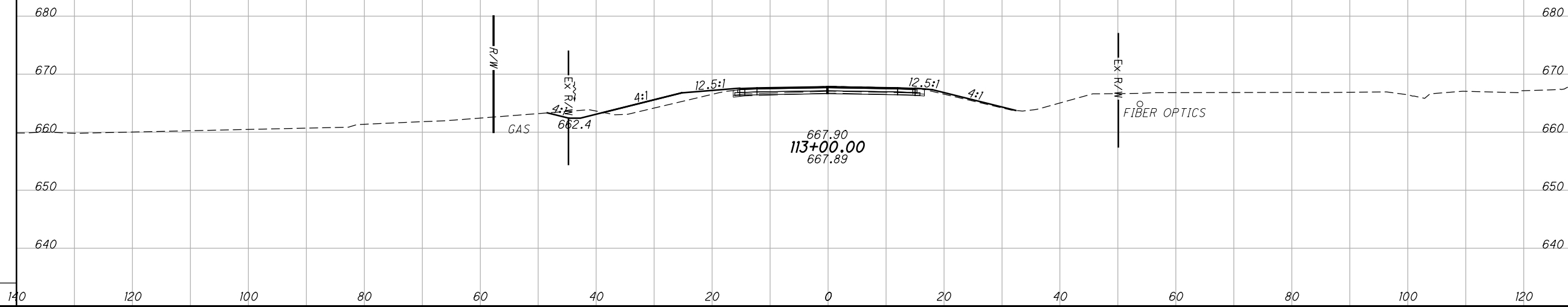
HEN-NEW MAUMEE
RIVER BRIDGE

52
180

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SEEDING	
END WIDTH	SO. YDS.

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



CROSS SECTIONS S.R. 110
STA. 113+00.00

HEN-NEW MAUMEE
RIVER BRIDGE

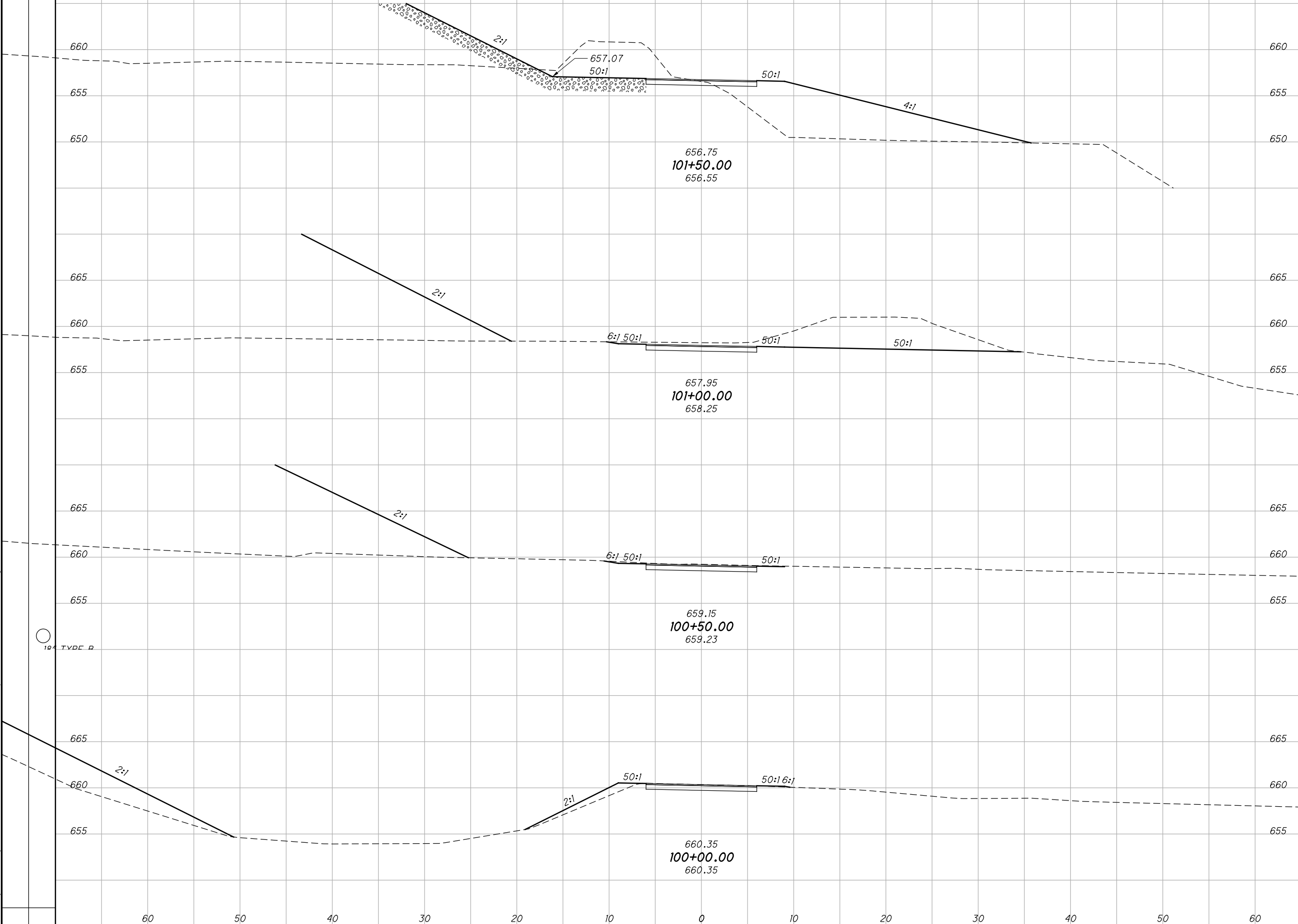
54
180

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SEEDING
END SO.
WIDTH YDS.

 TYPE C RCP 2.0' THICK WITH AGGREGATE FILTER

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



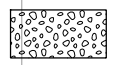
CROSS SECTIONS SHARED USE PATH
STA. 100+00.00 TO STA. 101+50.00

HEN-NEW MAUMEE
RIVER BRIDGE

56
180

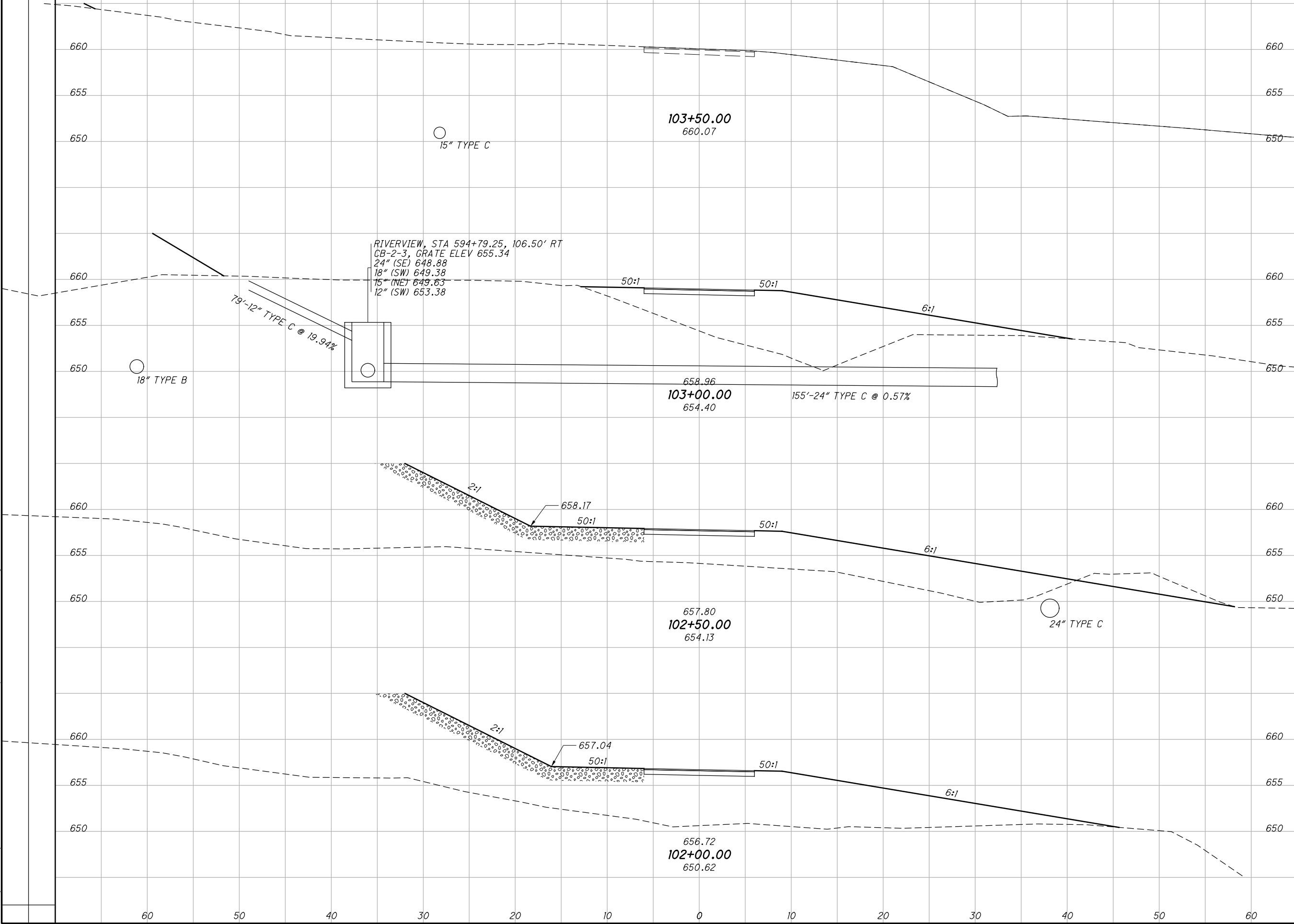
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SEEDING	
END WIDTH	SO. YDS.



TYPE C RCP 2.0' THICK WITH AGGREGATE FILTER

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



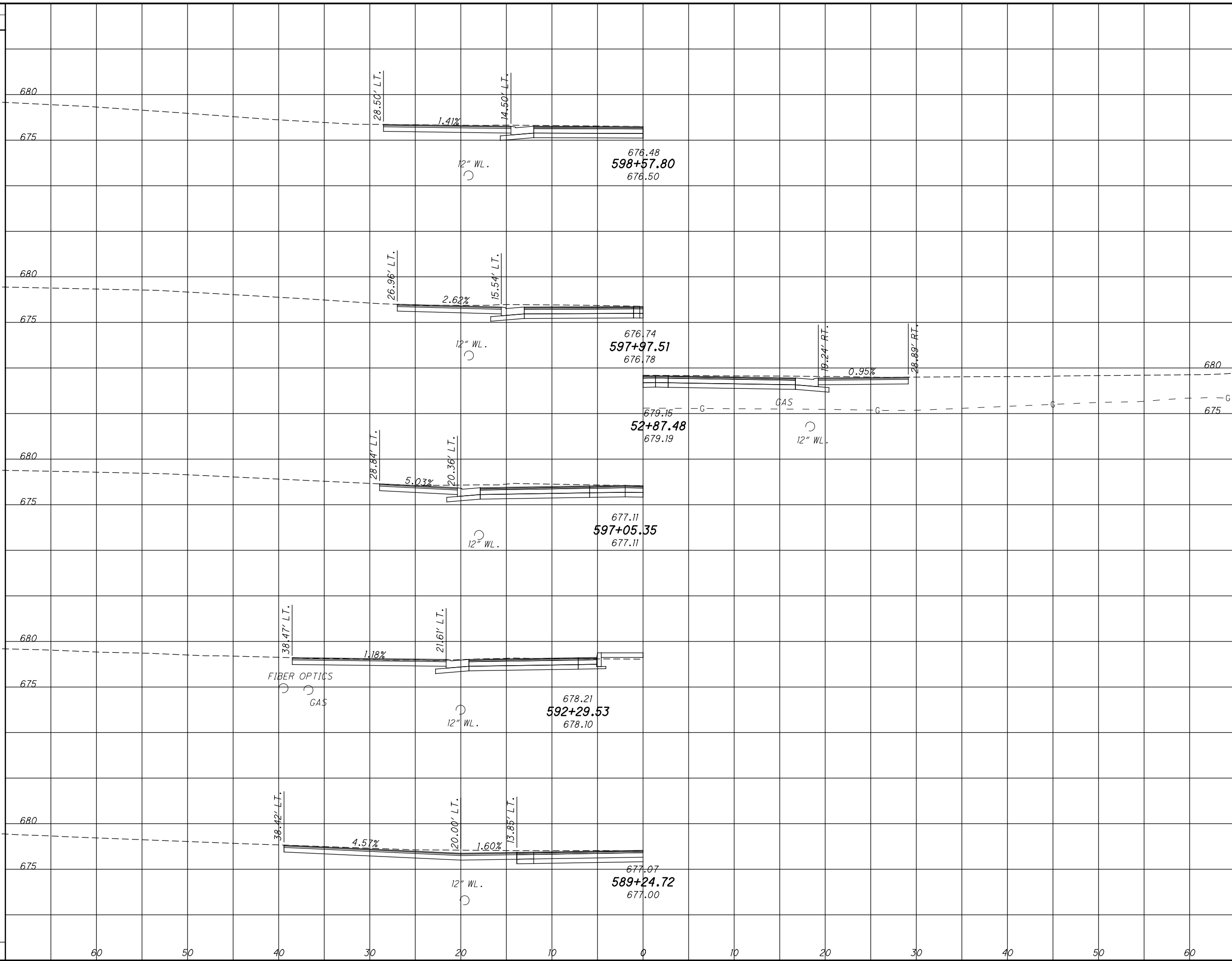
**CROSS SECTIONS SHARED USE PATH
 STA. 102+00.00 TO STA. 103+50.00**

**HEN-NEW MAUMEE
 RIVER BRIDGE**

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SEEDING
END SO.
WIDTH YDS.

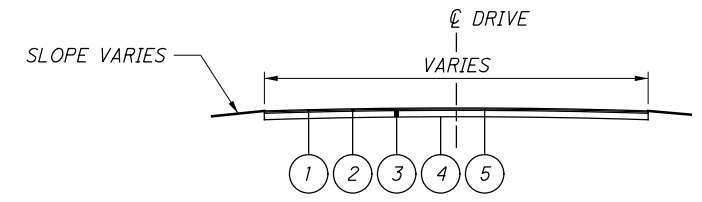
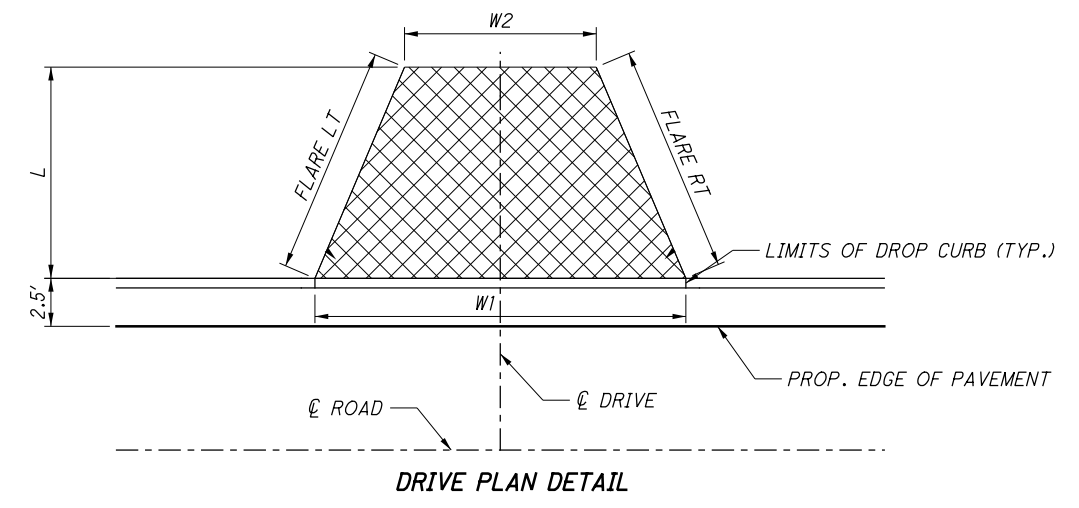
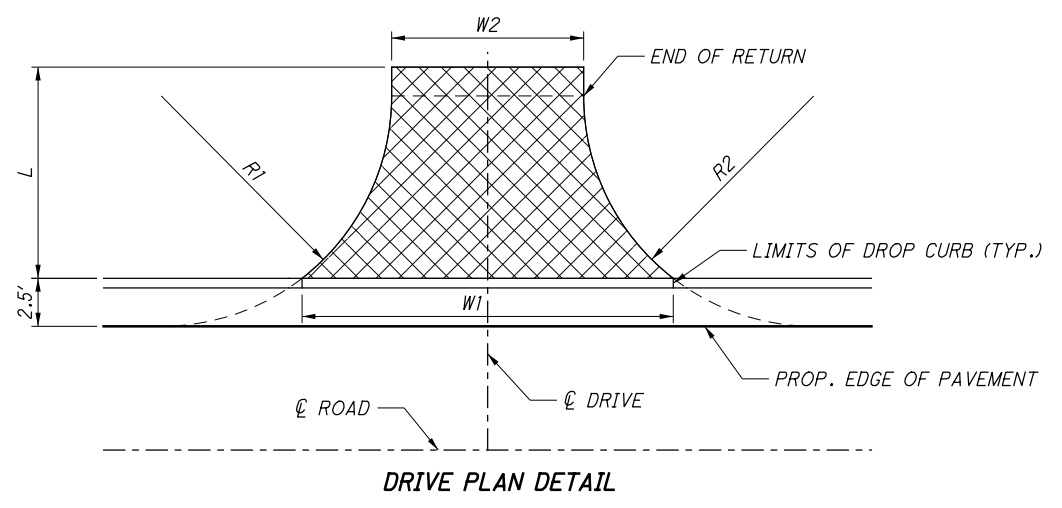
END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		



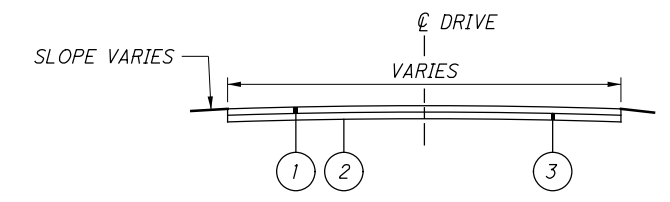
DRIVEWAY PROFILES
INDUSTRIAL DR.

HEN-NEW MAUMEE
RIVER BRIDGE

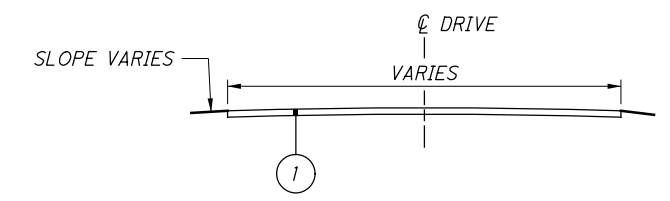
 MATCH EXISTING DRIVE MATERIAL



- ① ITEM 441 1 1/2" ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (448) (DRIVEWAYS)
- ② ITEM 441 1 3/4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448) (DRIVEWAYS)
- ③ ITEM 304 8" AGGREGATE BASE
- ④ ITEM 204, SUBGRADE COMPACTION
- ⑤ ITEM 690, SPECIAL-MISC.: TRACKLESS TACK COAT (0.04 GAL./SY. YD.)



- ① ITEM 452 8" NON-REINFORCED CONCRETE PAVEMENT
- ② ITEM 204 SUBGRADE COMPACTION
- ③ ITEM 304 6" AGGREGATE BASE



- ① ITEM 304 6" AGGREGATE BASE

SHEET NO.	STATION	SIDE	DRIVE TYPE	EXISTING DRIVE MATERIAL	DRIVE ANGLE	APRON LENGTH "L"	WIDTH "W1"	WIDTH "W2"	R1 (LEFT SIDE RADI) OF DRIVE LOOKING FROM CL)	R2 (RIGHT SIDE RADI) OF DRIVE LOOKING FROM CL)	FLARED RATE (LT)	FLARED RATE (RT)
RIVERVIEW AVE.												
GP201	589+24.72	LT	RES.	ASPH.	90.00	25.58	74.30	24.04	25.00	25.00		
GP202	592+29.53	LT	RES.	ASPH.	90.00	16.86	40.13	24.00	15.00	15.00		
GP203	597+05.35	LT	RES.	CONC.	91.86	8.50	28.65	20.00			9.47	9.86
GP203	597+97.51	LT	RES.	ASPH.	92.30	11.43	25.79	14.38			12.86	12.65
GP203	598+57.80	LT	RES.	CONC.	86.42	13.30	34.06	20.00			14.41	16.87
INDUSTRIAL DR.												
GP105	52+87.48	RT	RES.	ASPH.	78.36	9.65	26.73	15.00			9.95	11.98
S.R. 110												
GP301	101+35.00	LT	FIELD	GRVL	90.00	34.50	64.57	15.00	25.00	25.00		
GP303	111+36.05	RT	FIELD	GRVL	90.00	35.26	67.35	18.00	25.00	25.00		
GP303	113+10.00	LT	FIELD	GRVL	90.00	26.26	66.08	15.00	25.00	25.00		

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LINE TABLE		
LINE	LENGTH	BEARING
L1	53.71	N 61°05'54" E
L2	7.09	N 61°05'54" E
L3	62.27	N 35°44'47" W

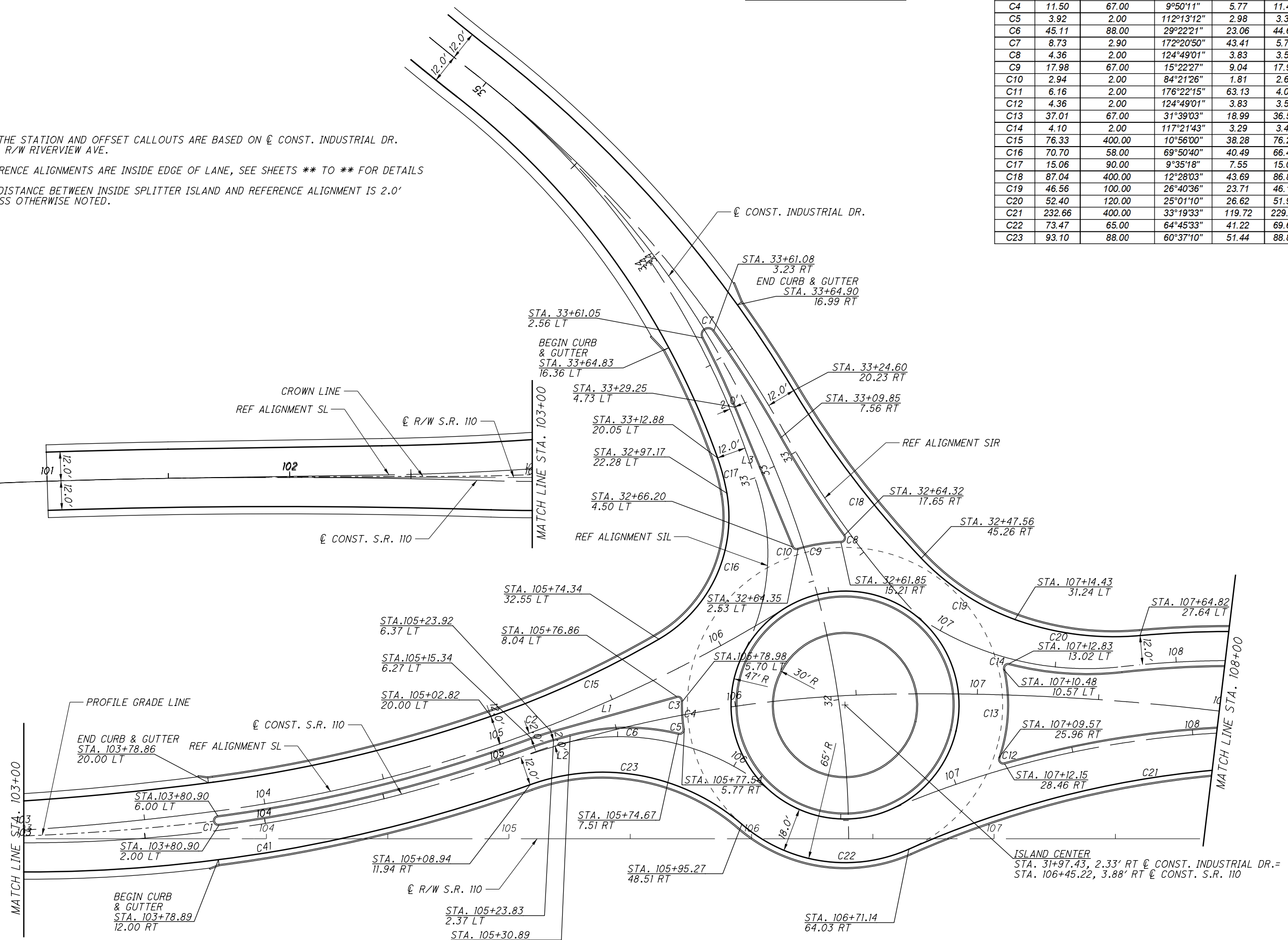
CURVE TABLE						
CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD	CHORD BRG
C1	6.28	2.00	180°0'00"	N/A	4.00	S 20°49'23" E
C2	8.69	104.00	4°47'9"	4.35	8.68	N 58°42'20" E
C3	3.73	2.00	106°59'20"	2.70	3.22	S 65°24'26" E
C4	11.50	67.00	9°50'11"	5.77	11.49	S 16°49'51" E
C5	3.92	2.00	112°13'12"	2.98	3.32	N 34°21'40" E
C6	45.11	88.00	29°22'21"	23.06	44.62	S 75°47'05" W
C7	8.73	2.90	172°20'50"	43.41	5.79	S 45°08'07" W
C8	4.36	2.00	124°49'01"	3.83	3.55	N 12°51'43" E
C9	17.98	67.00	15°22'27"	9.04	17.92	S 67°35'00" W
C10	2.94	2.00	84°21'26"	1.81	2.69	S 77°55'30" E
C11	6.16	2.00	176°22'15"	63.13	4.00	N 6°04'48" W
C12	4.36	2.00	124°49'01"	3.83	3.55	N 56°46'10" W
C13	37.01	67.00	31°39'03"	18.99	36.54	S 10°11'11" E
C14	4.10	2.00	117°21'43"	3.29	3.42	N 32°40'09" E
C15	76.33	400.00	10°56'00"	38.28	76.21	N 52°04'06" E
C16	70.70	58.00	69°50'40"	40.49	66.41	N 11°40'46" E
C17	15.06	90.00	9°35'18"	7.55	15.04	S 28°02'14" E
C18	87.04	400.00	12°28'03"	43.69	86.87	S 50°31'40" E
C19	46.56	100.00	26°40'36"	23.71	46.14	N 70°05'59" W
C20	52.40	120.00	25°01'10"	26.62	51.99	N 84°03'08" E
C21	232.66	400.00	33°19'33"	119.72	229.39	S 69°29'47" W
C22	73.47	65.00	64°45'33"	41.22	69.62	S 85°12'47" W
C23	93.10	88.00	60°37'10"	51.44	88.82	S 87°16'59" W



NOTE: ALL THE STATION AND OFFSET CALLOUTS ARE BASED ON \odot CONST. INDUSTRIAL DR. OR \odot R/W RIVERVIEW AVE.

REFERENCE ALIGNMENTS ARE INSIDE EDGE OF LANE, SEE SHEETS ** TO ** FOR DETAILS

THE DISTANCE BETWEEN INSIDE SPLITTER ISLAND AND REFERENCE ALIGNMENT IS 2.0' UNLESS OTHERWISE NOTED.



ROUNDABOUT GEOMETRIC DETAIL
INDUSTRIAL DR. & S.R. 110

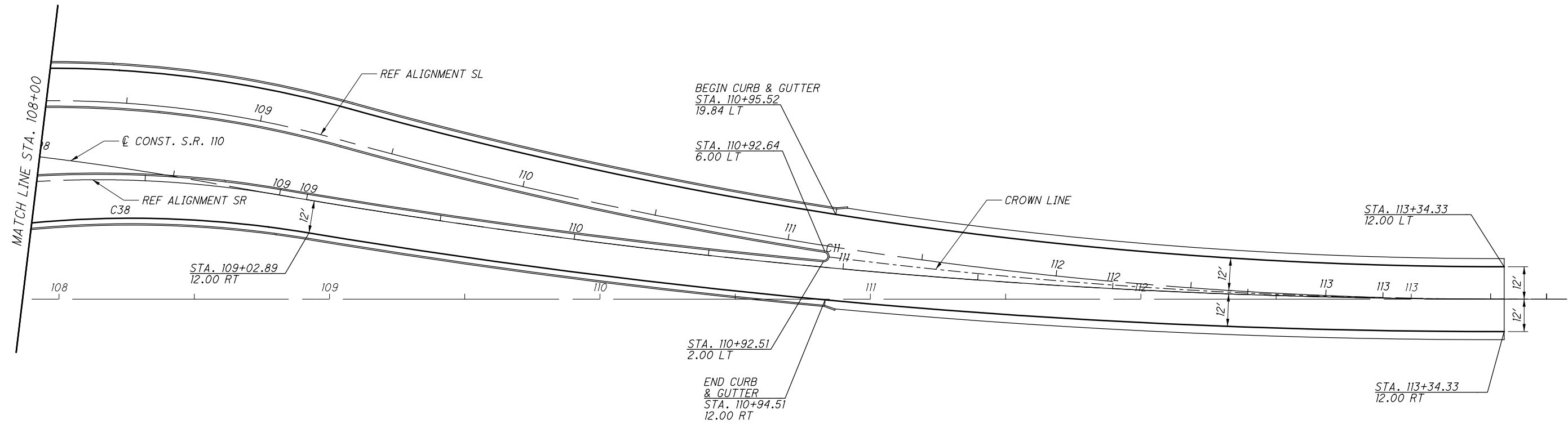
HEN-NEW MAUMEE
RIVER BRIDGE



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NOTE: ALL THE STATION AND OFFSET CALLOUTS ARE BASED ON \bar{C} CONST. S.R. 110.
REFERENCE ALIGNMENTS ARE INSIDE EDGE OF LANE, SEE SHEETS ** TO ** FOR DETAILS.
THE DISTANCE BETWEEN INSIDE SPLITTER ISLAND AND REFERENCE ALIGNMENT IS 2.0'
UNLESS OTHERWISE NOTED.
SEE SHEET ** FOR CURVE AND LINE DATA.



CALCULATED
CHECKED

0 20 40
HORIZONTAL
SCALE IN FEET

ROUNDABOUT GEOMETRIC DETAIL
INDUSTRIAL DR. & S.R. 110

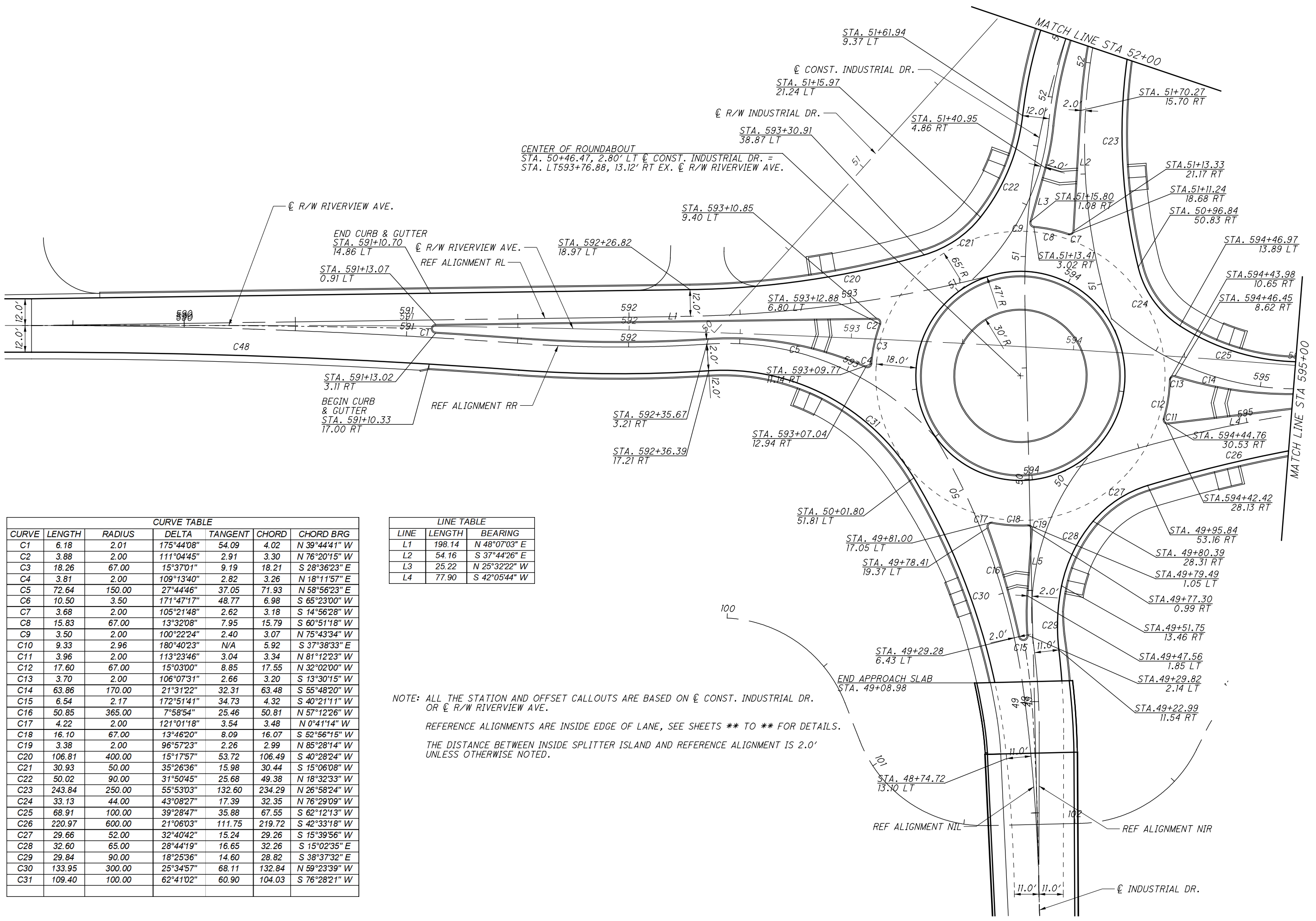
**HEN-NEW MAUMEE
RIVER BRIDGE**



CALCULATED
CHECKED

**ROUNDABOUT GEOMETRIC DETAIL
INDUSTRIAL DR. & RIVERVIEW AVE.**

**HEN-NEW MAUMEE
RIVER BRIDGE**



CURVE TABLE

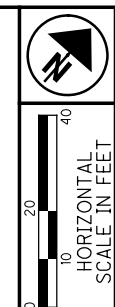
CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD	CHORD BRG
C1	6.18	2.01	175°44'08"	54.09	4.02	N 39°44'41" W
C2	3.88	2.00	111°04'45"	2.91	3.30	N 76°20'15" W
C3	18.26	67.00	15°37'01"	9.19	18.21	S 28°36'23" E
C4	3.81	2.00	109°13'40"	2.82	3.26	N 18°11'57" E
C5	72.64	150.00	27°44'46"	37.05	71.93	N 58°56'23" E
C6	10.50	3.50	171°47'17"	48.77	6.98	S 65°23'00" W
C7	3.68	2.00	105°21'48"	2.62	3.18	S 14°56'28" W
C8	15.83	67.00	13°32'08"	7.95	15.79	S 60°51'18" W
C9	3.50	2.00	100°22'24"	2.40	3.07	N 75°43'34" W
C10	9.33	2.96	180°40'23"	N/A	5.92	S 37°38'33" E
C11	3.96	2.00	113°23'46"	3.04	3.34	N 81°12'23" W
C12	17.60	67.00	15°03'00"	8.85	17.55	N 32°02'00" W
C13	3.70	2.00	106°07'31"	2.66	3.20	S 13°30'15" W
C14	63.86	170.00	21°31'22"	32.31	63.48	S 55°48'20" W
C15	6.54	2.17	172°51'41"	34.73	4.32	S 40°21'11" W
C16	50.85	365.00	7°58'54"	25.46	50.81	N 57°12'26" W
C17	4.22	2.00	121°01'18"	3.54	3.48	N 0°41'14" W
C18	16.10	67.00	13°46'20"	8.09	16.07	S 52°56'15" W
C19	3.38	2.00	96°57'23"	2.26	2.99	N 85°28'14" W
C20	106.81	400.00	15°17'57"	53.72	106.49	S 40°28'24" W
C21	30.93	50.00	35°26'36"	15.98	30.44	S 15°06'08" W
C22	50.02	90.00	31°50'45"	25.68	49.38	N 18°32'33" W
C23	243.84	250.00	55°53'03"	132.60	234.29	N 26°58'24" W
C24	33.13	44.00	43°08'27"	17.39	32.35	N 76°29'09" W
C25	68.91	100.00	39°28'47"	35.88	67.55	S 62°12'13" W
C26	220.97	600.00	21°06'03"	111.75	219.72	S 42°33'18" W
C27	29.66	52.00	32°40'42"	15.24	29.26	S 15°39'56" W
C28	32.60	65.00	28°44'19"	16.65	32.26	S 15°02'35" E
C29	29.84	90.00	18°25'36"	14.60	28.82	S 38°37'32" E
C30	133.95	300.00	25°34'57"	68.11	132.84	N 59°23'39" W
C31	109.40	100.00	62°41'02"	60.90	104.03	S 76°28'21" W

LINE TABLE

LINE	LENGTH	BEARING
L1	198.14	N 48°07'03" E
L2	54.16	S 37°44'26" E
L3	25.22	N 25°32'22" W
L4	77.90	S 42°05'44" W

NOTE: ALL THE STATION AND OFFSET CALLOUTS ARE BASED ON Q CONST. INDUSTRIAL DR. OR Q R/W RIVERVIEW AVE.
 REFERENCE ALIGNMENTS ARE INSIDE EDGE OF LANE, SEE SHEETS ** TO ** FOR DETAILS.
 THE DISTANCE BETWEEN INSIDE SPLITTER ISLAND AND REFERENCE ALIGNMENT IS 2.0' UNLESS OTHERWISE NOTED.

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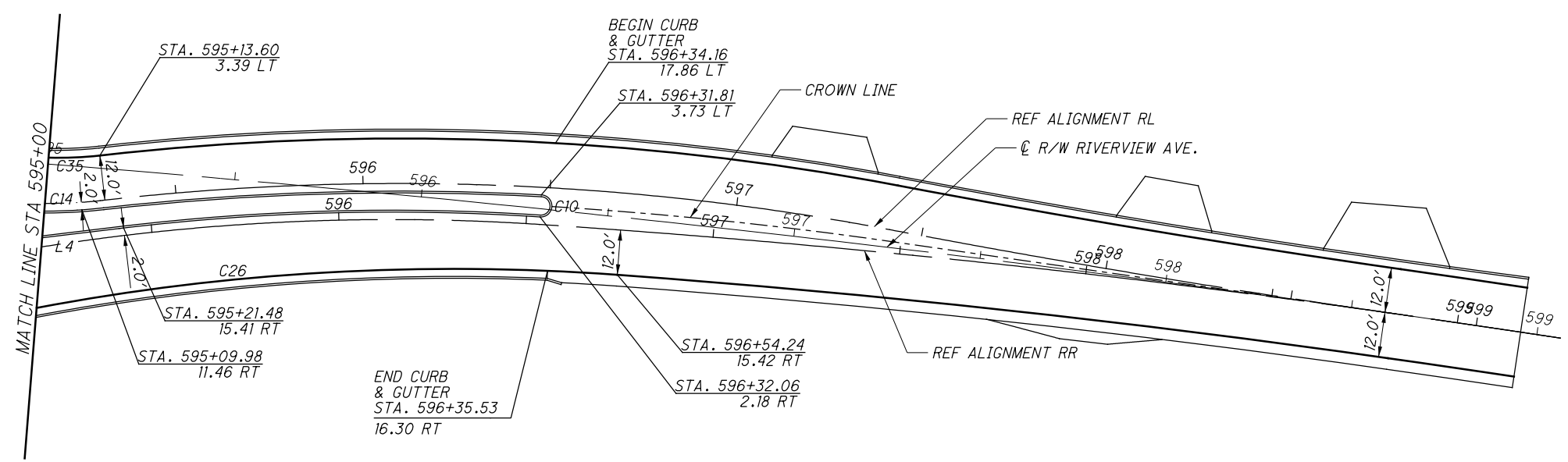
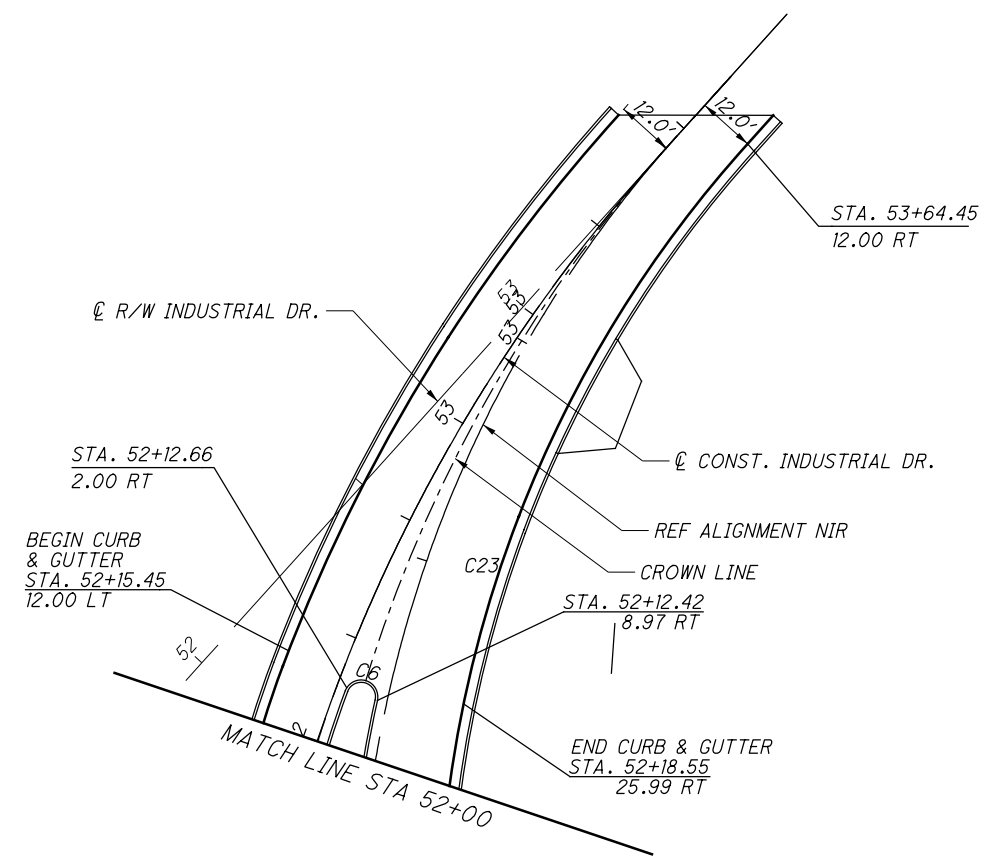


CALCULATED
CHECKED

**ROUNDABOUT GEOMETRIC DETAIL
INDUSTRIAL DR. & RIVERVIEW AVE.**

**HEN-NEW MAUMEE
RIVER BRIDGE**

NOTE: ALL THE STATION AND OFFSET CALLOUTS ARE BASED ON \varnothing CONST. INDUSTRIAL DR.
OR \varnothing R/W RIVERVIEW AVE.
REFERENCE ALIGNMENTS ARE INSIDE EDGE OF LANE, SEE SHEETS ** TO ** FOR DETAILS.
THE DISTANCE BETWEEN INSIDE SPLITTER ISLAND AND REFERENCE ALIGNMENT IS 2.0'
UNLESS OTHERWISE NOTED.
SEE SHEET ** FOR CURVE AND LINE DATA.



NOTE: SEE SHEET XX FOR CURB AND LINE DATA

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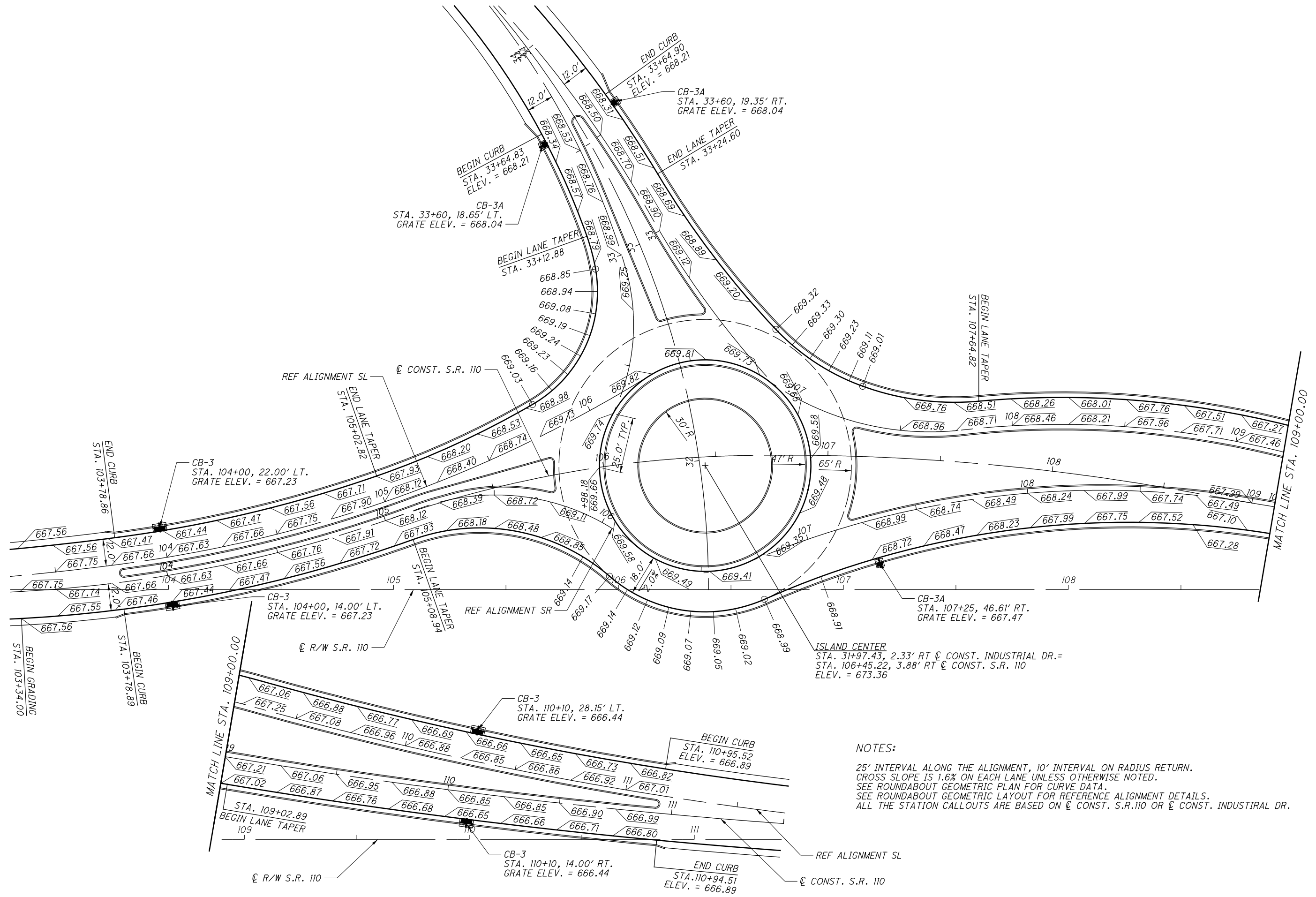


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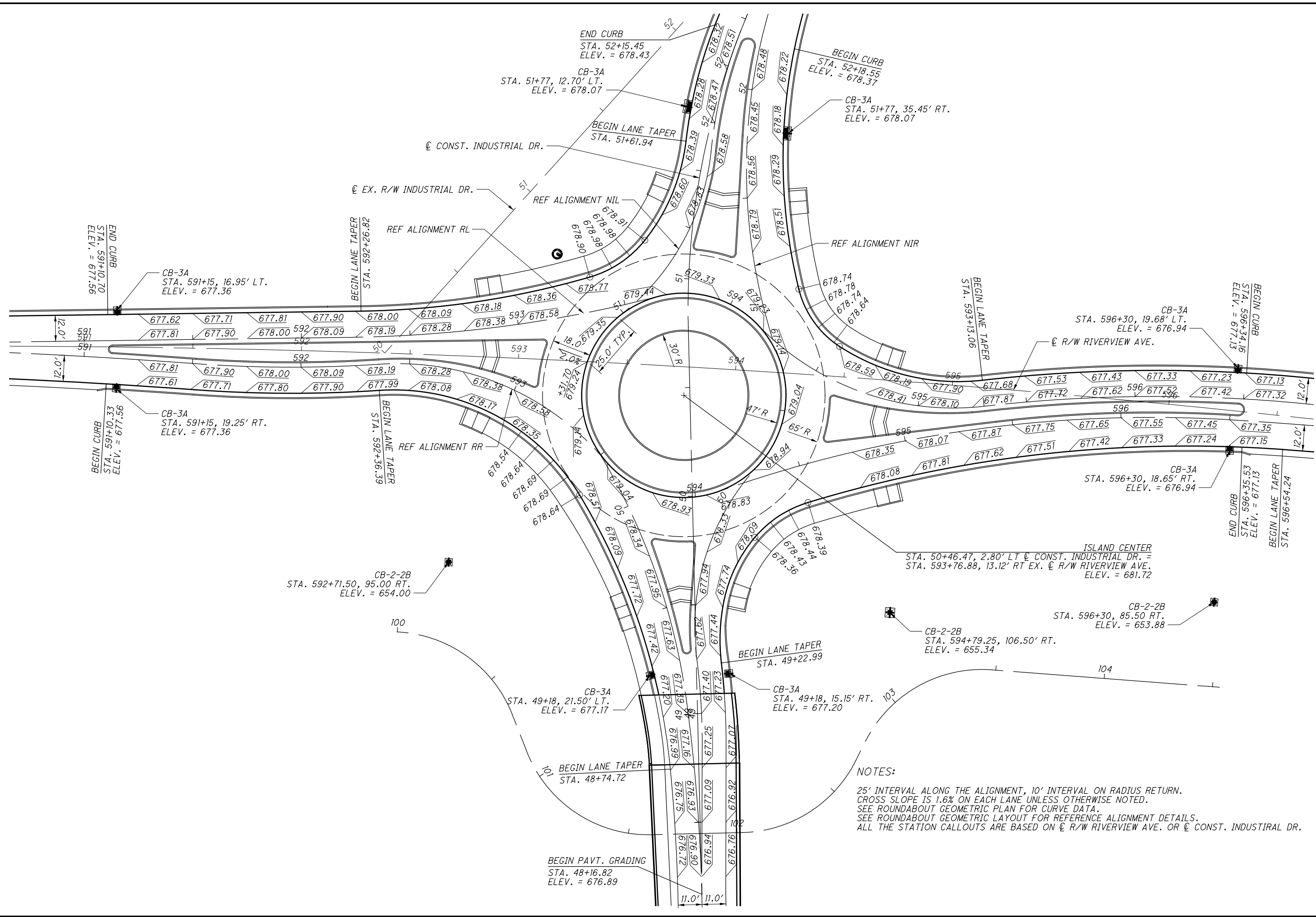
ROUNDABOUT GRADING DETAIL INDUSTRIAL DR. & S.R. 110

HEN-NEW MAUMEE RIVER BRIDGE

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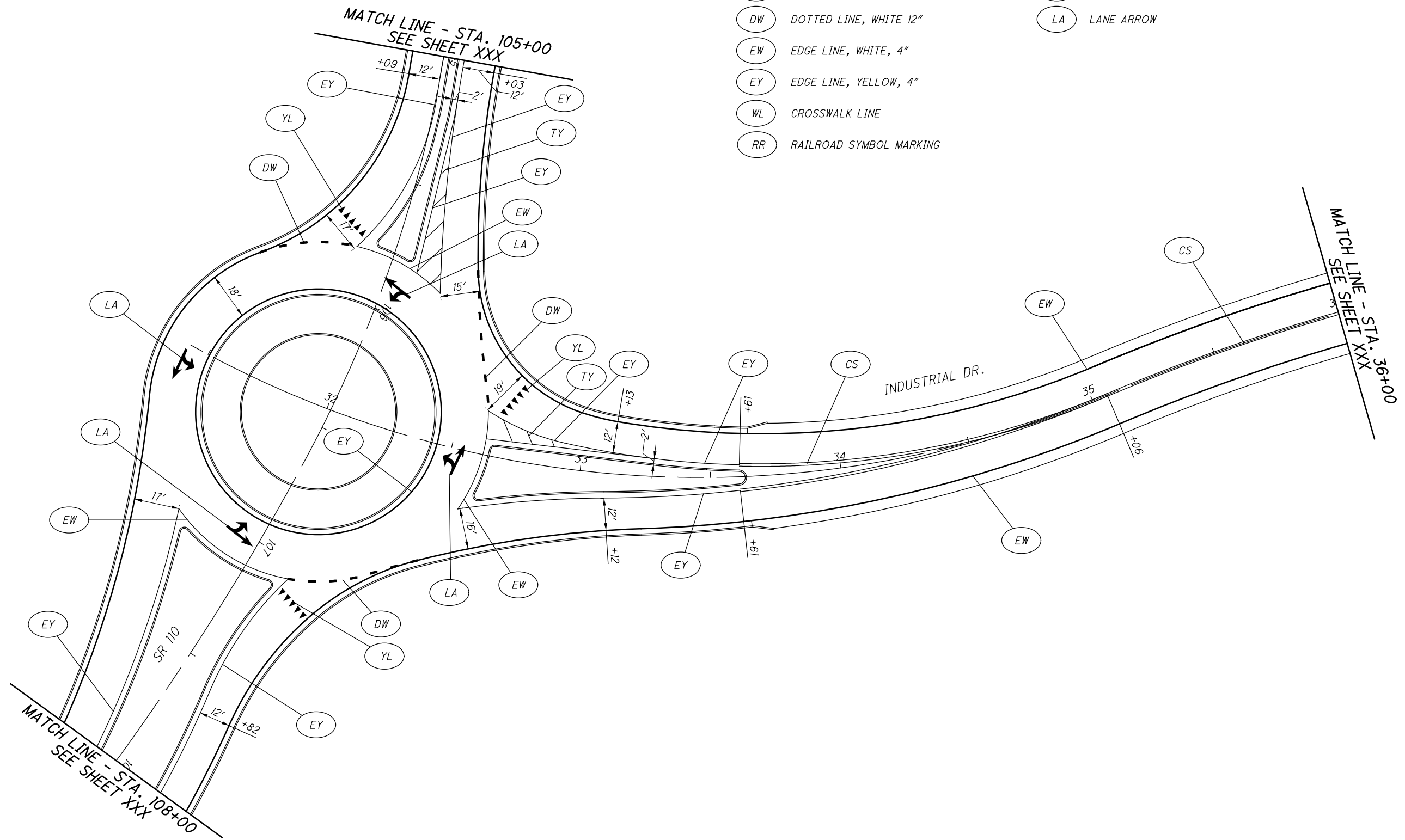


NOTES:
 25' INTERVAL ALONG THE ALIGNMENT, 10' INTERVAL ON RADIUS RETURN.
 CROSS SLOPE IS 1.6% ON EACH LANE UNLESS OTHERWISE NOTED.
 SEE ROUNDABOUT GEOMETRIC PLAN FOR CURVE DATA.
 SEE ROUNDABOUT GEOMETRIC LAYOUT FOR REFERENCE ALIGNMENT DETAILS.
 ALL THE STATION CALLOUTS ARE BASED ON @ CONST. S.R.110 OR @ CONST. INDUSTRIAL DR.



NOTES:
 25' INTERVAL ALONG THE ALIGNMENT, 10' INTERVAL ON RADIUS RETURN.
 CROSS SLOPE IS 1.6% ON EACH LANE UNLESS OTHERWISE NOTED.
 SEE ROUNDABOUT GEOMETRIC PLAN FOR CURVE DATA.
 SEE ROUNDABOUT GEOMETRIC LAYOUT FOR REFERENCE ALIGNMENT DETAILS.
 ALL THE STATION CALLOUTS ARE BASED ON CL R/W RIVERVIEW AVE. OR CL CONST. INDUSTRIAL DR.

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LEGEND

- CS CENTER LINE, DOUBLE SOLID
- CP CENTER LINE, SOLID/DASHED
- DW DOTTED LINE, WHITE 12"
- EW EDGE LINE, WHITE, 4"
- EY EDGE LINE, YELLOW, 4"
- WL CROSSWALK LINE
- RR RAILROAD SYMBOL MARKING
- TY TRANSVERSE/DIAGONAL LINE, YELLOW
- YL YIELD LINE (24" X 36' TRIANGLES)
- LA LANE ARROW

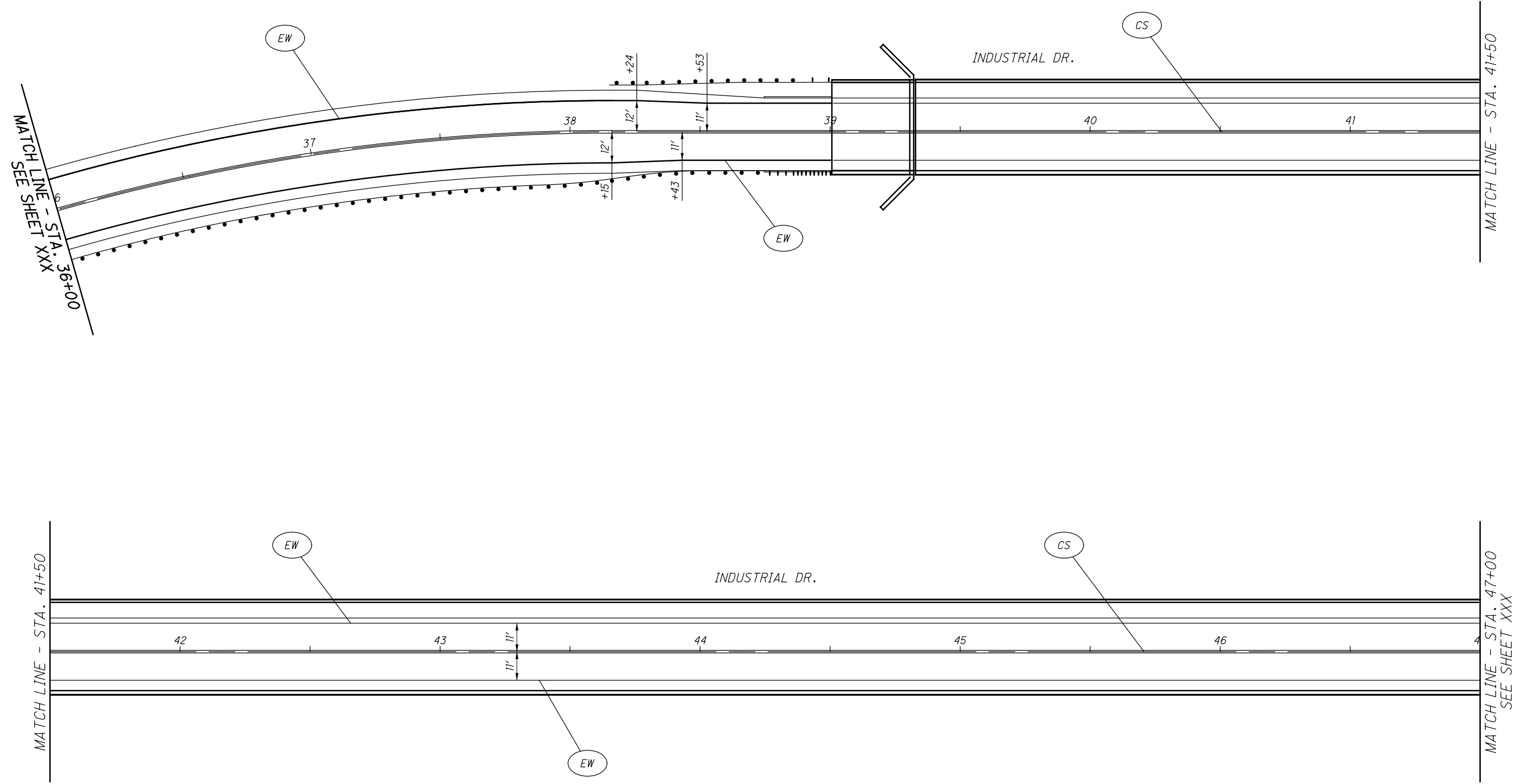
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10 HORIZONTAL SCALE IN FEET

PAVEMENT MARKING PLAN
ROUNDABOUT

HEN-NEW MAUMEE
RIVER BRIDGE

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FOR LEGEND, SEE SHEET XX.



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CHECKED	_____

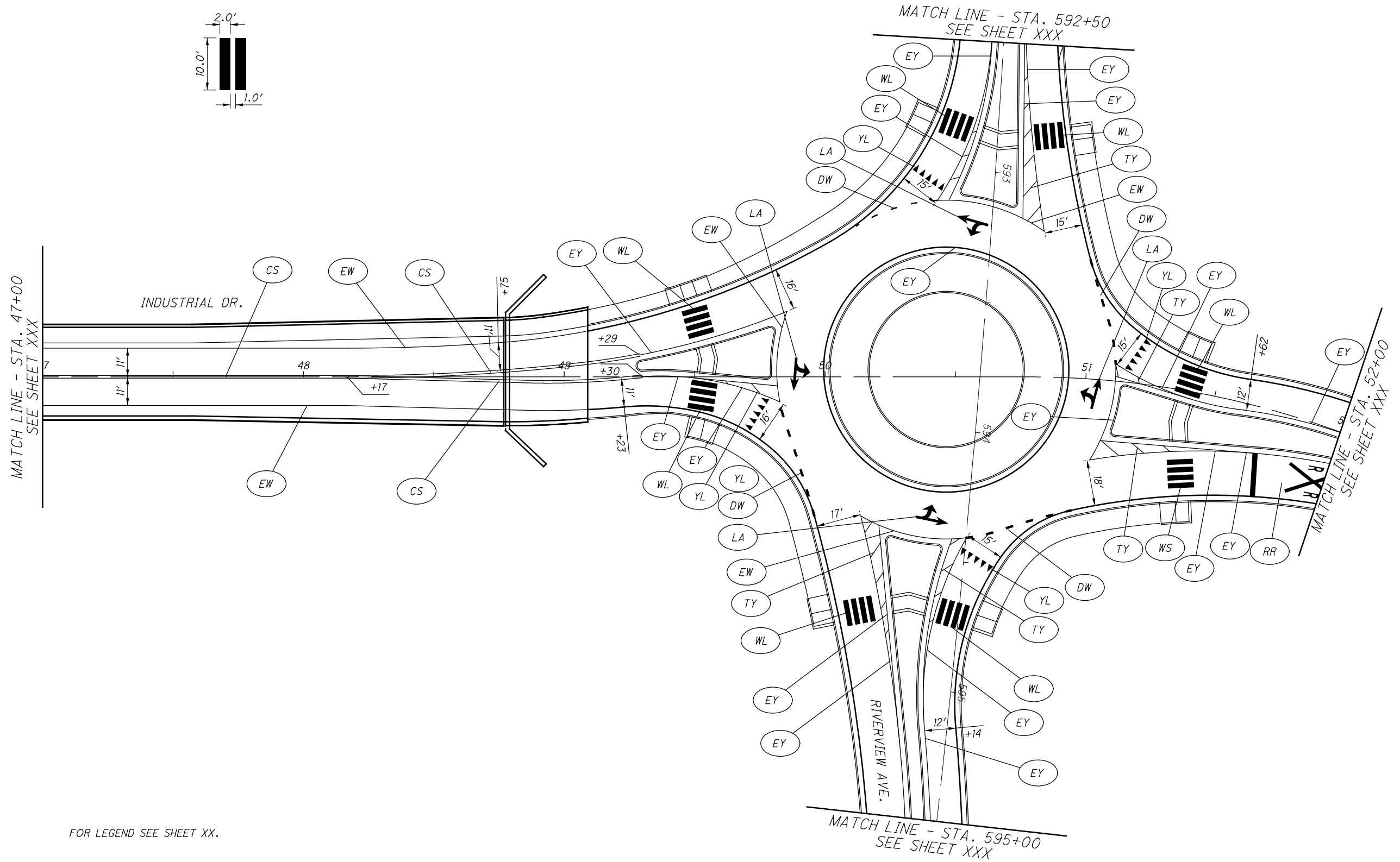
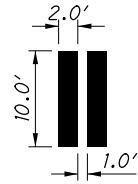
SIGNING AND PAVEMENT MARKING PLAN
INDUSTRIAL DR.

HEN-NEW MAUMEE
RIVER BRIDGE

NOTE:

ITEM 644 - CROSSWALK LINE

24" WHITE LONGITUDIAL LINES SHALL BE PLACED PARALLEL TO TRAFFIC FLOW AS SHOWN IN FOLLOWING DETAIL. THE MARKING DESIGN SHOULD AVOID THE WHEEL PATHS. THE METHOD OF MEASUREMENT SHALL BE PARALLEL TO TRAFFIC FLOW ALONG THE 24" LONGITUDINAL LINES.



FOR LEGEND SEE SHEET XX.



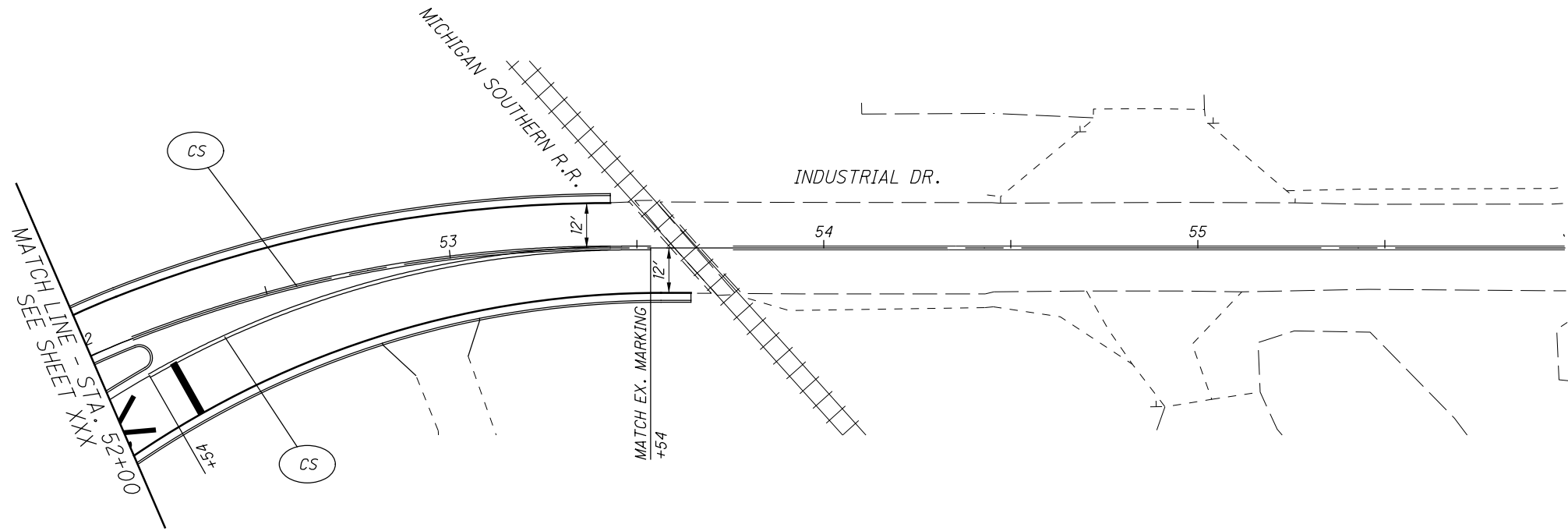
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CALCULATED
CHECKED

PAVEMENT MARKING PLAN
ROUNDBOUT

HEN-INDUSTRIAL DR.

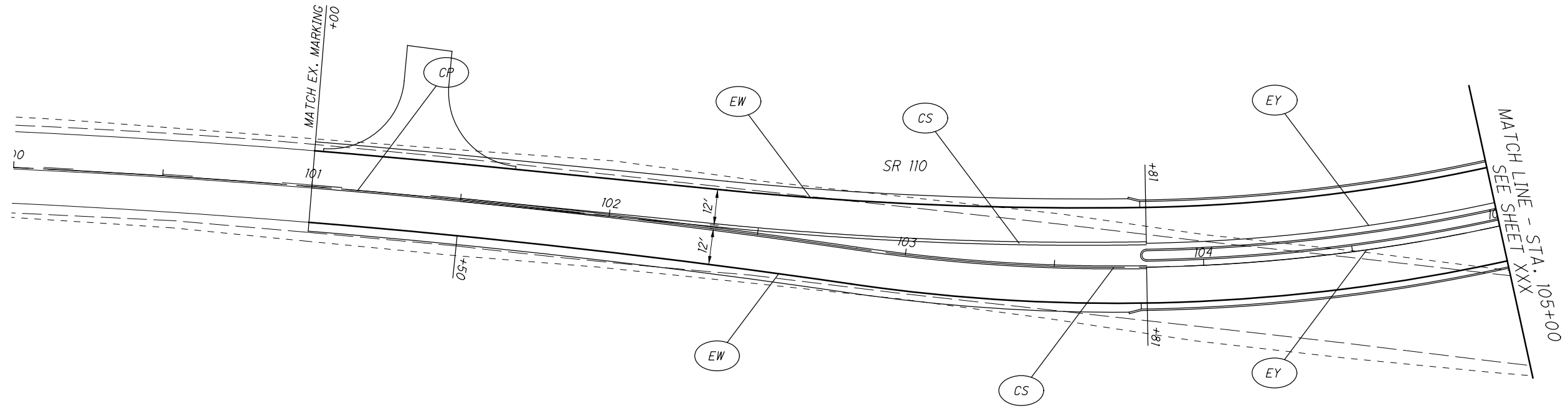
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SIGNING AND PAVEMENT MARKING PLAN
INDUSTRIAL DR.

HEN-NEW MAUMEE RIVER BRIDGE



FOR LEGEND, SEE SHEET XX.



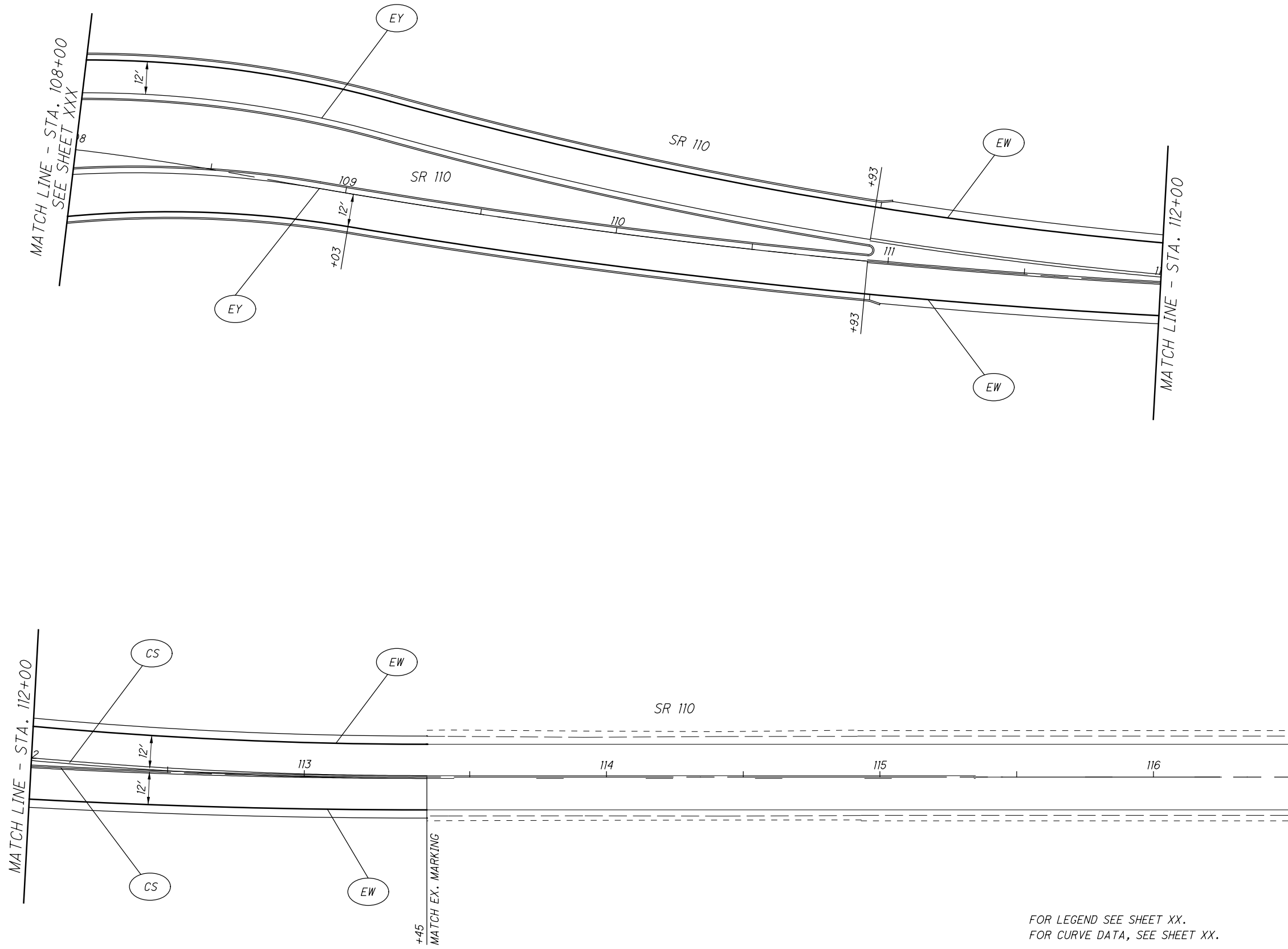
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SIGNING AND PAVEMENT MARKING PLAN
SR 110

HEN-NEW MAUMEE
RIVER BRIDGE

70
180

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FOR LEGEND SEE SHEET XX.
FOR CURVE DATA, SEE SHEET XX.

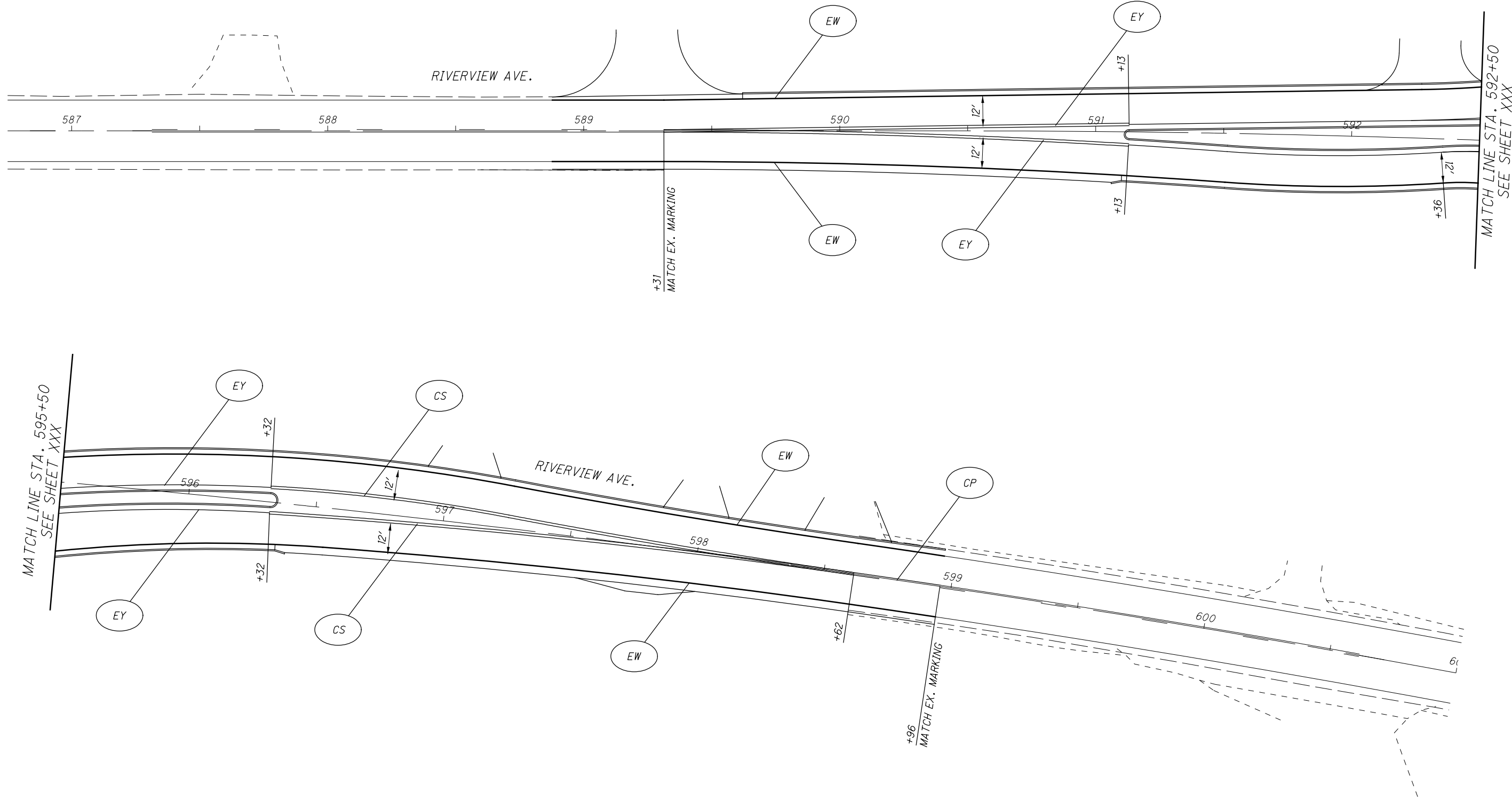
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CHECKED _____

0 20 40
10
HORIZONTAL
SCALE IN FEET

SIGNING AND PAVEMENT MARKING PLAN
SR 110

**HEN-NEW MAUMEE
RIVER BRIDGE**

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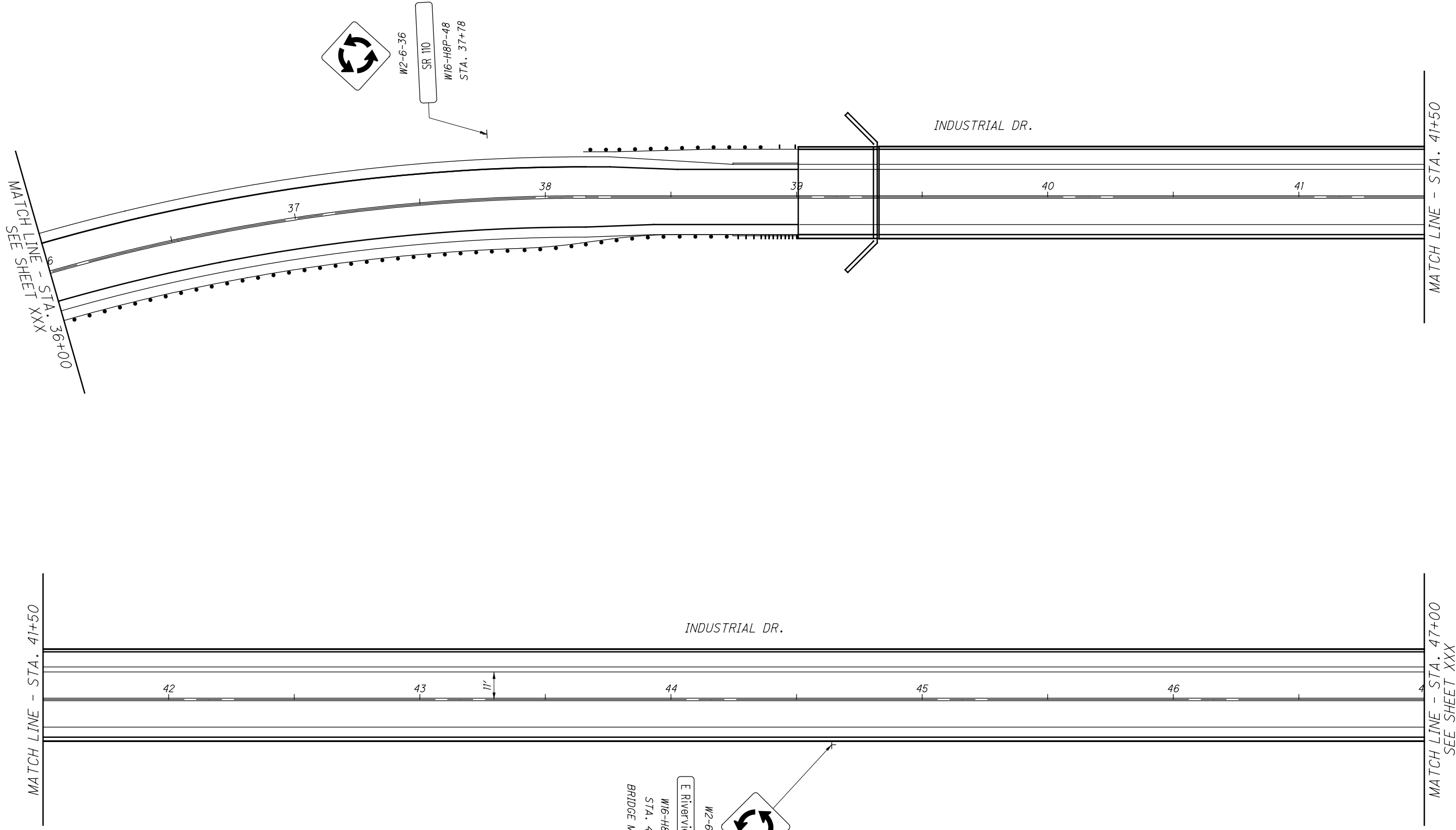
FOR LEGEND SEE SHEET XX.



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SIGNING AND PAVEMENT MARKING PLAN
RIVERVIEW AVE.

HEN-NEW MAUMEE
RIVER BRIDGE



MATCH LINE - STA. 36+00
SEE SHEET XXX

MATCH LINE - STA. 41+50

MATCH LINE - STA. 47+00
SEE SHEET XXX

MATCH LINE - STA. 41+50



W2-6-36
SR 110
W16-H8P-48
STA. 37+78



W2-6-36
E Riverview Ave
W16-H8P-48
STA. 44+77
BRIDGE MOUNTED

INDUSTRIAL DR.

INDUSTRIAL DR.

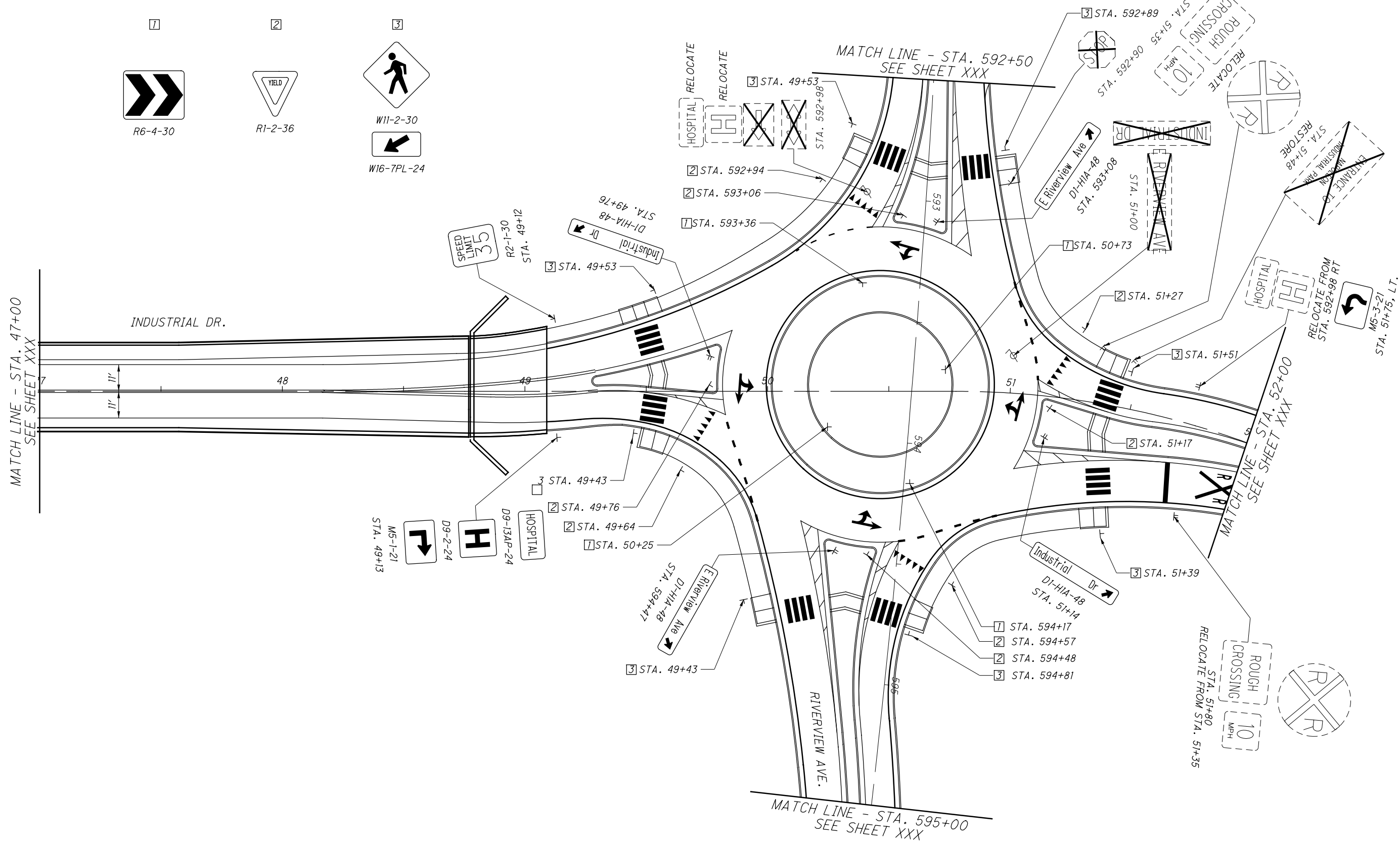
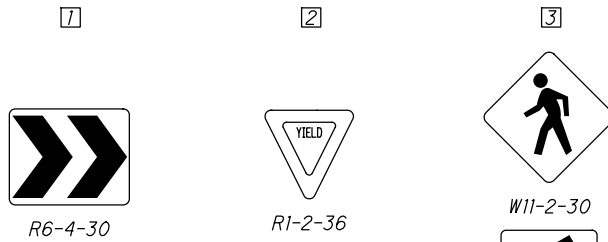


CALCULATED
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SIGNING AND PAVEMENT MARKING PLAN
INDUSTRIAL DR.

HEN-NEW MAUMEE RIVER BRIDGE

MATCH LINE - STA. 47+00
SEE SHEET XXX



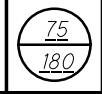
FOR LEGEND SEE SHEET

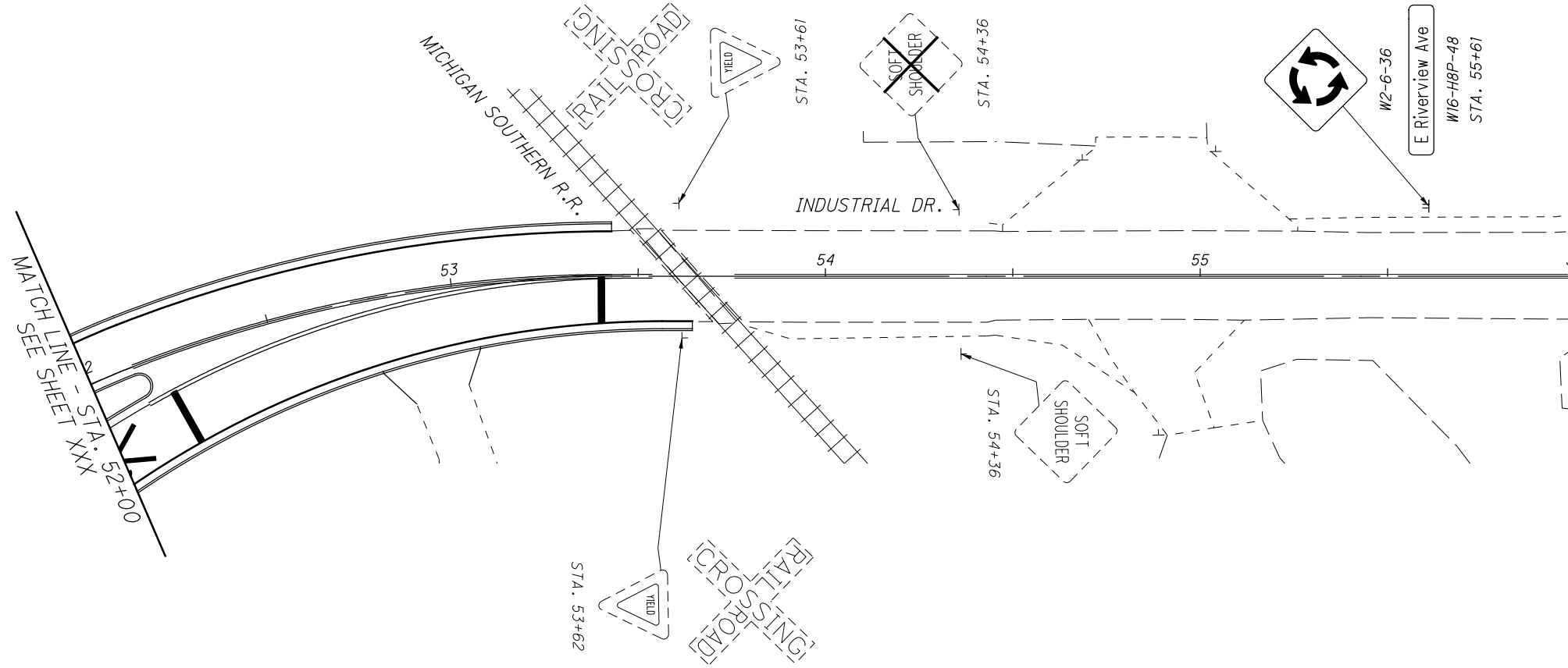


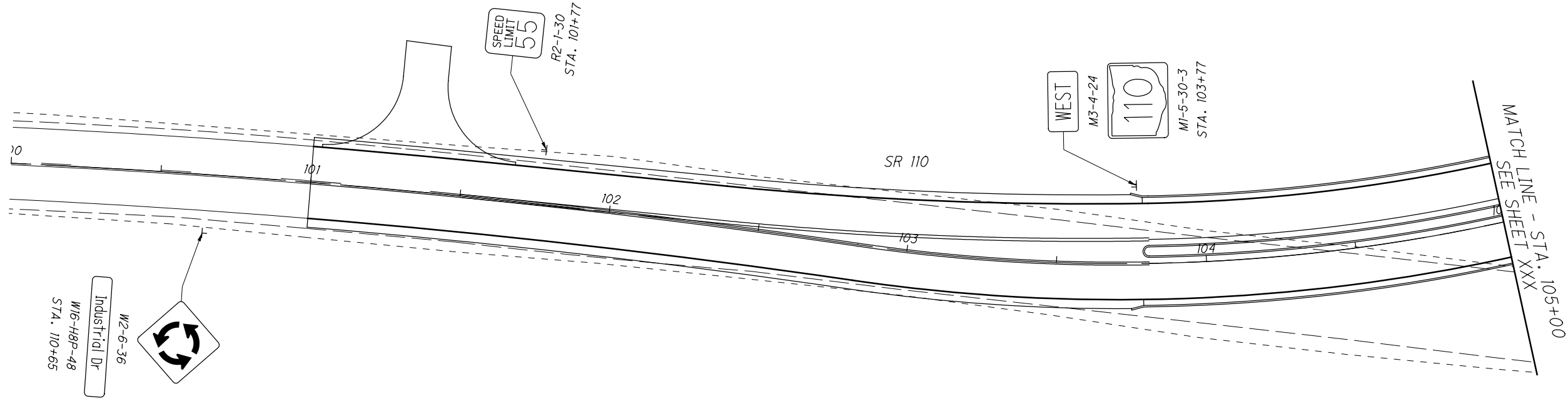
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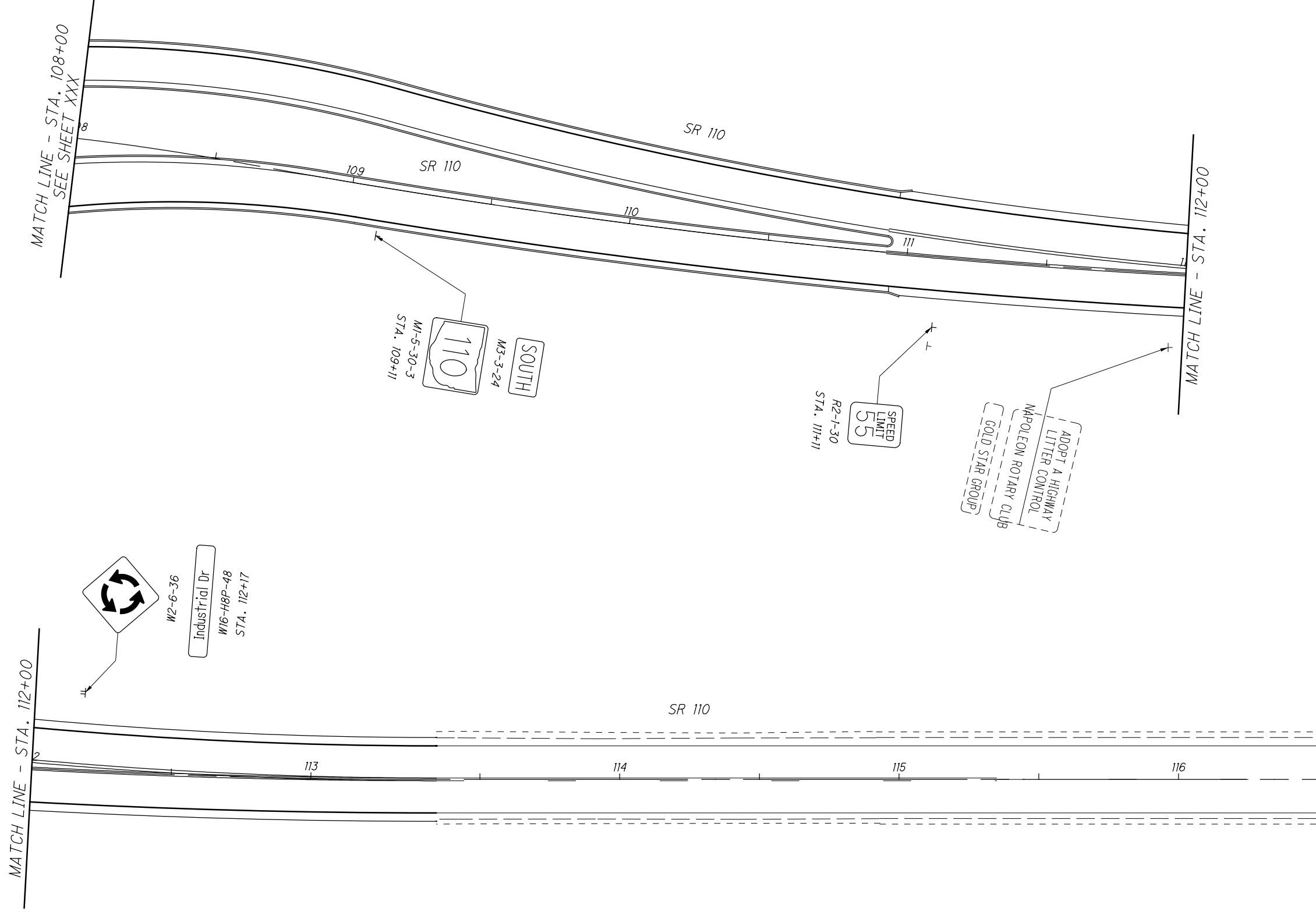
SIGNING PLAN ROUNDBOUT

HEN-NEW MAUMEE RIVER BRIDGE







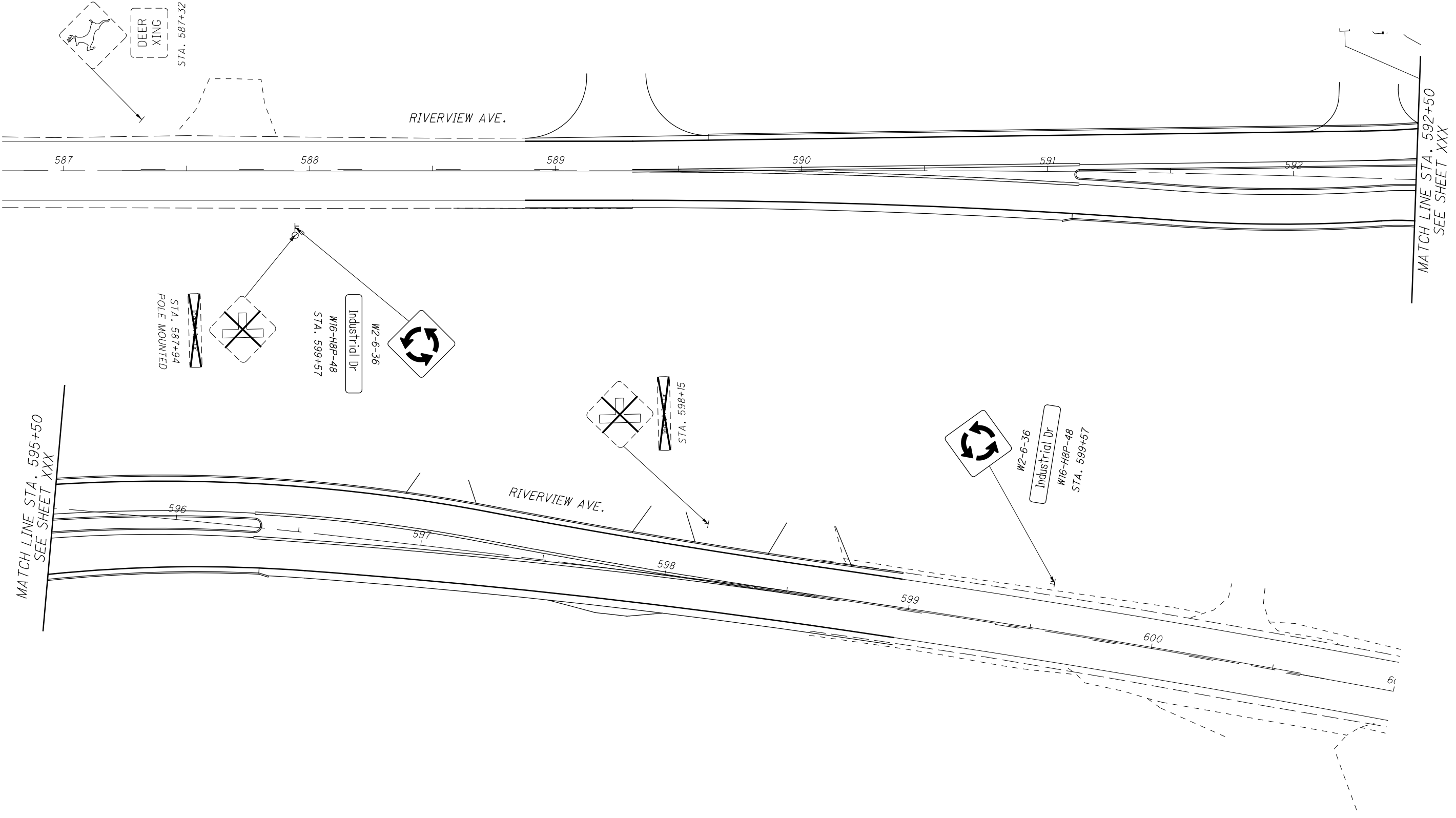


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0 20 40
 HORIZONTAL
 SCALE IN FEET

SIGNING AND PAVEMENT MARKING PLAN
SR 110

**HEN-NEW MAUMEE
 RIVER BRIDGE**



SPECIFICATIONS

LIGHTING GENERAL NOTES ARE SUPPLEMENTAL TO ITEMS 625 AND 725 OF THE OHIO DEPARTMENT OF TRANSPORTATION (ODOT) CONSTRUCTION AND MATERIAL SPECIFICATIONS, DATED JANUARY 1, 2013, WHICH SHALL GOVERN ALL WORK OF THIS PROJECT, EXCEPT AS HEREINAFTER MODIFIED.

REFERENCES SHALL BE MADE TO STANDARD CONSTRUCTION DRAWINGS LISTED ON THE TITLE SHEET.

UNDERDRAINS FOR PULL BOXES

REFERENCE IS MADE TO ODOT STANDARD DRAWING HL-30.11 FOR DETAILS OF DRAINING PULL BOXES. UNDERDRAINS FOR PULL BOXES SHALL BE USED AS DIRECTED BY THE ENGINEER AND SHALL BE PROVIDED WHERE THE LENGTH REQUIRED FOR A SATISFACTORY OUTLET DOES NOT EXCEED 20 FEET. THE COST FOR THIS WORK IS TO BE INCLUDED IN THE PULLBOX ITEMS.

625, POWER SERVICE, AS PER PLAN

IN ADDITION TO THE REQUIREMENTS OF THE SPECIFICATIONS, THE FOLLOWING IS ADDED.

THE POWER SUPPLYING AGENCY FOR THIS PROJECT IS:

TOLEDO EDISON
MS: A-HLOC-2332
6099 ANGOLA ROAD
TOLEDO, OHIO, 43528
MR. BRAD RUETZ
419-249-5903

THE ENGINEER SHALL ENSURE THAT EACH POWER SERVICE ELECTRICAL ENERGY ACCOUNT IS IN THE NAME OF AND THAT THE BILLING ADDRESS IS TO THE MAINTAINING AGENCY NOTED IN THE PLANS.

PAYMENT WILL BE MADE AT THE UNIT BID PRICE FOR EACH CMS ITEM 625, "POWER SERVICE, AS PER PLAN" WHICH SHALL BE FULL COMPENSATION FOR ALL LABOR, MATERIALS AND INCIDENTALS REQUIRED TO COMPLETE THIS ITEM IN A SATISFACTORY AND WORKMANLIKE MANNER.

PADLOCKS AND KEYS

PADLOCKS FURNISHED SHALL BE EITHER BRASS OR BRONZE, EQUAL TO MASTER NO. 4BKA OR WILSON BOHANNAN 660A, AND SHALL BE KEYED IN ACCORDANCE WITH CMS 631.06. PAYMENT SHALL BE INCLUDED IN THE BID FOR THE ITEM(S) BEING LOCKED.

CONTROL CENTER DATA									
CONTROL CENTER	LINE VOLTS	CONNECTED LOAD (KVA)	SERVICE ENTRANCE CONDUCTOR SIZE-AWG	ENCLOSURE RATING (AMPS)	CIRCUIT NO.	CIRCUIT LOAD AMPS	CIRCUIT FUSE SIZE AMPS	CIRCUIT FUSE SIZE AWG	MAINTAINING AGENCY
	120/240V SINGLE PHASE		PER TOLEDO EDISON	100					ODOT / CITY
NOTE: FOR ADDITIONAL CONTROL CENTER DETAILS, SEE STANDARD DRAWINGS									

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PLAN LEGEND

- ▲ PROPOSED POWER SERVICE
- A/// NO. 4 1-1/2" DUCT CABLE IN 24" DEEP TRENCH (CIRCUIT, AWG SIZE, #CONDUCTORS)
- CIRCUIT PB-X# [Symbol] PB REF. NO. PULL BOX, (725.08, 18")
- EXISTING LUMINAIRE & BRACKET ARM
- ⊕ EXISTING WOOD LIGHT POLE/ UTILITY POLE
- PROPOSED X" CONDUIT
- PROPOSED LUMINAIRE & BRACKET ARM
- CIRCUIT LP-X# [Symbol] POLE REF. NO. PROPOSED HL-10.12 LIGHT POLE & HL-20.11 LIGHT POLE FOUNDATION

PB-A-3 PULL BOX
STA 593+52, 98 LT

LP-A-3 LIGHT POLE
STA 593+52, 77 LT

LP-A-2 LIGHT POLE
STA 592+92, 41 LT

LP-A-8 LIGHT POLE
STA 592+83, 76 RT

LP-A-1 LIGHT POLE
STA 593+45, 106 RT

PB-A-1 PULL BOX
STA 593+48, 122 RT

PULL BOX
STA. 594+25, 95 LT PB-A-4

Light Pole
594+31, 72 LT LP-A-4

LIGHT POLE
STA 594+65, 19 LT LP-A-5

PULL BOX
STA 595+11, 15 LT PB-A-5

PULL BOX
STA 595+29, 45 RT PB-A-6

LIGHT POLE
STA 594+68, 61 RT LP-A-6

LIGHT POLE
STA 594+16, 98 RT LP-A-7

PULL BOX
STA. 594+16, 190 RT PB-A-2




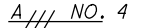






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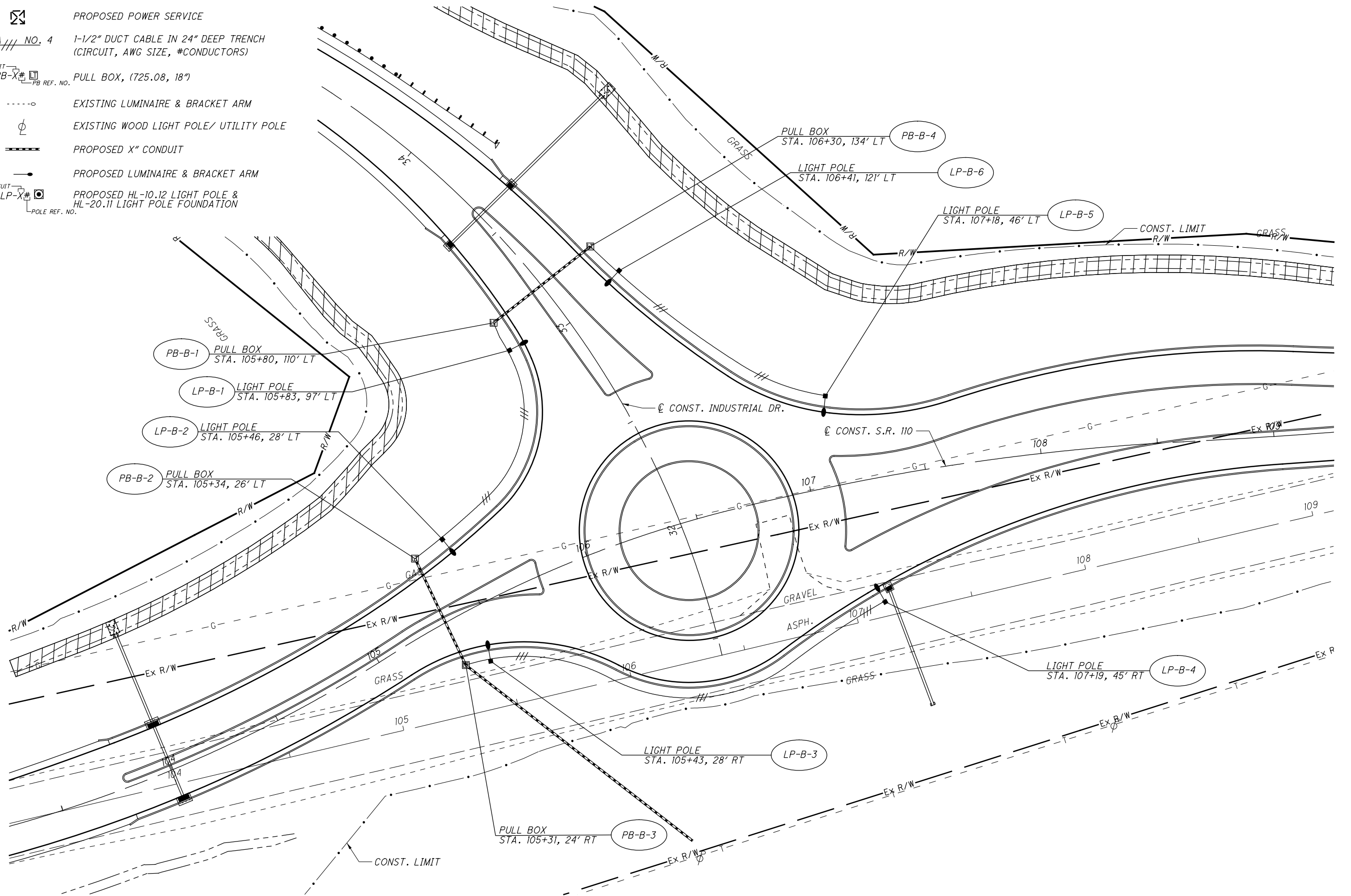
**LIGHTING PLAN
RIVERVIEW AVE. AND INDUSTRIAL DR.**

**HEN-NEW MAUMEE
RIVER BRIDGE**

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PLAN LEGEND

-  PROPOSED POWER SERVICE
-  NO. 4 1-1/2" DUCT CABLE IN 24" DEEP TRENCH (CIRCUIT, AWG SIZE, #CONDUCTORS)
-  PULL BOX, (725.08, 18")
-  EXISTING LUMINAIRE & BRACKET ARM
-  EXISTING WOOD LIGHT POLE/ UTILITY POLE
-  PROPOSED X" CONDUIT
-  PROPOSED LUMINAIRE & BRACKET ARM
-  PROPOSED HL-10.12 LIGHT POLE & HL-20.11 LIGHT POLE FOUNDATION



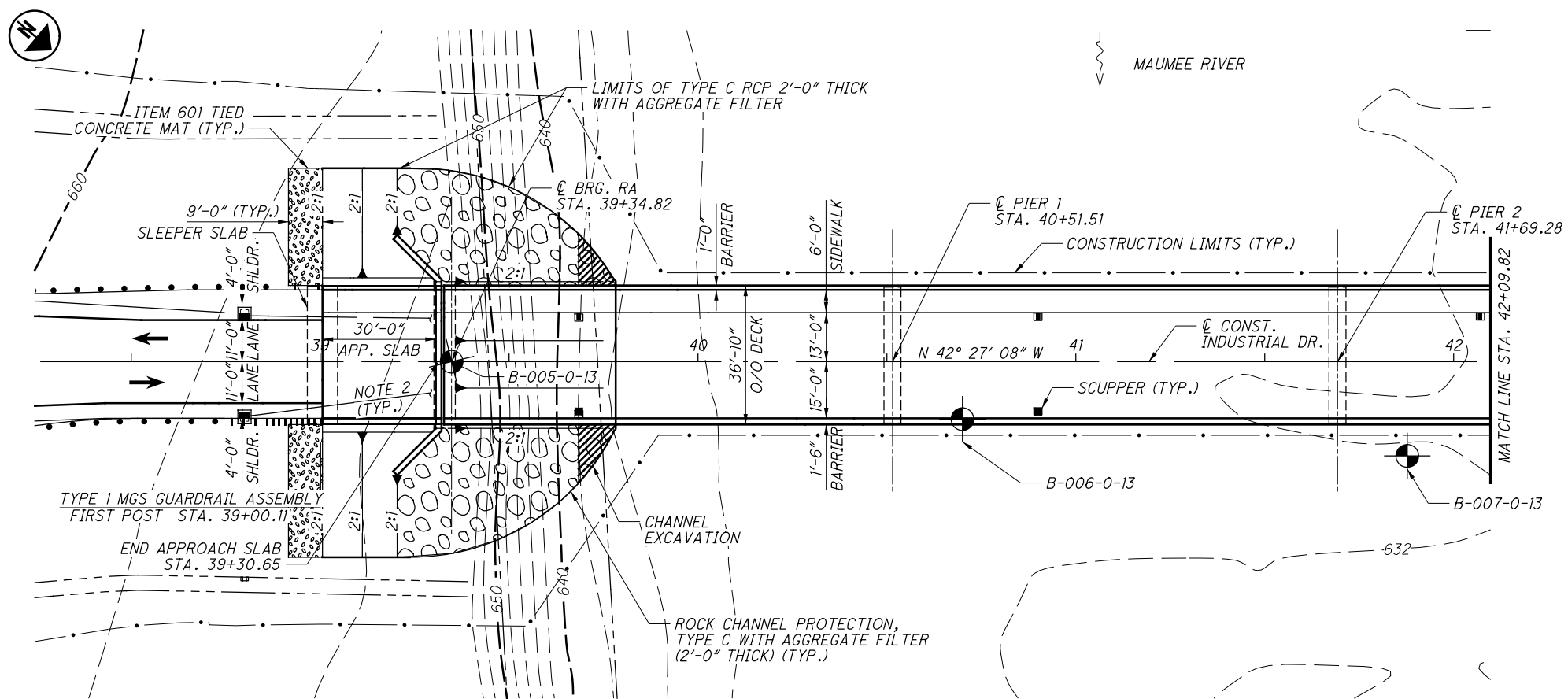
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CHECKED _____

LIGHTING PLAN
S.R. 110 AND INDUSTRIAL DRIVE

HEN-NEW MAUMEE
RIVER BRIDGE

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BENCHMARK DATA		
BM #1 STA. 55+02.58	ELEV. 672.71	OFFSET 19.45' RT
BM #2 STA. 65+94.80	ELEV. 680.83	OFFSET 134.94' LT
BM #3 STA.	ELEV.	OFFSET
BM #4 STA.	ELEV.	OFFSET

- NOTES**
- EARTHWORK LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS SECTIONS.
 - 12"φ PVC, SDR 35 (707.45), OUTLET TO CATCH BASINS. SEE ROADWAY PLANS FOR MORE INFORMATION.

DESIGN TRAFFIC:

2015 ADT = 7660 2015 ADTT = 690

2035 ADT = 9860 2035 ADTT = 890

DIRECTIONAL DISTRIBUTION = 0.55

- LEGEND**
- BORING LOCATION
 - CHANNEL EXCAVATION
 - T.O.S.
 - ROCK CHANNEL PROTECTION
 - CONCRETE MAT
 - PROPOSED STRUCTURE

HYDRAULIC DATA

DRAINAGE AREA = 5650 SQ. MILES

Q (25) = 87780 CFS V (25) = 5.00 FT/S

Q (100) = 110100 CFS V (100) = 5.49 FT/S

STRUCTURE CLEARS THE 25 YEAR

DESIGN HW BY 8.48 FEET.

APPROXIMATE TOP OF BEDROCK ELEVATIONS

B-005-0-13	EL. 633.2
B-006-0-13	EL. 632.4
B-007-0-13	EL. 632.3
B-008-0-13	EL. 631.0
B-008-1-13	EL. 631.8
B-009-0-13	EL. 632.2
B-010-0-13	EL. 634.3
B-011-0-13	EL. 631.3
B-012-0-13	EL. 634.8
B-013-1-13	EL. 631.3

PROPOSED STRUCTURE

TYPE: EIGHT SPAN COMPOSITE PRESTRESSED I-BEAM SUPERSTRUCTURE WITH REINFORCED CONCRETE DECK SUPPORTED BY REINFORCED CONCRETE ABUTMENTS AND PIERS WITH SLOPED EMBANKMENTS

SPANS: 8 SPANS @ 115'-7 1/4", 115'-7 1/4", 115'-7 1/4", 115'-7 1/4", 115'-7 1/4", 115'-7 1/4", 115'-7 1/4" AND 115'-7 1/4" C/C BEARINGS MEASURED ALONG CL CONSTRUCTION INDUSTRIAL DR.

ROADWAY: VARIABLE WIDTH 28'-0" MIN. TO 32'-0" MAX. TOE/TOE BARRIER WITH 6'-0" SIDEWALK (LEFT SIDE)

LOADING: HL-93 W/ 60 PSF FUTURE WEARING SURFACE

SKEW: NONE

APPROACH SLABS: 30'-0" LONG (AS-1-15) MODIFIED

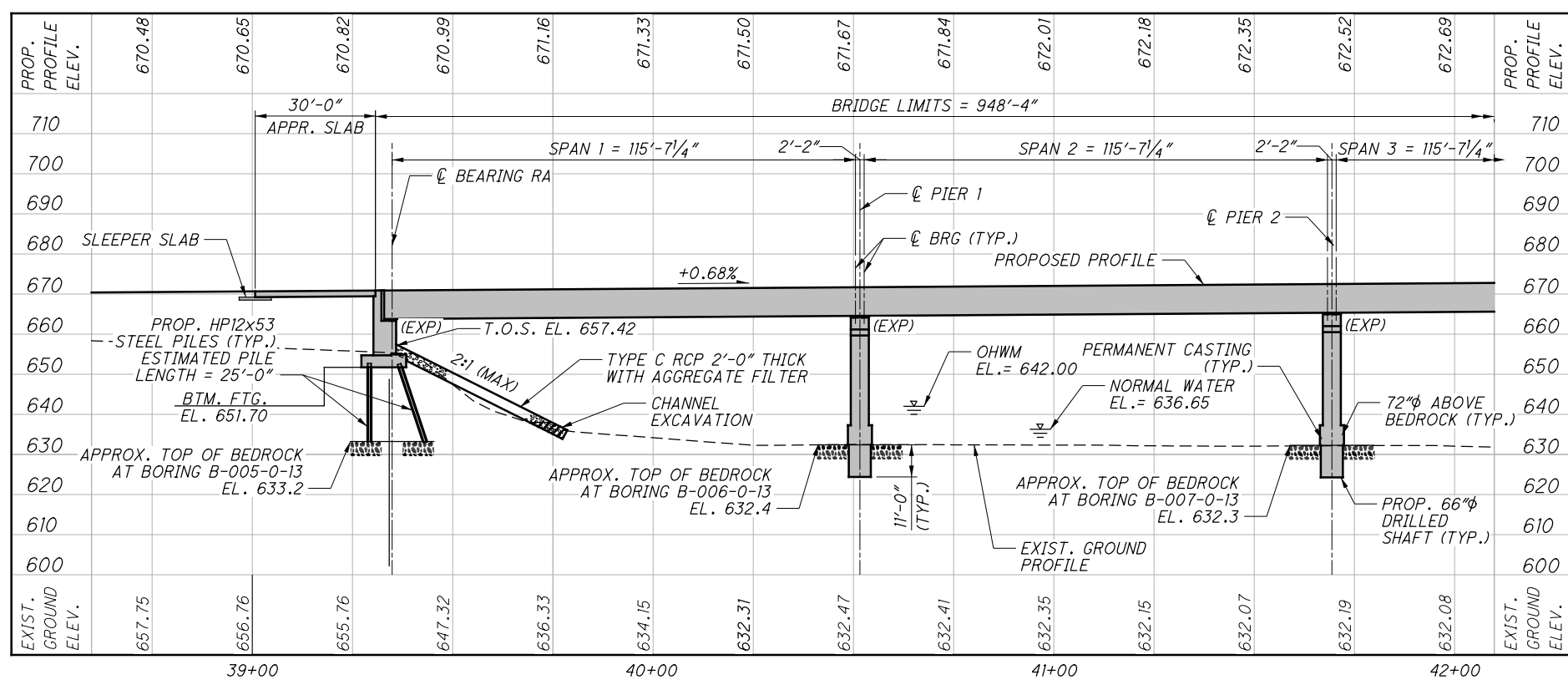
ALIGNMENT: TANGENT

CROWN: 0.016 FT/FT

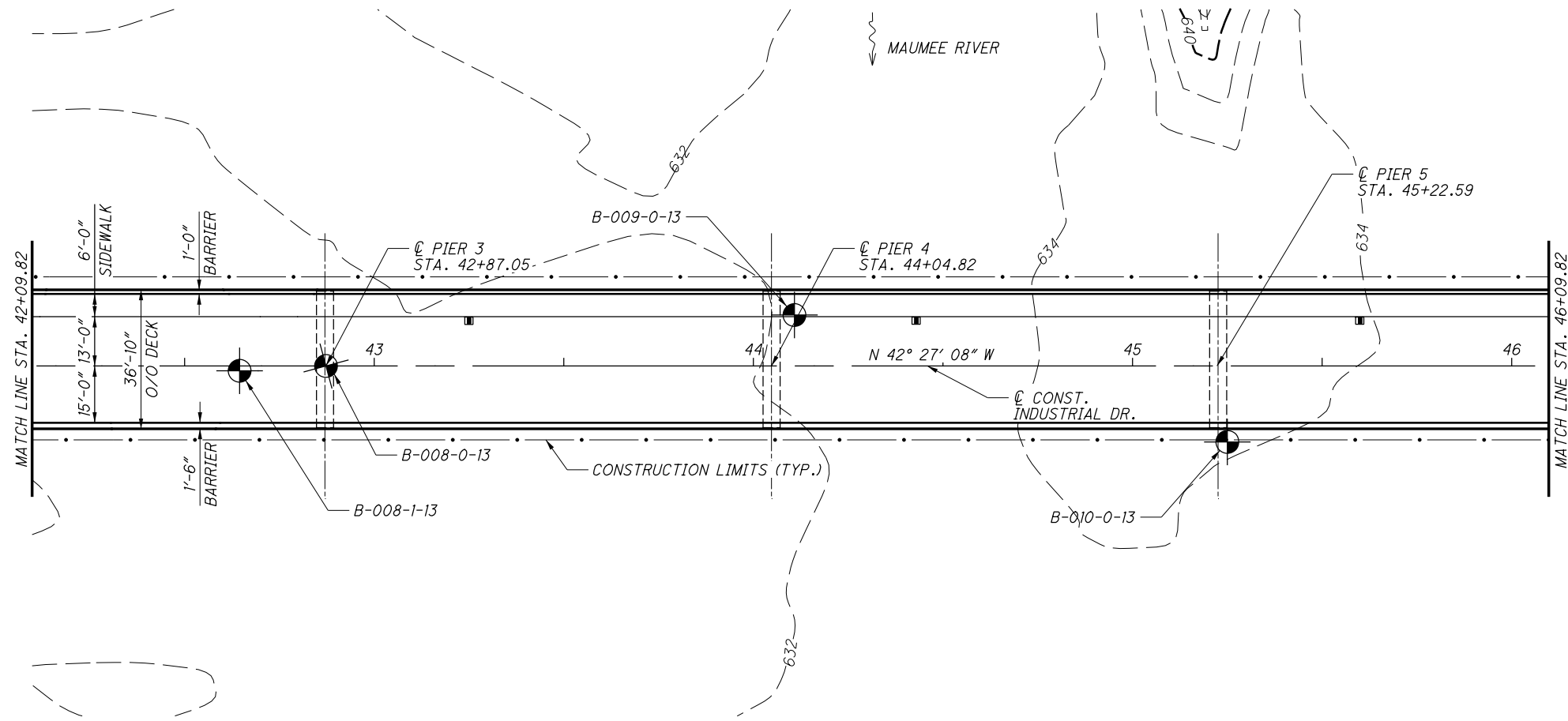
WEARING SURFACE: 1" MONOLITHIC CONCRETE

COORDINATES: LATITUDE N 41° 24' 17"

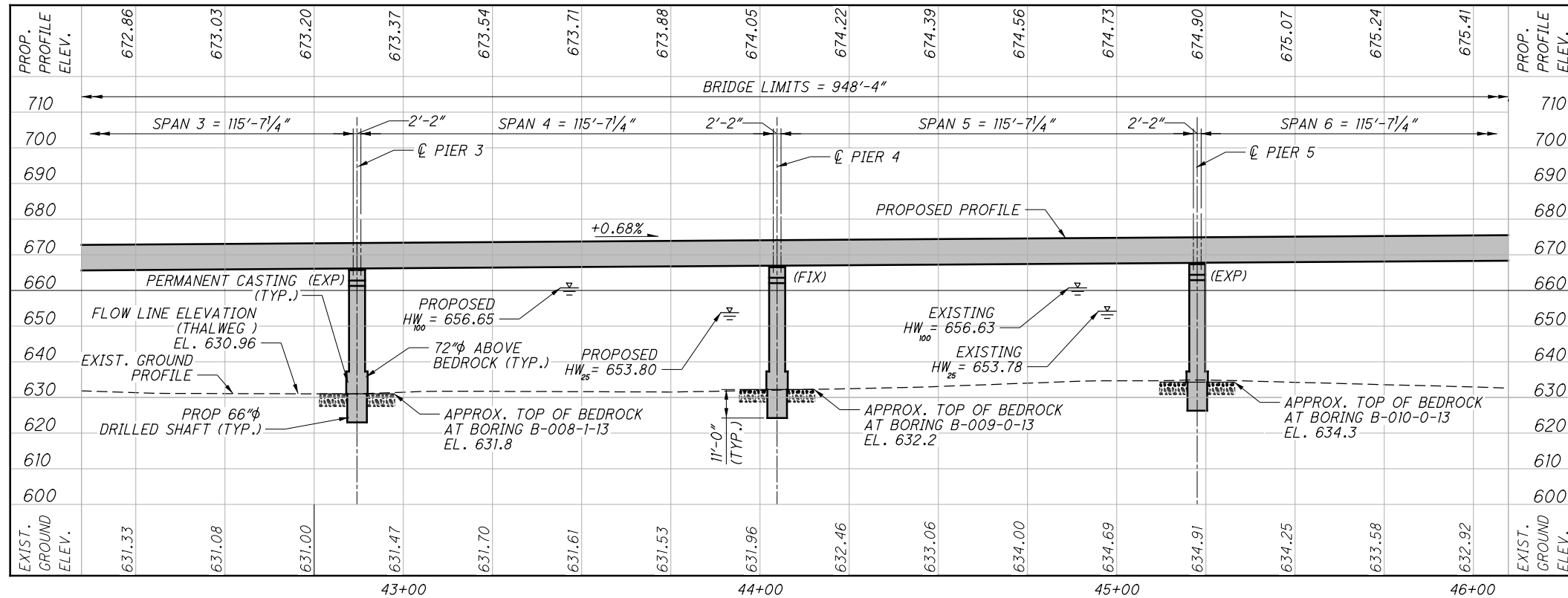
LONGITUDE W 84° 06' 14"



1800 INDIAN WOOD CIRCLE
 MAUMEE, OHIO 43537
Mannik Smith GROUP
 DATE: 05/2015
 REVIEWED: TLR
 DRAWN: ANK
 DESIGNED: KRH
 HENRY COUNTY
 STA. 39+46.50
 STA. 48+93.50
SITE PLAN
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER
 HEN-IND-00.00
 PID No. 22984
 1/64
 85
 180



NOTES
 1. FOR PLAN NOTES AND LEGEND SEE SHEET 1/64



1800 INDIAN WOOD CIRCLE

 MAUMEE, OHIO 43537

DESIGNED	CRH	CHECKED	SCT
DRAWN	ANK	REVISED	
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	05/2015		

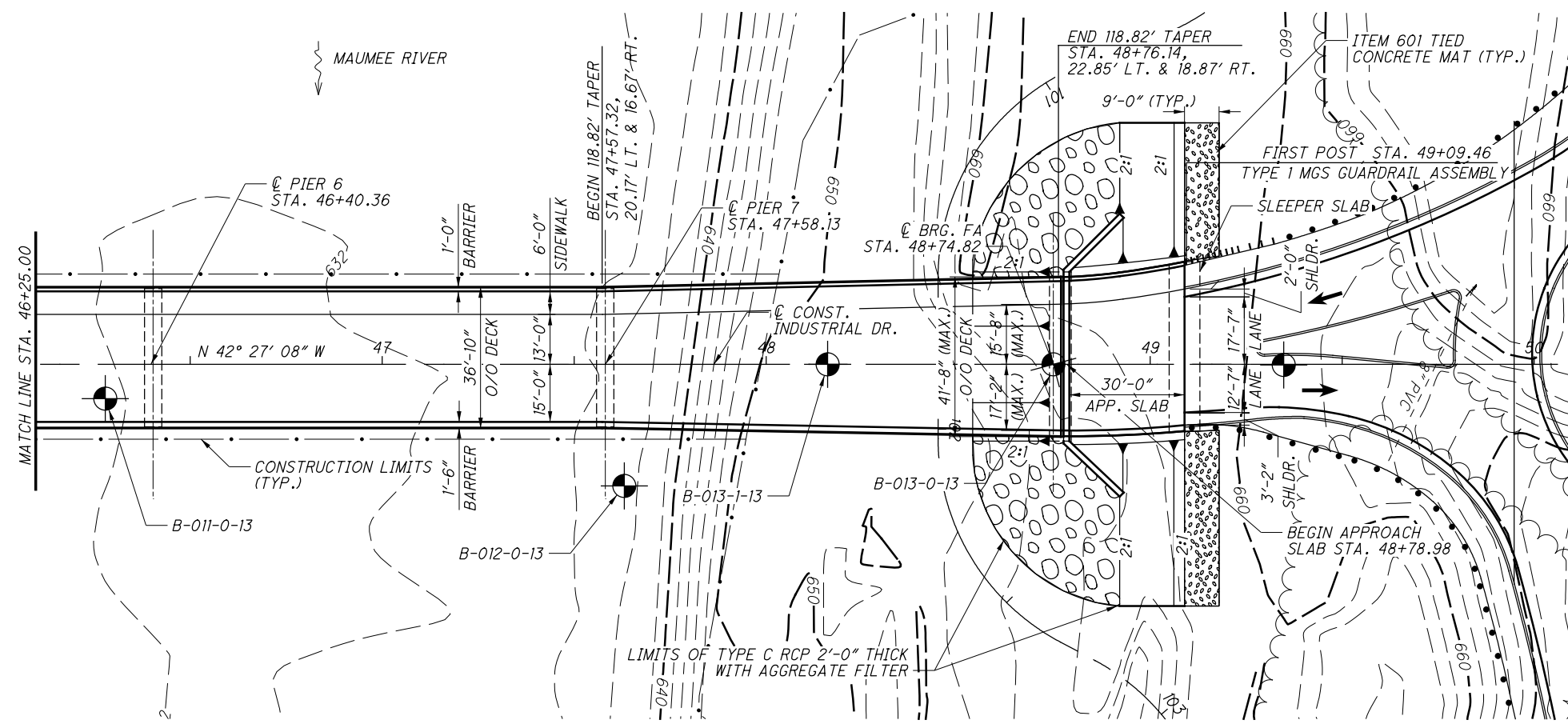
SITE PLAN

HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
 PID No. 22984

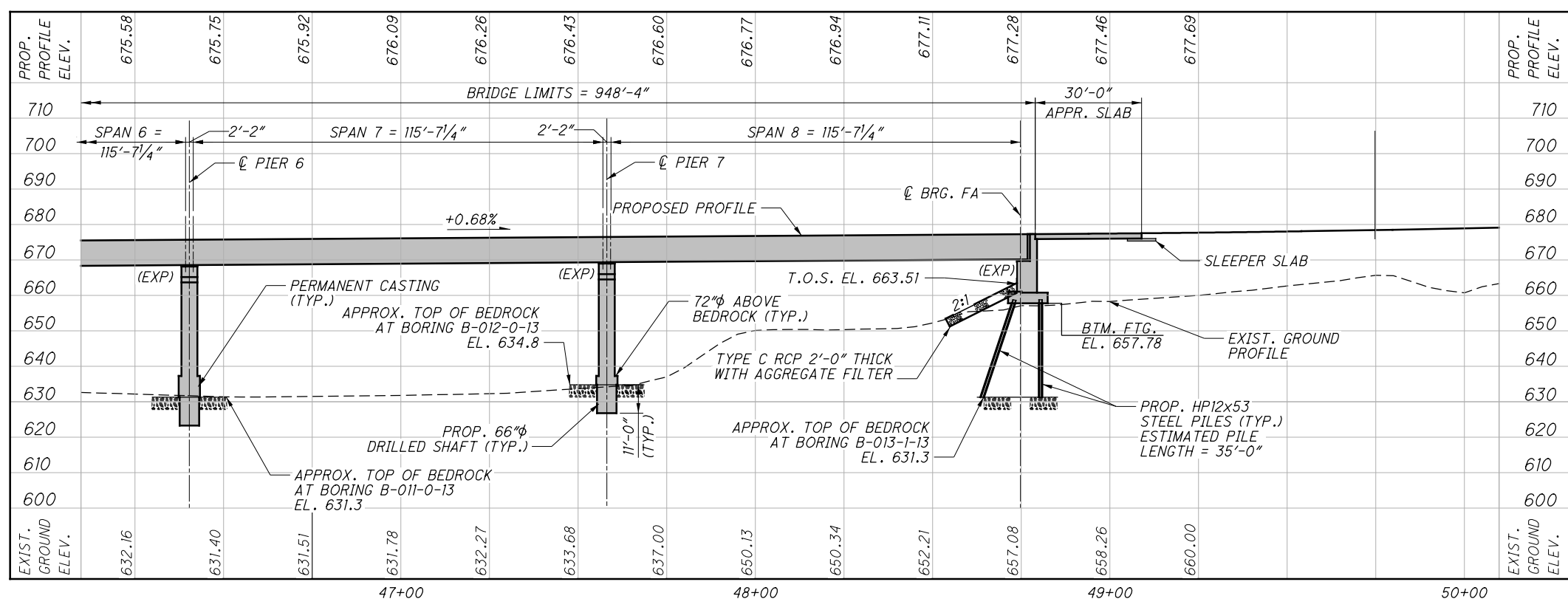
HENRY COUNTY
 STA. 39+46.50
 STA. 48+93.50

2 / 64
86
180



PART PLAN

NOTES
 1. FOR PLAN NOTES AND LEGEND SEE SHEET 1/64



PART PROFILE ALONG C CONST. INDUSTRIAL DRIVE

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	1800 INDIAN WOOD CIRCLE MAUMEE, OHIO 43537
HEN-IND-00.00 PID No. 22984	SITE PLAN HEN-INDUSTRIAL DRIVE-0000 INDUSTRIAL DRIVE OVER MAUMEE RIVER
3 / 64 <div style="border: 1px solid black; border-radius: 50%; width: 30px; height: 30px; display: flex; align-items: center; justify-content: center; margin: 0 auto;"> 87 180 </div>	HENRY COUNTY STA. 39+46.50 STA. 48+93.50
DESIGNED KRH CHECKED SCT	DRAWN ANK REVISED
REVIEWED TLR STRUCTURE FILE NUMBER TBD	DATE 05/2015

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STANDARD DRAWINGS AND SUPPLEMENTAL SPECIFICATIONS

REFER TO THE FOLLOWING STANDARD BRIDGE DRAWING(S):

- AS-1-15 DATED/REVISED 1/16/2015
- AS-2-15 DATED/REVISED 1/16/2015
- BR-2-98 DATED/REVISED 7/20/2012
- EXJ-6-06 DATED/REVISED 1/18/2013
- PSID-1-13 DATED/REVISED 1/16/2015
- SBR-1-13 DATED/REVISED 1/17/2014

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATION(S):

- 800 DATED 4/17/2015 832 DATED 1/17/2014
- 846 DATED 4/17/2015

DESIGN SPECIFICATIONS

DESIGN SPECIFICATIONS: THIS STRUCTURE CONFORMS TO THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 8TH EDITION, INCLUDING THE 2013 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL, 2007.

LOAD MODIFIER FOR OPERATIONAL IMPORTANCE

OPERATIONAL IMPORTANCE: A LOAD MODIFIER OF 1.00 HAS BEEN ASSUMED FOR THE DESIGN OF THIS STRUCTURE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, ARTICLE 1.3.5 AND THE ODOT BRIDGE DESIGN MANUAL, 2007.

DESIGN LOADING

DESIGN LOADING: HL-93
FUTURE WEARING SURFACE (FWS) OF 0.060 KIPS/SQ.FT.

DESIGN DATA

- DESIGN DATA:
- CONCRETE CLASS QC2 - COMPRESSIVE STRENGTH 4.5 KSI (SUPERSTRUCTURE)
- CONCRETE CLASS QC1 - COMPRESSIVE STRENGTH 4.0 KSI (SUBSTRUCTURE)
- CONCRETE CLASS QC2 - COMPRESSIVE STRENGTH 4.0 KSI (DRILLED SHAFT)
- REINFORCING STEEL - MINIMUM YIELD STRENGTH 60 KSI
- STEEL H-PILES - ASTM A572 - YIELD STRENGTH 50 KSI

DESIGN DATA

- CONCRETE FOR PRESTRESSED BEAMS:
- COMPRESSIVE STRENGTH (FINAL) - 7.0 KSI
- COMPRESSIVE STRENGTH (RELEASE) - 5.0 KSI
- PRESTRESSING STRAND:
- AREA = 0.217 SQ. IN.
- ULTIMATE STRENGTH = 270 KSI
- INITIAL STRESS = 202.5 KSI (LOW RELAXATION STRANDS)

DECK PROTECTION METHOD

- EPOXY COATED REINFORCING STEEL
- 2.5" CONCRETE COVER

PILE DRIVING CONSTRAINTS

PILE DRIVING CONSTRAINTS: PRIOR TO DRIVING PILES, CONSTRUCT THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FT BEHIND EACH ABUTMENT. DO NOT BEGIN THE EXCAVATION FOR THE ABUTMENT FOOTINGS AND THE INSTALLATION OF THE ABUTMENT PILES UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED.

PILES TO BEDROCK

PILES TO BEDROCK: DRIVE PILES TO REFUSAL ON BEDROCK. THE DEPARTMENT WILL CONSIDER REFUSAL TO BE OBTAINED WHEN THE PILE PENETRATION IS AN INCH OR LESS AFTER RECEIVING AT LEAST 20 BLOWS FROM THE PILE HAMMER. SELECT THE HAMMER SIZE TO ACHIEVE THE REQUIRED DEPTH TO BEDROCK AND REFUSAL.

THE TOTAL FACTORED LOAD IS 317 KIPS PER PILE FOR THE REAR ABUTMENT PILES AND 336 KIPS PER PILE FOR THE FORWARD ABUTMENT PILES. THE ABUTMENT PILES INCLUDE AN ADDITIONAL 36 KIPS OF FACTORED LOAD PER PILE TO ACCOUNT FOR POSSIBLE DOWNDRAW FORCES.

ABUTMENT PILES:

- REAR: 26 HP12X53 PILES 25 FEET LONG, ORDER LENGTH
- FORWARD: 28 HP12X53 PILES 35 FEET LONG, ORDER LENGTH

PILE SPLICES

PILE SPLICES: IN LIEU OF USING THE FULL PENETRATION BUTT WELDS SPECIFIED IN CMS 507.09 TO SPLICE STEEL H-PILES, THE CONTRACTOR MAY USE A MANUFACTURED H-PILE SPLICER. FURNISH SPLICERS FROM THE FOLLOWING MANUFACTURER:

ASSOCIATED PILE AND FITTING CORPORATION
8 WOOD HOLLOW RD. PLAZA 1
PARSIPPANY, NJ 07054

INSTALL AND WELD THE SPLICER TO THE PILE SECTIONS IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN ASSEMBLY PROCEDURE SUPPLIED TO THE ENGINEER BEFORE THE WELDING IS PERFORMED.

ITEM 203 EMBANKMENT, AS PER PLAN

PLACE AND COMPACT EMBANKMENT MATERIAL IN 6 INCH LIFTS FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT BETWEEN STATIONS 38+34.82 TO 49+74.82.

ITEM 503 UNCLASSIFIED EXCAVATION, AS PER PLAN:

UNCLASSIFIED EXCAVATION SHALL BE IN ACCORDANCE WITH 503 EXCEPT THAT THE BACKFILL MATERIAL PLACED BEHIND THE ABUTMENTS SHALL BE 703.17 MATERIAL PLACED IN 6 INCH LIFTS AS PER 304.05.

ITEM 509 - EPOXY COATED REINFORCING STEEL, AS PER PLAN

ITEM - 509 EPOXY COATED REINFORCING STEEL, AS PER PLAN: IN ADDITION TO THE PROVISIONS OF TIEM 509, FIELD BEND AND/OR FIELD CUT THE REINFORCING STEEL DESIGNATED IN THE PLANS, AS NECESSARY, IN ORDER TO MAINTAIN THE REQUIRED CLEARANCES AND BAR SPACINGS. REPAIR ALL DAMAGE TO THE EPOXY COATING, AS A RESULT OF THIS WORK, ACCORDING TO 709.00

DECK PLACEMENT DESIGN ASSUMPTIONS:

DECK PLACEMENT DESIGN ASSUMPTIONS: THE FOLLOWING ASSUMPTIONS OF CONSTRUCTION MEANS AND METHODS WERE MADE FOR THE ANALYSIS AND DESIGN OF THE SUPERSTRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE FALSEWORK SUPPORT SYSTEM WITHIN THESE PARAMETERS AND WILL ASSUME RESPONSIBILITY FOR SUPERSTRUCTURE ANALYSIS FOR DEVIATION FROM THESE DESIGN ASSUMPTIONS.

AN EIGHT WHEEL FINISHING MACHINE WITH A MAXIMUM WHEEL LOAD OF 2.2 KIPS FOR A TOTAL MACHINE LOAD OF 17.6 KIPS.

A MINIMUM OUT-TO-OUT WHEEL SPACING AT EACH END OF THE MACHINE OF 103".

A MAXIMUM SPACING OF OVERHANG FALSEWORK BRACKETS OF 48 IN.

A MAXIMUM DISTANCE FROM THE CENTERLINE OF THE FASCIA GIRDER TO THE FACE OF THE SAFETY HANDRAIL OF 65".

MONOLITHIC WEARING SURFACE

MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES, TO BE 1 INCH THICK.

ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA CONCRETE, SUPERSTRUCTURE, AS PER PLAN (BRIDGE DECK)

DESCRIPTION:

IN ADDITION TO THE WORK REQUIREMENTS OF 511, THE CONTRACTOR MAY EITHER PROVIDE TRADITIONAL BRIDGE DECK FORMS CONFORMING TO CMS 508 OR DESIGN, BUILD, PROVIDE, AND CONSTRUCT GALVANIZED STEEL STAY-IN-PLACE (SIP) FABRICATED METAL FORMS CONFORMING TO CMS 508 AND THESE ADDITIONAL REQUIREMENTS. THE DEPARTMENT WILL NOT SEPARATELY PAY FOR SIP FORMS. THE COST OF THIS WORK IF CHOSEN BY THE CONTRACTOR SHALL BE INCLUDED FOR PAYMENT IN THE PRICE BID FOR ITEM 511. THE DEPARTMENT WILL PAY NO EXTRA FOR ANY ADDITIONAL CONCRETE, REINFORCEMENT STEEL, OR STRUCTURAL STEEL THAT MAY BE REQUIRED WHEN USING SIP FORMS. ANY ADDITIONAL COST AND/OR DESIGN ASSOCIATED WITH THE USE OF SIP FORMS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE ADDITIONAL DEAD LOAD OF THE SIP FORM PLUS THE WEIGHT OF THE ADDITIONAL CONCRETE SHALL BE DETERMINED FROM THE REQUIRED BEAM SPACING AND DEPTH OF FORM. THIS LOAD WILL BE IN ADDITION TO THE LOADS AS SPECIFIED AS DESIGN LOADS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DESIGN, FABRICATION, AND INSTALLATION MODIFICATIONS TO THE BRIDGE COMPONENTS INCLUDING THE BRIDGE BEAMS OR GIRDERS, CAMBER DIAGRAMS, DECK SCREED TABLES, BRIDGE BEARINGS, AND SUBSTRUCTURES. ALL PLAN MODIFICATIONS SHALL BE PREPARED AS PER 501.

DESIGN, BUILD, SIP FORMS WILL NOT BE PERMITTED AT OVERHANGS, AND WITHIN EIGHT FEET OF ALL EXPANSION JOINTS AND FOUR FEET OF ALL THROUGH DECK DRAINAGE SYSTEMS. IN ADDITION SIP FORMS WILL NOT BE PERMITTED WITHIN STRINGER BAYS WHERE CONSTRUCTION IS PHASED OR CLOSURE POURS ARE USED.

IF THE CONTRACTOR ELECTS TO USE CONVENTIONAL FORM METHODS OR SIP FORMS, THEN THE FORM METHOD SELECTED SHALL BE USED FOR THE ENTIRE BRIDGE PROJECT UNLESS SPECIFIED IN THE PLANS OR APPROVED BY THE ENGINEER. THIS INCLUDES PROJECTS UTILIZING PHASED CONSTRUCTION AND STIPULATES THAT ALL PHASES BE CONSTRUCTED ALIKE.

DESIGN: SUBMIT CONSTRUCTION PLANS ACCORDING TO 501.05.B.3. DESIGN SIP FORMS TO SUPPORT THE SELF WEIGHT OF SIP FORMS, REINFORCEMENT, WET CONCRETE FOR THE DECK, ANY CONSTRUCTION EQUIPMENT LOADS, AND AT LEAST A 50 PSI LOAD FOR CONSTRUCTION LIVE LOADS MEET THE DEFLECTION REQUIREMENTS OF 508.

DESIGN SIP FORMS THAT HAVE THE DEPTH OF THE FORM CORRUGATION FILLED WITH CONCRETE.

INCLUDED THE FOLLOWING INFORMATION IN THE CONSTRUCTION PLAN:

- A: DESIGN CALCULATIONS
- B: PHYSICAL PROPERTIES OF THE SIP FORMS (GAGE, SECTION MODULUS, WEIGHT, DEPTH, AND PITCH)
- C: CROSS SECTION VIEW AND DIMENSIONS OF: SIP FORMS, SUPPORT ANGLES, CHANNELS CLOSURES, SAFETY STOPS, CLIPS, PLATES, AND HARDWARE.
- D: INCLUDE AN OVERALL LAYOUT PLAN WITH
 - WORKING POINTS OR CONTROL ELEVATIONS NECESSARY TO SET SUPPORT ANGLES.
 - TYPICAL AND SPECIFIC CROSS SECTIONS OR DETAILS: SUPPORT CONNECTIONS TO THE STRUCTURAL MEMBERS, SIP FORM CONNECTIONS TO SUPPORTS, FORM LAPS, AND CLOSURE SECTIONS.
 - MINIMUM BEARING LENGTHS (EDGE DISTANCES) OF SIP FORMS TO THE SUPPORT ANGLES.
 - WELDING DETAILS: SIZE, LENGTH, LOCATIONS, ELECTRODES, AND PROCESS.
- E: WORKER SAFETY RESTRICTIONS.
- F: INSTALLATION INSPECTION CHECK LISTS.

MATERIALS:

SUBMIT 501.06 TEST REPORTS AND WRITTEN ACCEPTANCE LETTERS TO THE ENGINEER. MATERIALS INSPECTION AND ACCEPTANCE IS PERFORMED BY THE ENGINEER AT THE PROJECT SITE. FURNISH FORM, SUPPORT MATERIALS, AND HARDWARE CONFORMING TO THE FOLLOWING:

- A: FORM AND SUPPORT MATERIAL, ASTM A653 HAVING A COATING DESIGNATION OF G235, AND CONFORMING TO THE MECHANICAL PROPERTIES THE DESIGN REQUIRES.
- B: PROVIDE DECK FORMS WITH A 2 INCH MINIMUM FORM DEPTH.
- C: PROVIDE MINIMUM MATERIAL THICKNESS AS FOLLOWS: SIP FORMS (20 GAGE), SUPPORT ANGLES (12 GAGE) AND SUPPORT BARS (12 GAGE).
- D: SUPPLY DECK, SELF DRILLING FASTENERS WITH CADMIUM PLATING PER ASTM B766 WITH MINIMUM THICKNESS OF 5, TEN THOUSANDTHS. (0.0005 INCH). THE HEADS OF THESE FASTENERS WILL BE A HIGHLY VISIBLE COLOR, RED OR OTHER, TO AID INSPECTION.

GENERAL NOTES (1 OF 3)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN - IND - 00.00
PID No. 22984

4 / 64

88
180

DATE: 05/2015
REVIEWED: TLR
DRAWN: RJS
DESIGNED: DRH
CHECKED: SCT

STRUCTURE FILE NUMBER
TBD

Morrisk
Smith
GROUP
1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

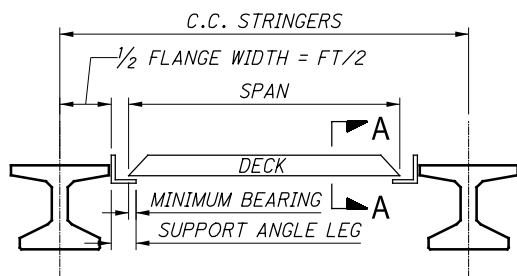
ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA CONCRETE, BRIDGE DECK, AS PER PLAN (CONTINUED)

WELDING:

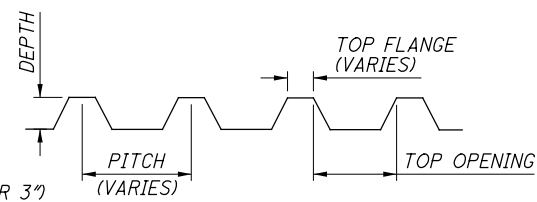
DO NOT WELD SIP FORM OR THEIR SUPPORTS TO THE STEEL BRIDGE MEMBERS. SIP SUPPORTS MAY BE WELDED TO ANCHORS CAST INTO PRECAST CONCRETE BRIDGE MEMBERS. PERFORM WELDING PER 513.21.

INSTALLATION LIMITATIONS:

- A: FIELD CUT SIP FORMS USING MECHANICAL CUTTING METHODS. THERMAL CUTTING IS NOT PERMITTED.
- B: PLACE FORMS ON FORM SUPPORTS. DO NOT INSTALL SIP FORMS DIRECTLY TO THE BRIDGE'S STRUCTURAL MEMBERS.
- C: ADJUST THE SCREED ELEVATIONS BY PRORATING THE CONCRETE DEAD LOAD DEFLECTION TO ACCOUNT FOR THE ADDITIONAL PERMANENT DEAD LOADS ASSOCIATED WITH CONCRETE FILLED STAY IN PLACE FORMS.
- D: SET THE HEIGHT OF THE FORM SUPPORTS TO DEVELOP THE ADJUSTED SCREED ELEVATIONS, DECK THICKNESS, AND PLAN PROFILE.
- E: PLACE SIP FORMS TO FORM SUPPORTS TO ACHIEVE MINIMUM BEARING LENGTH PER MANUFACTURER'S DESIGN.
- F: CONNECT SIP FORMS TO FORM SUPPORTS BEFORE USING THE SIP AS A WORKING SURFACE AND BEFORE THE END OF EACH WORK SHIFT.
- G: PROVIDE SAFETY STOPS TO ELIMINATE HAZARDS FROM SUDDEN UPLIFT AND LATERAL MOVEMENT. AFTER THE DECK CONCRETE MEETS THE LOADING REQUIREMENTS OF C&MS 511.17, REMOVE THE VISIBLE PORTION OF ALL SAFETY STOPS.
- H: COATINGS DAMAGED BY MECHANICAL CUTTING OR FIELD WELDING NEED NOT BE REPAIRED UNLESS SPECIFIED BY THE SIP FORM MANUFACTURER.
- I: THE CONTRACTOR SHALL PROTECT INSTALLED SIP FORMS FROM ANY CLEANING SOLUTIONS, BLASTING, OR OTHER WORK OPERATIONS THAT MAY DAMAGE THE FORM COATING. FORMS THAT ARE DAMAGED FROM LACK OF PROTECTION SHALL BE REPAIRED OR REMOVED AS DIRECTED BY THE ENGINEER. IF DIRECTED TO REPAIR, THE DAMAGED AREAS SHALL BE METALIZED AS PER 516.03 AND SUPPLEMENTAL SPECIFICATION 845. ALL COST FOR THE REPAIR OR REMOVAL SHALL BE PAID BY THE CONTRACTOR.



STAY IN PLACE DECK ELEVATION
NTS



SECTION A-A
STAY IN PLACE FORM TYPICAL DETAIL
(OPTIONAL)
NTS

INSPECTIONS:

THE ENGINEER WILL CHECK SIP MATERIALS MEET DESIGN REQUIREMENTS AND EVALUATE INSTALLATION BASED ON CONSTRUCTION PLAN.

BASIS OF PAYMENT:

THE DEPARTMENT WILL NOT SEPARATELY PAY FOR SIP FORMS. THE COST OF THIS WORK IS INCLUDED FOR PAYMENT IN THE PRICE BID FOR THE ITEM FOR WHICH THE SIP FORMS ARE USED.

ITEM 513 STRUCTURAL STEEL MEMBERS, LEVEL UF, AS PER PLAN

ALL MATERIAL, LABOR, AND INCIDENTALS ASSOCIATED WITH INTERMEDIATE SCUPPER SUPPORTS SHALL BE INCLUDED WITH ABOVE ITEM 513 FOR PAYMENT.

ITEM 515 INTERMEDIATE DIAPHRAGM, AS PER PLAN

THE GALVANIZED STEEL OPTION FOR INTERMEDIATE DIAPHRAGMS SHALL BE USED. THE CONCRETE OPTION SHALL NOT BE USED.

ITEM 516 SPECIAL: MODULAR EXPANSION JOINT

ABUTMENT JOINTS SHALL BE WATSON BOWMAN ACME (WABO) MODULAR D-600, DS BROWN D-160, OR APPROVED ALTERNATE. THE MANUFACTURER SHALL SUBMIT DESIGN CALCULATIONS SHOWING THAT THE DEVICE CAN MEET THE IMPACT AND FATIGUE DESIGN REQUIREMENTS SET FORTH BY AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 6TH EDITION, SECTION 14.5.

A. DESCRIPTION:

FURNISH ALL MATERIALS, SERVICES, LABOR, TOOLS, EQUIPMENT, AND INCIDENTALS NECESSARY TO DESIGN, FABRICATE, INSPECT, TEST AND INSTALL MODULAR EXPANSION JOINTS IN ACCORDANCE WITH THE PLANS AND THESE NOTES. ALL REQUIREMENTS OF 513, SF LEVEL FABRICATION APPLY, UNLESS MODIFIED BY THESE NOTES.

B. DESIGN:

1. PREPARE AND CHECK THE DESIGN UNDER THE AUTHORITY OF AN OHIO REGISTERED PROFESSIONAL ENGINEER. THE REGISTERED ENGINEER SHALL SEAL, SIGN, AND DATE THE DESIGN CALCULATIONS AND SHOP DRAWINGS.

2. INCLUDE DESIGN CALCULATIONS WITH THE CONTRACTOR'S SUBMISSION OF SHOP DRAWINGS PER 513.06.

3. PROVIDE A DETAILED INSTALLATION PROCEDURE AND INCLUDE ANY SPECIFIC MANUFACTURER'S NOTES NECESSARY FOR COMPLETION OF THE WORK.

4. DESIGN AND TEST THE MODULAR JOINT COMPONENTS, JOINT ARMOR AND ANCHORAGES ACCORDING TO THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND THE NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM (NCHRP) REPORT 402 "FATIGUE DESIGN OF MODULAR BRIDGE EXPANSION JOINTS" APPENDIX A AND B.

5. DESIGN TEMPORARY AND FIELD CONNECTIONS TO THE BRIDGE TO ACCOMMODATE ADJUSTMENTS FOR ROADWAY GEOMETRY AND VARYING TEMPERATURE.

6. DESIGN FOR THE PLAN SPECIFIED MOVEMENT PER AASHTO LRFD 3.12.2 FOR A COLD CLIMATE (TEMPERATE RANGE IS FROM -30°F TO +120°F WITH BASE TEMPERATURE SET TO 60°F).

7. SUPPLY SUPPORT BAR BEARINGS TO TRANSFER THE LOAD FROM THE SUPPORT BARS TO THE JOINT ARMOR.

8. FOR DESIGN OF THE DECK JOINT AT ALL LIMIT STATES, THE DYNAMIC LOAD ALLOWANCE (DM) SHALL BE TAKEN AS 125% OF THE STATIC EFFECT OF EITHER THE DESIGN TRUCK OR THE DESIGN TANDEM.

9. SUPPLY EQUALIZATION SPRINGS TO COUNTER THE COMPRESSION FORCES FROM THE SEALING ELEMENTS AND MAINTAIN EQUAL EXPANSION PROPERTIES FOR EACH SEALING ELEMENT ACROSS THE JOINT.

10. SUPPLY CONTROL SPRINGS WHICH WORK LONGITUDINALLY TO MAINTAIN EQUAL DISTANT SPACING BETWEEN TRANSVERSE SEPERATION BEAMS.

11. SUPPLY SEPERATION BEAMS/TRANSVERSE DIVIDERS/CENTER BEAMS TO LIMIT TOTAL HORIZONTAL MOVEMENT IN ANY INDIVIDUAL STRIP SEAL.

12. SUPPLY A STRIP SEAL TYPE SEAL CONNECTED TO MATCHING RETAINERS CONNECTED TO THE JOINT ARMOR AND THE SEPERATION BEAMS. DO NOT EXCEED 3.15 INCHES OF TOTAL HORIZONTAL MOVEMENT IN ANY INDIVIDUAL STRIP SEAL.

13. SUPPLY REMOVABLE AND REPLACEABLE NEOPRENE SEALS, SUPPORT BAR BEARINGS AND EQUILIZATION SPRINGS.

14. SET SEALS AND RETAINERS 1/8" LOWER THAN THE ROADWAY SURFACE.

15. DESIGN AND FABRICATE THE MODULAR JOINT AS A CONTINUOUS FULL LENGTH MEMBER WITHOUT FIELD SPLICES.

C. MATERIALS:

1. SUPPLY STRUCTURAL STEEL MEETING ASTM A709 GRADE 50. SUPPLY SEPERATION BEAMS/TRANSVERSE DIVIDERS/CENTER BEAMS, EDGE BEAMS AND JOINT ARMOR MEETING CHARPY V NOTCH IMPACT REQUIREMENTS PER ASTM A709 TABLE S12 ZONE 2 TEMPERATURE RANGE. SUPPLY TUBE SECTIONS MEETING ASTM A501 OR A500 GRADE B.

2. SUPPLY ASTM A240, TYPE 304 STAINLESS STEEL, 13 GAGE MINIMUM THICKNESSES WITH NO. 8 FINISH FOR SLIDING SURFACES IN CONTACT WITH PTFE.

3. SUPPLY TESTING AND REPORTS BY THE MANUFACTURER OR AN INDEPENDENT TESTING LABORATORY FOR ALL ELASTOMERIC, PTFE URETHANE AND PREFORMED FABRIC MATERIALS USED IN ALL BEARINGS AND SPRINGS. THE SUBMISSION OF MATERIAL CERTIFICATION AND TESTING DATA SHALL BE PER 514.08. THESE MATERIALS SHALL BE TESTED ACCORDING TO THE NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM (NCHRP) REPORT 402 APPENDIX A "A GUIDELINE FOR DURABILITY (NCHRP) REPORT 402 APPENDIX A "A GUIDELINE FOR DURABILITY TESTING OF SPRINGS AND BEARINGS FOR MBEJ."

4. SUPPLY STRIP SEALS CONFORMING TO ASTM D5973. SUBMIT CERTIFIED TEST DATA PER 513.08 FROM THE MANUFACTURER OR AN ACCREDITED LABORATORY. D5973 SECTION 8, LOT SIZE IS ONE SAMPLE PER JOINT. A SAMPLE IS A PIECE 4 FEET LONG WITH ALL MANUFACTURER'S MARKINGS. THE SEAL AND RETAINER ARE AN INTEGRAL SYSTEM SUPPLIED BY ONE MANUFACTURER.

5. SEAL RETAINERS: EXTRUDE, HOT ROLL OR MACHINE, STEEL RETAINERS INTO A SOLID SHAPE. RETAINERS MANUFACTURED FROM BENT PLATE OR BUILT UP PIECES ARE NOT ACCEPTABLE. THE INTERNAL DIMENSION OF THE RETAINER SHALL BE SPECIFIED BY THE MANUFACTURER TO ACHIEVE POSITIVE SEAL ANCHORAGE.

6. SEPERATION BEAMS/TRANSVERSE DIVIDERS/CENTER BEAMS SHALL BE A SOLID, NON WELDED MACHINED OR EXTRUDED STEEL SECTION.

7. LUBRICANT - ADHESIVE, ONE PART MOISTURE CURING POLYURETHANE COMPOUND MEETING THE REQUIREMENTS OF ASTM D4070 AND AS SPECIFIED BY THE SEAL MANUFACTURER.

8. HARDWARE SHALL BE ASTM A325 TYPE 1, GALVANIZED, OR A449 GALVANIZED.

D. FABRICATION:

1. THE MODULAR JOINTS SHALL BE FABRICATED ACCORDING TO CMS 513.

2. SHOP ASSEMBLE THE MODULAR JOINT WITH ALL COMPONENTS EXCEPT, NEOPRENE SEALS, PER 513.24 EXCEPT THAT FULL ASSEMBLY IS REQUIRED WITH PHASED CONSTRUCTION.

3. JOINTS IN STRIP SEALS: NO JOINTS ARE ALLOWED.


4. JOINTS IN RETAINERS: WELDS ARE WATER TIGHT, PARTIAL PENETRATION WELDS AROUND THE OUTER PERIPHERY OF THE ABUTTING SURFACES. MAKE SPLICES ONLY IN COMPRESSION ZONES OF THE JOINT ARMOR. GRIND FLUSH ALL WELDS IN CONTACT WITH THE SEAL AND JOINT ARMOR. DO NOT USE SHORT PIECES OF RETAINERS LESS THAN 6'-0" LONG, UNLESS REQUIRED. AT CURBS OR SIDEWALKS. DO NOT PROVIDE ADDITIONAL SPLICES IN RETAINERS AT CURB OR SIDEWALK SECTIONS OTHER THAN REQUIRED FOR GEOMETRY.

5. SHOP OR FIELD WELDS OR CENTER BEAMS AND JOINT ARMOR, SHALL BE COMPLETE PENETRATION WELDS, GROUND TO PROVIDE SMOOTH TRANSITIONS AND BE 100% ULTRASONICALLY TESTED PER AASHTO/AWS BRIDGE WELDING CODE, WITH TENSION ACCEPTANCE CRITERIA, WITNESSED BY THE DEPARTMENT.

6. CODE, WITH TENSION ACCEPTANCE CRITERIA, WITNESSED BY THE DEPARTMENT. SUPPORT BAR CONNECTIONS SHALL BE COMPLETE PENETRATION WELDS GROUND TO PROVIDE SMOOTH TRANSITIONS AND BE 100% ULTRASONICALLY TESTED PER AASHTO/AWS BRIDGE WELDING CODE, WITH TENSION ACCEPTANCE CRITERIA, WITNESSED BY THE DEPARTMENT.

7. TEMPORARY SUPPORTS: FABRICATOR DESIGNED AND INSTALLED SUPPORTS ARE REQUIRED TO SUPPORT SHIPPING, ERECTION AND CONSTRUCTION FORCES WITHOUT DAMAGE TO THE STEEL ARMOR OR COATINGS. THESE SUPPORTS SHALL BE ADJUSTABLE FOR FIELD TEMPERATURE SETTING.

W:\Projects\Projects F - J\H2530002\22984\structures\HENIND_0000\Sheets\IND_0000CGN002.dgn 5/22/2015 4:04:55 PM SValentin


GENERAL NOTES (2 OF 3)
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER
HEN-IND-00.00
 PID No. 22984
 5 / 64
 89
 180

ITEM 516 SPECIAL: MODULAR EXPANSION JOINT (CONTINUED)

E. COATING:

1. GALVANIZE OR METALIZE ALL STEEL SURFACES AND COMPONENTS, EXCEPT AT STAINLESS STEEL AND PTFE SLIDING SURFACES. THESE COATING MAY BE MIXED ON ONE ASSEMBLY, IF ALL SIMILAR COMPONENTS OF THE ASSEMBLY HAVE THE SAME COATING TYPE.
2. PROVIDE A GALVANIZED COATING PER ASTM A123, WITH A MINIMUM THICKNESS OF 4 MILS. CLEAN EXCESSIVE GALVANIZING AS NECESSARY TO ACHIEVE MECHANICAL MOVEMENT AND SEAL INSTALLATION.
3. PROVIDE A METALIZED COATING PER SOCIETY FOR PROTECTIVE COATINGS (SSPC) SPECIFICATION SSPC-CS23.00 (MARCH 17, 2003) FOR THERMAL SPRAY METALLIC COATINGS. THE COATING SHALL BE A MINIMUM OF 8 MILS THICK. THE METALIZING WIRE SHALL BE 100% ZINC. AREAS OF STRUCTURAL STEEL THAT ARE IN CONTACT WITH CAST-IN-PLACE CONCRETE SHALL HAVE AN ADDITIONAL COATING. THE COATING SHALL BE THE EPOXY INTERMEDIAT COAT SPECIFIED IN CMS 514. THE COATING THICKNESS WILL COVER ALL PEAKS, VALLEYS AND SURFACE ROUGHNESS ATTRIBUTED TO METALIZING.
4. COATING REPAIRS: DAMAGED COATINGS SHALL BE REPAIRED BY ASTM A780, ANNEX "A1. REPAIR USING ZINC BASED ALLOYS". THE PROCEDURE SHALL BE AS FOLLOWS: REMOVE SURFACE CONTAMINATES, PREHEAT TO 600°F, AND APPLY ZINC COATING BY RUBBING WITH A PURE ZINC STICK OR SPRINKLING ZINC POWDER ON THE PREHEATED SURFACE, TO ACHIEVE A MINIMUM COATING THICKNESS OF 6 MILS.
5. THE METALIZED OR GALVANIZED COATINGS SHOULD NOT BE FIELD PAINTED. DAMAGED AREAS SHALL BE METALIZED AS PER 516.03 AND SUPPLEMENTAL SPECIFICATION 845.
6. PRIOR TO SHIPPING, RETAINER GROOVES SHALL BE PROTECTED FROM CONSTRUCTION DEBRIS BY THE INSTALLATION OF BACKER RODS OR OTHER EFFECTIVE MASKING TECHNIQUES.

F. INSTALLATION:

1. A JOINT MANUFACTURER'S TECHNICAL REPRESENTATIVE TO PHYSICALLY OVERSEE THE FABRICATION, INSTALLATION, ADJUSTMENT AND TESTING DURING ALL OPERATIONS. WHERE SPECIAL INSTRUCTIONS ARE NOT CONTAINED HEREIN OR ELSEWHERE IN THESE NOTES, DIRECTION FOR THE INSTALLATION SHALL BE ACCORDING TO THE RECOMMENDATIONS OF THE TECHNICAL REPRESENTATIVE.
2. COORDINATE AND SCHEDULE THE TECHNICAL REPRESENTATIVE.
3. INSTALL THE SUPERSTRUCTURE SUPPORTING UNITS BEFORE INSTALLING THE MODULAR JOINT. POSITION THE JOINT TO MATCH ROADWAY GEOMETRY SUPERSTRUCTURE CONNECTIONS AND TEMPERATURE OPENING. TAKE CARE TO MAINTAIN EXACT ALIGNMENT OF ADJACENT ENDS OF THE ARMOR AND SEPARATION BEAS/TRANSVERSE DIVIDERS/CENTER BEAMS FOR FIELD WELDED UNITS. PROVIDE TEMPORARY SUPPORTS AS DIRECTED BY THE MANUFACTURER TO MAINTAIN THE PROPER POSITIONING. FOR PHASED CONSTRUCTION, THE CONTRACTOR'S METHODS FOR INSTALLATION AND TEMPORARY SUPPORTS SHALL ACHIEVE SEPARATION OF THE PHASES AND UNRESTRICTED TEMPERATURE MOVEMENT.

4. PERFORM CONCRETE PLACEMENT USING VIBRATION AND HAND WORK AS NECESSARY TO ACHIEVE CONSOLIDATION AND ELIMINATE AIR VOIDS. THE MAXIMUM AGGREGATE SIZE SHALL BE #8 FOR CONCRETE BLOCKOUT AREAS.
5. PLACE THE DECK CONCRETE FIRST. CHECK THE ABUTMENT OR ADJACENT SPAN SIDE OF THE MODULAR JOINT FOR ALIGNMENT AND TEMPERATURE ADJUSTMENT. THE TEMPERATURE SHALL BE MEASURED AT THE UNDERSIDE OF THE CONCRETE DECK AT EACH END AND MID-SPAN TO ACHIEVE THE AVERAGE SUPERSTRUCTURE TEMPERATURE. PLACE THE BACKWALL OR ADJACENT SPAN CONCRETE SECOND. THE MANUFACTURER'S REPRESENTATIVE SHALL CHECK THAT TEMPERATURE MOVEMENT HAS NOT CAUSED ANY DAMAGE TO THE BOND BETWEEN THE JOINT AND THE CONCRETE.
6. EXAMINE SEAL RETAINERS FOR SOIL OR DEFECTS THAT CAN DAMAGE THE SEAL. REPAIR ANY DEFECTS AS DIRECTED BY THE MANUFACTURER'S REPRESENTATIVE.
7. SOLVENT CLEAN THE NEOPRENE SEAL ELEMENTS AND THE RETAINER GROOVES TO REMOVE OIL, GREASE OR OTHER SOIL IMMEDIATELY PRIOR TO INSTALLING THE SEALS. INSTALL SEALS USING PROCEDURES AND ADHESIVE SPECIFIED BY THE JOINT MANUFACTURER. KEEP THE BONDING SURFACES CLEAN, DRY AND WARMER THAN 45°F.
8. TEST THE INSTALLED MODULAR JOINT FOR LEAKS. FLOOD THE TOTAL EXPANSION JOINT LENGTH WITH WATER FOR A PERIOD OF NOT LESS THAN ONE HOUR. COVER THE ENTIRE JOINT SYSTEM BY EITHER PONDING OR FLOWING WATER. LOCATE ANY POINTS OR LEAKAGE AND TAKE ANY AND ALL MEASURES NECESSARY TO STOP THE LEAKAGE. PERFORM THIS WORK AT THE CONTRACTOR'S EXPENSE. PERFORM A SECOND WATER TEST AFTER ALL REPAIRS HAVE BEEN MADE.

ITEM 524 DRILLED SHAFTS, AS PER PLAN

DRILLED SHAFTS:
 THE MAXIMUM FACTORED LOAD TO BE SUPPORTED BY EACH DRILLED SHAFT IS 1367 KIPS AT PIERS 1 - 6 AND 1407 KIPS AT THE PIER 7. THIS LOAD IS RESISTED BY SIDE RESISTANCE WITHIN A PORTION OF THE BEDROCK SOCKET AND ALSO BY TIP RESISTANCE. THE FACTORED RESISTANCE DEVELOPED BY SIDE RESISTANCE IS 5.5 KSF, ASSUMED TO ACT ALONG THE BOTTOM 8 FEET OF THE BEDROCK SOCKET FOR THE PIERS. THE FACTORED RESISTANCE PROVIDED BY THE DRILLED SHAFT TIP IS 91.5 KSF.

FOR HOLE EXCAVATION SEE C&MS 524.04, CASE D PERMANENT CASING CONSTRUCTION METHOD SHALL BE USED TO CONSTRUCT PIERS 1 THRU 7.

SUPPLY ALL REQUIRED EQUIPMENT AND PERSONNEL NECESSARY TO PERFORM VIDEO INSPECTION OF THE DRILLED SHAFT EXCAVATION, INCLUDING THE INSPECTIONS PERFORMED UNDERWATER OR WITHIN SLURRY. PROVIDE EQUIPMENT CAPABLE OF THE FOLLOWING: MEASURING THE DEPTH OF LOOSE OR DISTURBED MATERIAL AT THE BOTTOM OF THE SHAFT, AND RECORDING COLOR VIDEO IMAGES OF THE INSPECTION TO A DVD OR VIDEOTAPE. FURNISH ALL NECESSARY SUPPLIES, FUEL AND ELECTRIC SERVICE TO OPERATE THE EQUIPMENT. PERFORM THE VIDEO INSPECTION IMMEDIATELY BEFORE POURING THE CONCRETE, AND IN THE PRESENCE OF THE ENGINEER OR INSPECTOR. SUBMIT RECORDINGS OF ALL VIDEO INSPECTIONS TO THE ENGINEER AFTER COMPLETING ALL VIDEO INSPECTIONS, OR WHEN REQUESTED BY THE ENGINEER.

PAYMENT IS FULL COMPENSATION FOR SUPPLYING THE REQUIRED EQUIPMENT AND PERSONNEL, AND FOR PERFORMING THE VIDEO INSPECTION OF THE DRILLED SHAFT EXCAVATIONS. THE DEPARTMENT WILL PAY FOR ACCEPTED QUANTITIES AT THE LUMP SUM CONTRACT PRICE FOR ITEM 524 DRILLED SHAFT, 66" DIAMETER, INTO BEDROCK, AS PER PLAN

ABBREVIATIONS:

ABUT.	- ABUTMENT	MAX.	- MAXIMUM
APPR.	- APPROACH	MIN.	- MINIMUM
APPROX.	- APPROXIMATELY	NF	- NEAR FACE
BRG.	- BEARING	NS	- NEAR SIDE
BOT.	- BOTTOM	NO.	- NUMBER
BTWN.	- BETWEEN	N.P.C.P.P.	- NON-PERFORATED CORRUGATED PLASTIC PIPE
C/C	- CENTER TO CENTER	O/O	- OUT TO OUT
CJ	- CONSTRUCTION JOINT	P.C.P.P.	- PERFORATED CORRUGATED PLASTIC PIPE
CLR.	- CLEAR	P.E.J.F.	- PREFORMED EXPANSION JOINT FILLER
CONST.	- CONSTRUCTION	PG	- PROFILE GRADE
DIA.	- DIAMETER	PROP.	- PROPOSED
DWG.	- DRAWING	RA	- REAR ABUTMENT
EF	- EACH FACE	RT.	- RIGHT
EL.	- ELEVATION	SER	- SERIES
EQ.	- EQUAL	SHLDR.	- SHOULDER
EXP.	- EXPANSION	SPA.	- SPACE OR SPACES
FA	- FORWARD ABUTMENT	STA.	- STATION
FF	- FAR FACE	STD.	- STANDARD
FS	- FAR SIDE	STR	- STRAIGHT
FWD.	- FORWARD	T	- TOP
INV.	- INVERT	TEMP.	- TEMPORARY
JT.	- JOINT	THK.	- THICK
JTS.	- JOINTS	T.O.S.	- TOP OF SLOPE
LT.	- LEFT	T/T	- TOE TO TOE
		TYP.	- TYPICAL
		U.N.O.	- UNLESS NOTED OTHERWISE
		VAR.	- VARIES



DESIGNED	DRH	CHECKED	SCT
DRAWN	RJS	REVISED	
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	05/2015		

GENERAL NOTES (3 OF 3)
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
 PID No. 22984



ESTIMATED QUANTITIES

CALCULATED: RJS DATE: 5-13-15

CHECKED: DRH DATE: 5-15-15

ITEM	EXTENSION	TOTAL	UNIT	DESCRIPTION	ABUT.	PIERS	SUPER.	GEN.	SHEET #
503	11100	LUMP		COFFERDAMS AND EXCAVATION BRACING					
503	21301	LUMP		UNCLASSIFIED EXCAVATION, AS PER PLAN					4
505	11100	LUMP		PILE DRIVING EQUIPMENT MOBILIZATION					
507	00200		FT	STEEL PILES HP12X53, FURNISHED					
507	00250		FT	STEEL PILES HP12X53, DRIVEN					
509	10001		POUND	EPOXY COATED REINFORCING STEEL, AS PER PLAN					4
511	21543		CU YD	CLASS QC2 CONCRETE WITH QC/QA, SUPERSTRUCTURE, AS PER PLAN (DECK)					4,5
511	33418		CU YD	CLASS QC2 CONCRETE WITH QC/QA, SUPERSTRUCTURE (DIAPHRAGMS)					
511	34450		CU YD	CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET)					
511	41013		CU YD	CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS, AS PER PLAN					21
511	43512		CU YD	CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT INCLUDING FOOTING					
511	51512		CU YD	CLASS QC2 CONCRETE WITH QC/QA, SIDEWALK					
512	10100		SQ YD	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)					
512	33000		SQ YD	TYPE 2 WATERPROOFING					
513	10201		LB	STRUCTURAL STEEL MEMBERS, LEVEL UF, AS PER PLAN					5
515	15130		EACH	DRAPED STRAND PRESTRESSED CONCRETE BRIDGE I-BEAM MEMBERS, LEVEL 3, TYPE WF72-49					
515	20001		EACH	INTERMEDIATE DIAPHRAGMS, AS PER PLAN					
SPECIAL	51612400		FT	MODULAR EXPANSION JOINT					5,6
516	13600		SQ FT	1" PREFORMED EXPANSION JOINT FILLER					
516	13900		SQ FT	2" PREFORMED EXPANSION JOINT FILLER					
516	44100		EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES (14"x22"x2.36") AND LOAD PLATE (15"x41"x2") (NEOPRENE)					
516	44100		EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES (14"x22"x2.36") AND LOAD PLATE (15"x41"x2.1875") (NEOPRENE)					
516	44200		EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES (14.5"x25"x3.39") AND LOAD PLATE (15.5"x41"x2") (NEOPRENE)					
517	75120		FT	RAILING (CONCRETE PARAPET WITH TWIN STEEL TUBE RAILING)					
518	12001		EACH	SCUPPERS INCLUDING SUPPORTS, AS PER PLAN					59
518	21200		CU YD	POROUS BACKFILL WITH FILTER FABRIC					
518	40000		FT	6" PERFORATED CORRUGATED PLASTIC PIPE					
518	40010		FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS					
518	43300		FT	6" PIPE DOWNSPOUT, INCLUDING SPECIALS					
524	94947		FT	DRILLED SHAFTS, 72" DIAMETER, ABOVE BEDROCK, AS PER PLAN					6
524	94935		FT	DRILLED SHAFTS, 66" DIAMETER, INTO BEDROCK, AS PER PLAN					6
526	30001		SQ YD	REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=17"), AS PER PLAN					57,58
526	90010		FT	TYPE A INSTALLATION					
611	04600		FT	12" CONDUIT, TYPE C, 707.45					
846	00100		FT	POLYMER MODIFIED ASPHALT EXPANSION JOINT SYSTEM					

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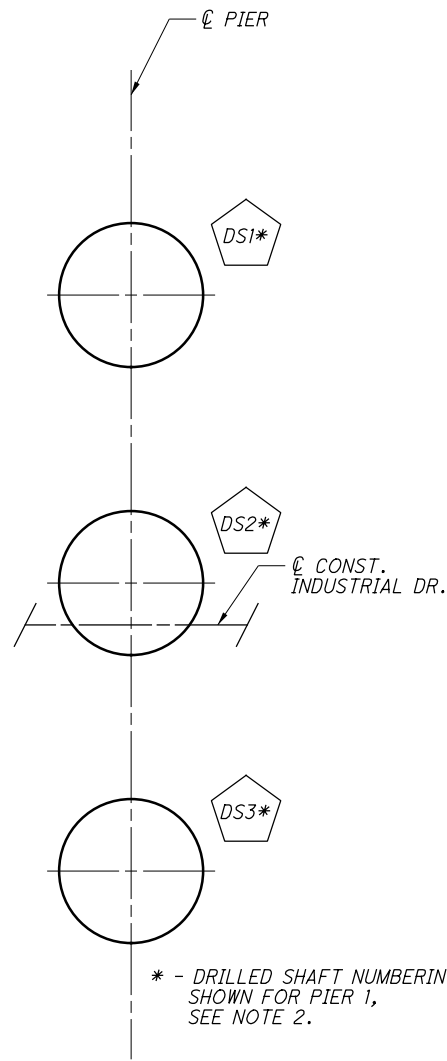
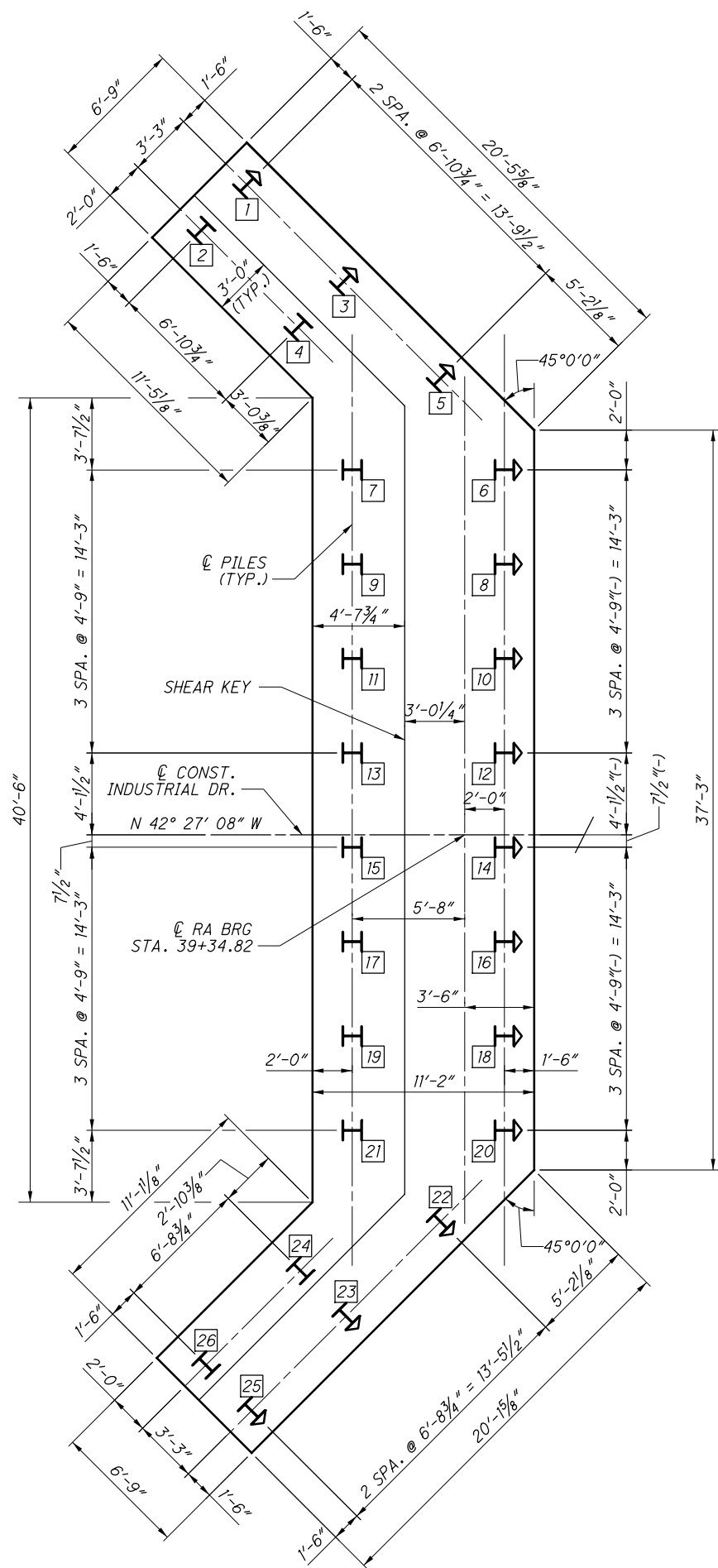


DATE: 05/2015
 REVIEWED: TLR
 STRUCTURE FILE NUMBER: TBD

DESIGNED: RJS
 CHECKED: SCT

ESTIMATED QUANTITIES
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
 PID No. 22984



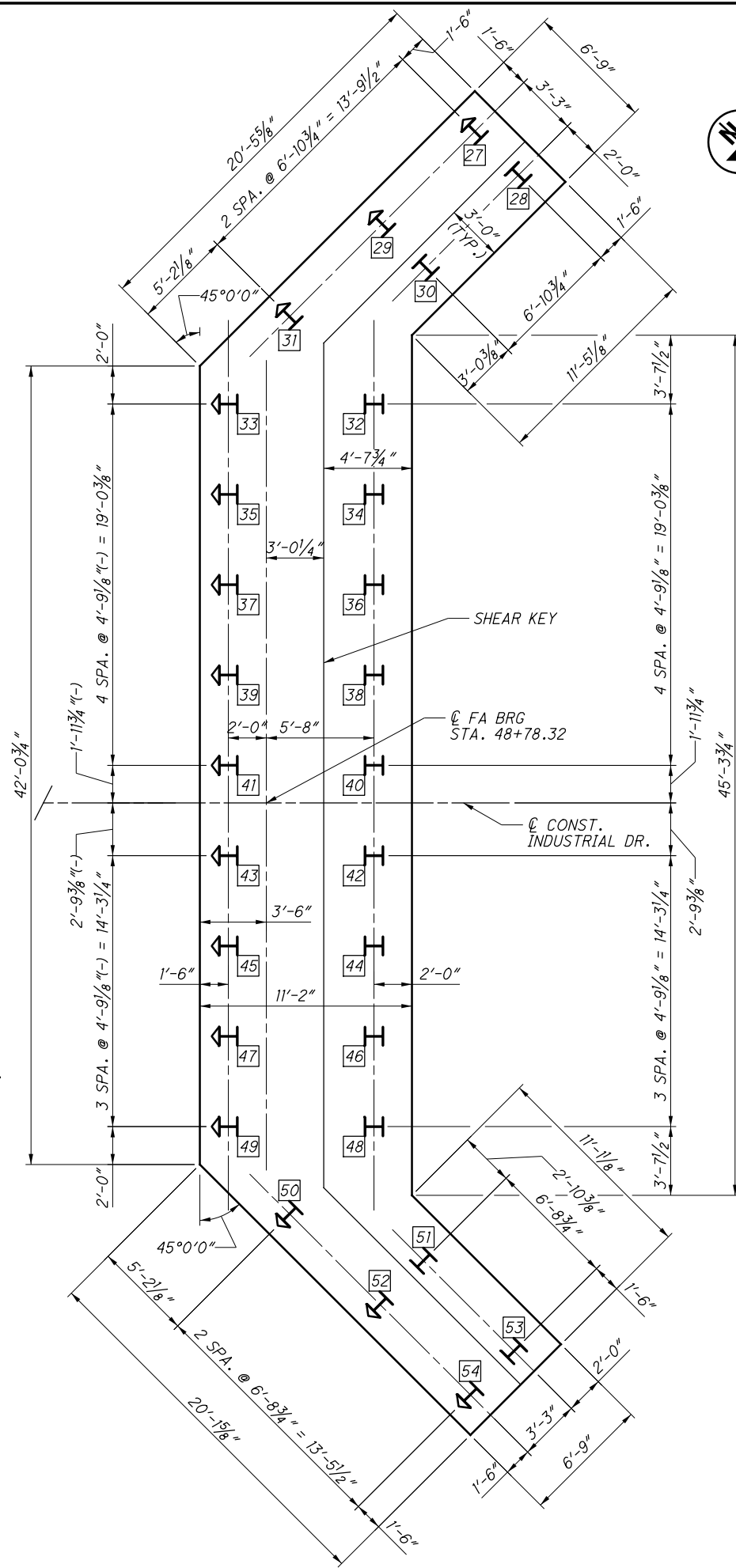
* - DRILLED SHAFT NUMBERING SHOWN FOR PIER 1, SEE NOTE 2.

NOTES:

1. FOR PILE AND DRILLED SHAFT CAPACITY, SEE SHEETS 4/64 AND 6/64
2. FOR DRILLED SHAFT NUMBERING AND LOCATIONS, SEE SHEETS 9-10/64

LEGEND:

- # - PILE NUMBER
- DS1* - DRILLED SHAFT NUMBER
- ⇓ - HP12x53 DRIVEN AT 3:1 BATTER IN DIRECTION SHOWN
- I - HP12x53 STRAIGHT



FOUNDATION PLAN

HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

8/64

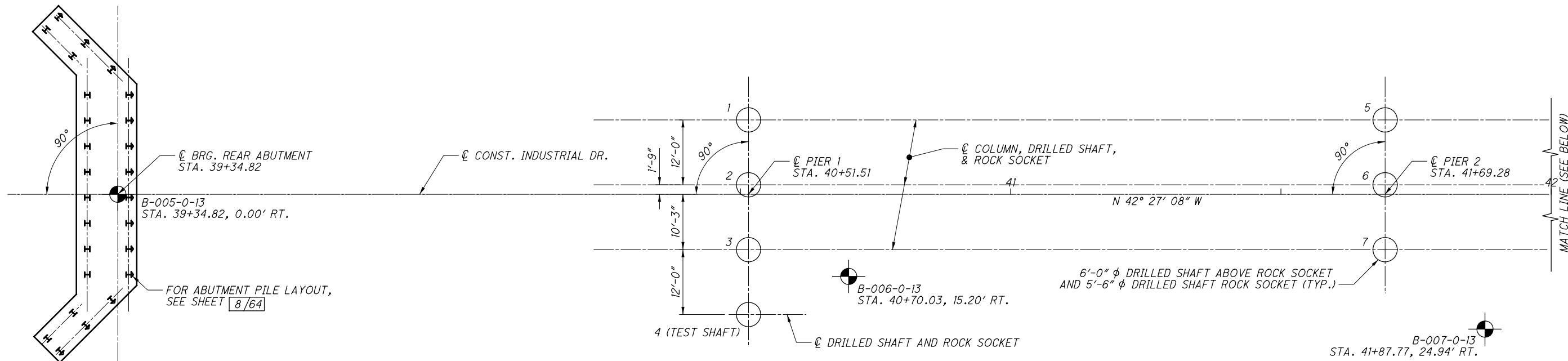
92/180

DESIGNED	CRH	CHECKED	SCT
DRAWN	KRH	REVISED	
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	05/2015		

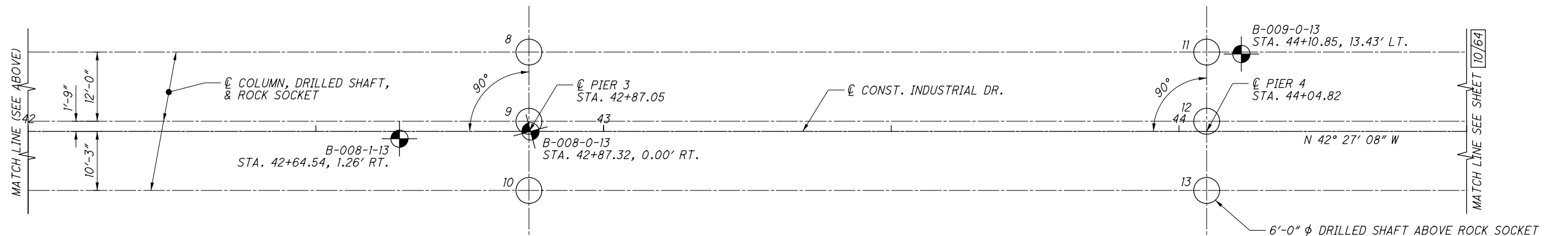


1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

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PART FOUNDATION PLAN



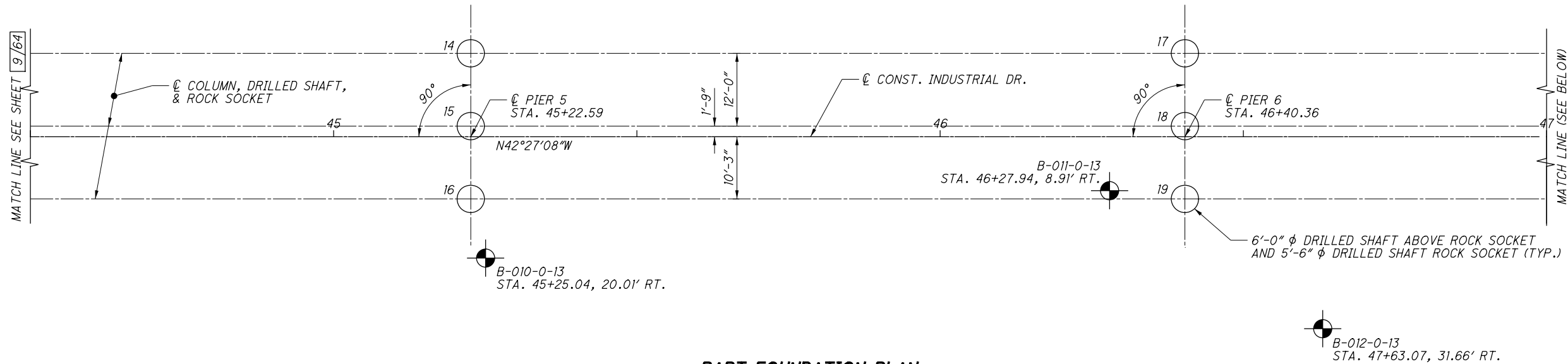
PART FOUNDATION PLAN

NOTES:

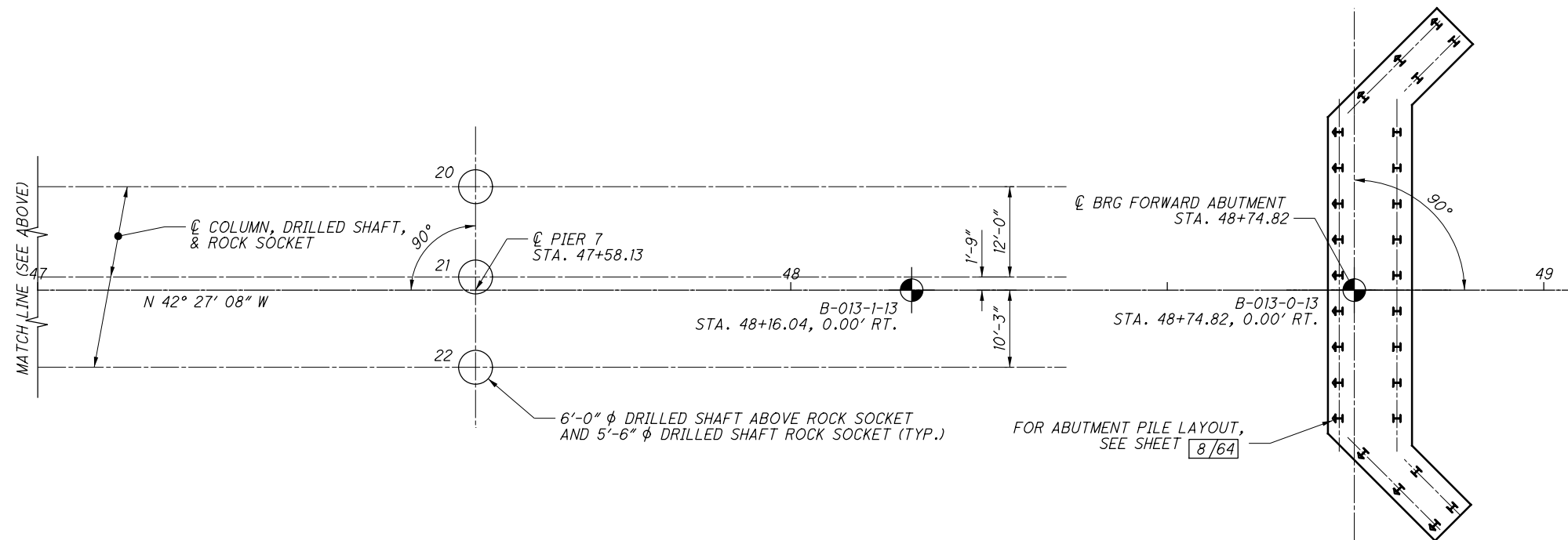
- 7 ○ DENOTES DRILLED SHAFT LOCATION AND NUMBER.
- DENOTES SOIL BORING AND NUMBER

	1800 INDIAN WOOD CIRCLE MAUMEE, OHIO 43537
HEN-IND-00.00 PID No. 22984	PIER FOUNDATION LAYOUT (1 OF 2) HEN-INDUSTRIAL DRIVE-0000 INDUSTRIAL DRIVE OVER MAUMEE RIVER
9/64	DATE: 05/2015 TLR: TLR STRUCTURE FILE NUMBER: TBD
DESIGNED: CMZ CHECKED: CMZ	DRAWN: JEC REVISED:

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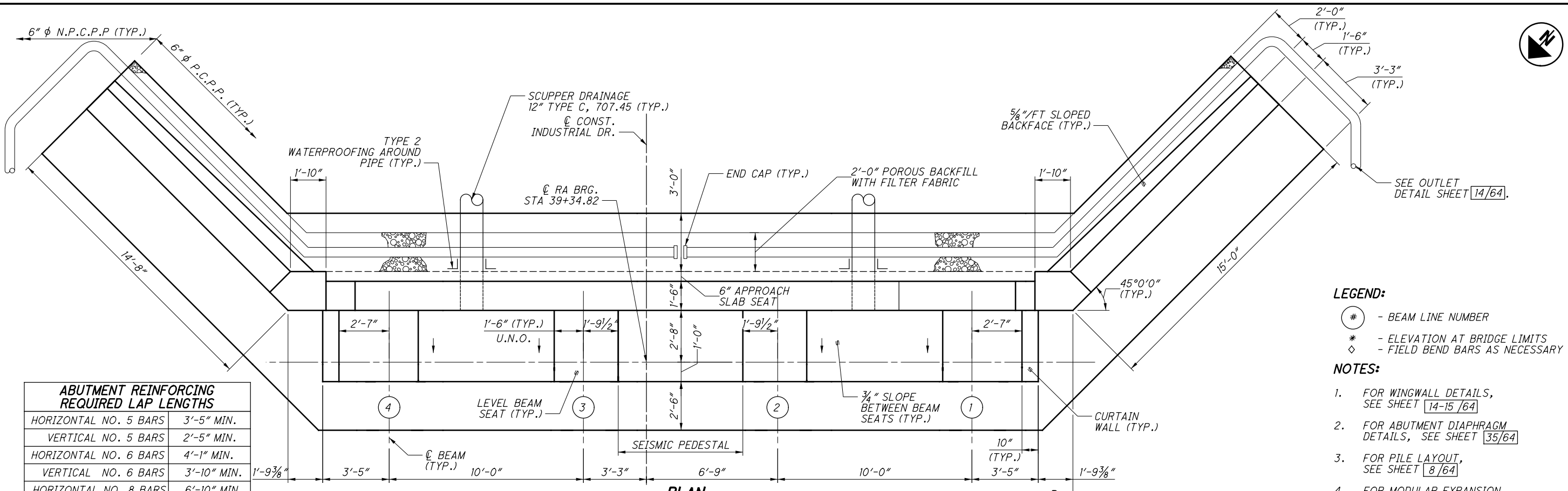
PART FOUNDATION PLAN



PART FOUNDATION PLAN

- NOTES:**
- 19 DENOTES DRILLED SHAFT LOCATION AND NUMBER.
 - DENOTES SOIL BORING AND NUMBER

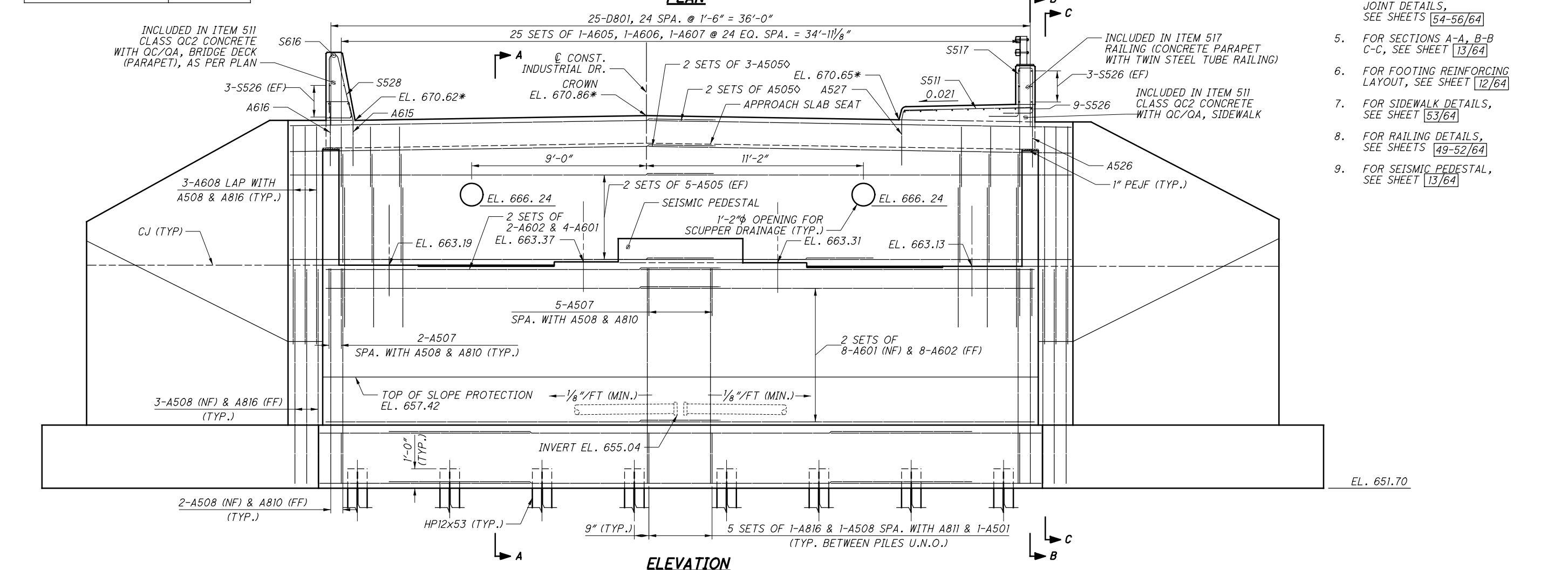
	1800 INDIAN WOOD CIRCLE MAUMEE, OHIO 43537
PIER FOUNDATION LAYOUT (2 OF 2) HEN-INDUSTRIAL DRIVE-0000 INDUSTRIAL DRIVE OVER MAUMEE RIVER	DATE: 05/2015 REVIEWED: TLR DRAWN: JEC CHECKED: CMZ DESIGNED: CMZ STRUCTURE FILE NUMBER: TBD
HEN-IND-00.00 PID No. 22984	10/64 94/180



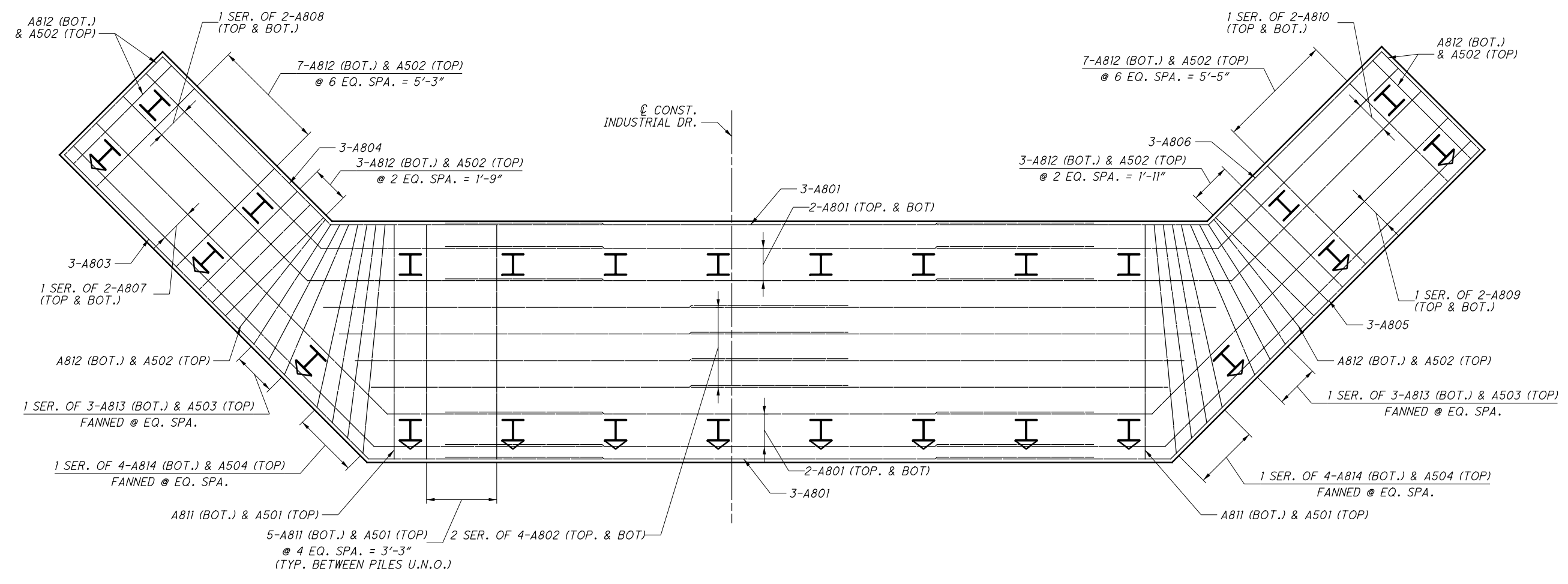
ABUTMENT REINFORCING REQUIRED LAP LENGTHS

HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

- LEGEND:**
- # - BEAM LINE NUMBER
 - * - ELEVATION AT BRIDGE LIMITS
 - ◇ - FIELD BEND BARS AS NECESSARY
- NOTES:**
- FOR WINGWALL DETAILS, SEE SHEET [14/64]
 - FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET [35/64]
 - FOR PILE LAYOUT, SEE SHEET [8/64]
 - FOR MODULAR EXPANSION JOINT DETAILS, SEE SHEETS [54-56/64]
 - FOR SECTIONS A-A, B-B C-C, SEE SHEET [13/64]
 - FOR FOOTING REINFORCING LAYOUT, SEE SHEET [12/64]
 - FOR SIDEWALK DETAILS, SEE SHEET [53/64]
 - FOR RAILING DETAILS, SEE SHEETS [49-52/64]
 - FOR SEISMIC PEDESTAL, SEE SHEET [13/64]



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REAR ABUTMENT FOOTING REINFORCING PLAN

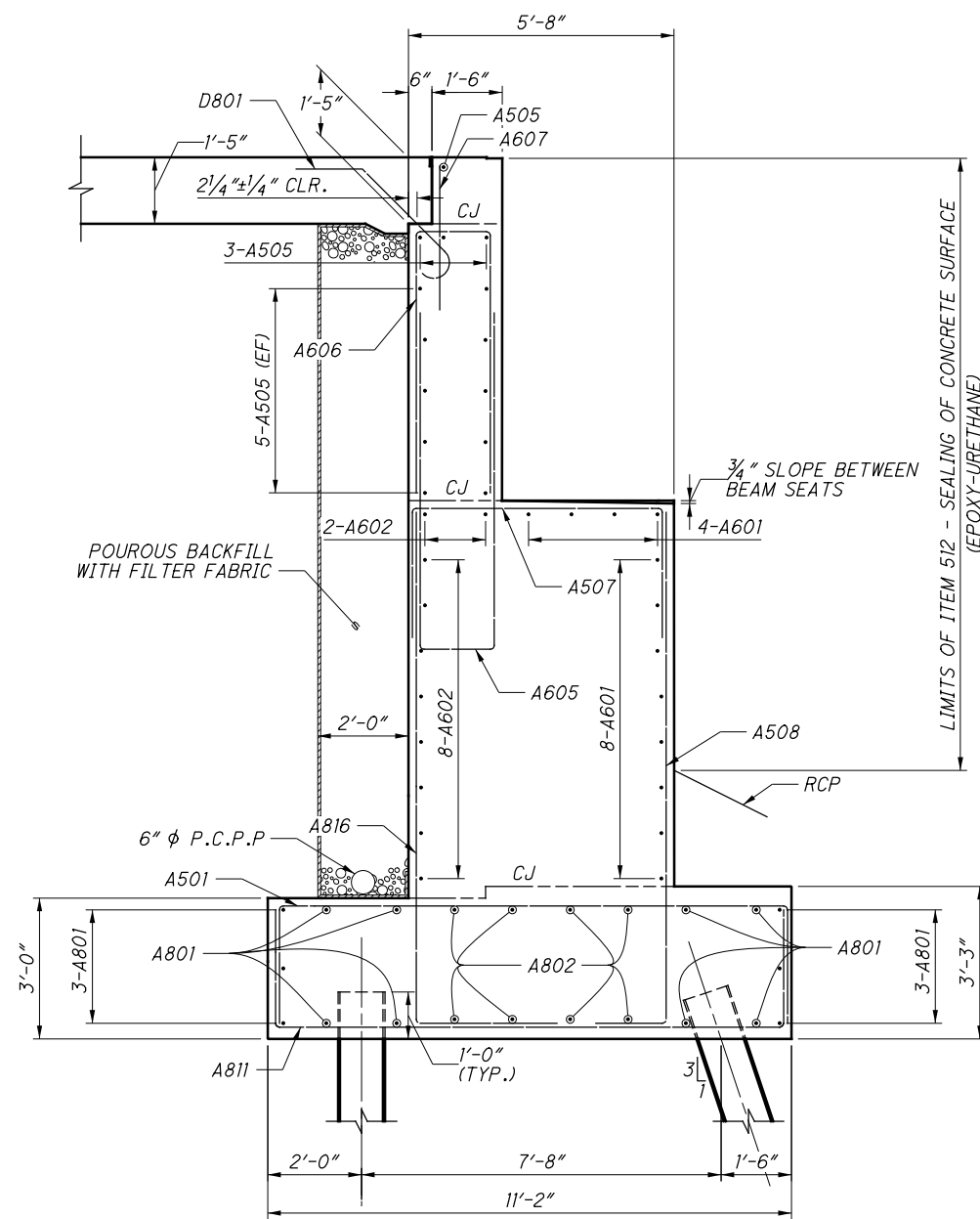
NOTES:

1. FOR ABUTMENT PLAN AND ELEVATION, SEE SHEET **11/64**
2. FOR PILE LAYOUT, SEE SHEET **8/64**

ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

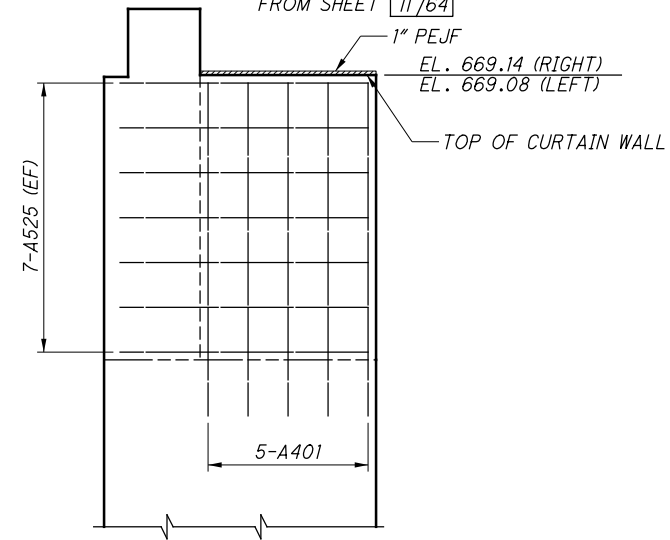
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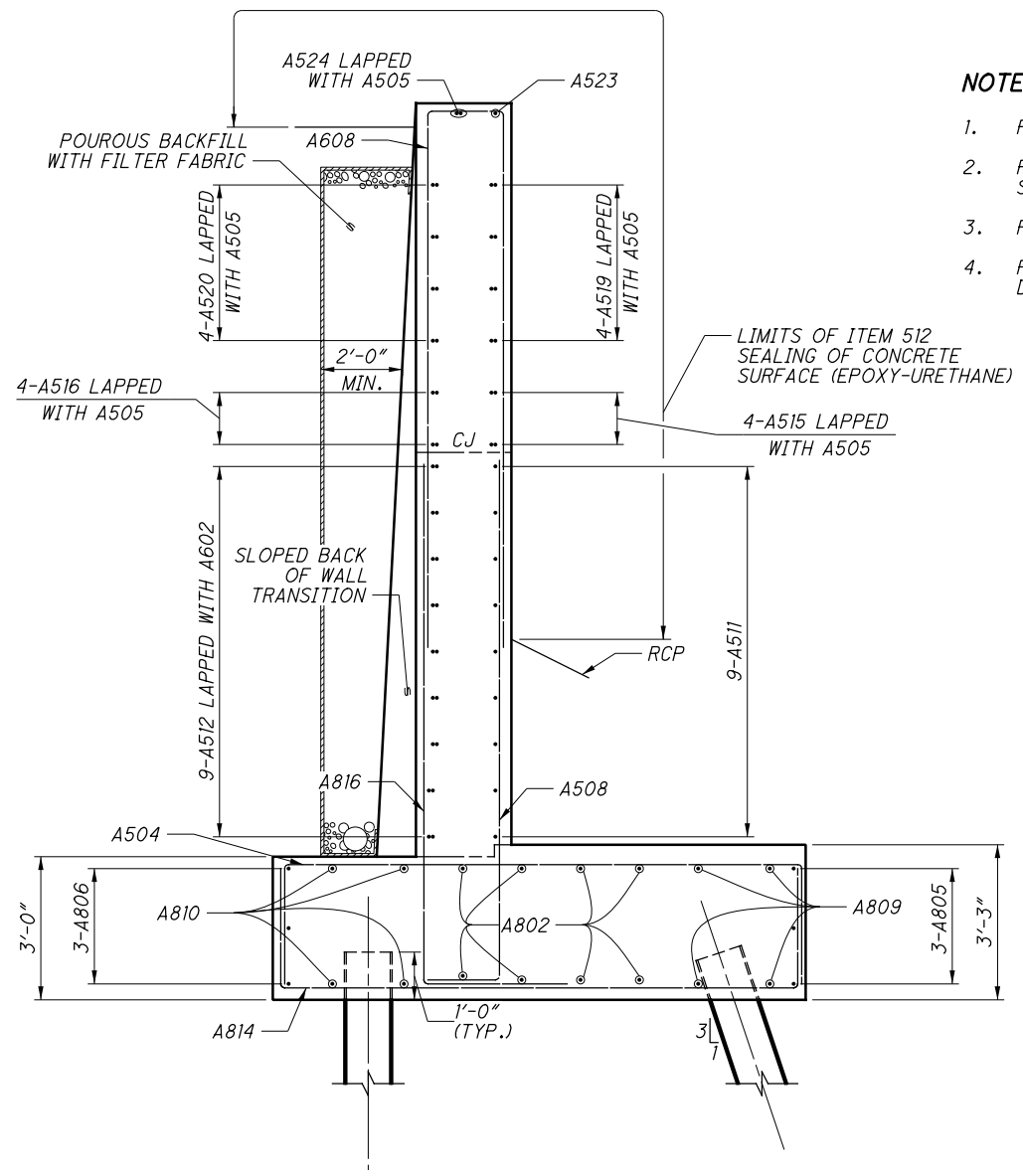
SECTION A-A

FROM SHEET 11/64



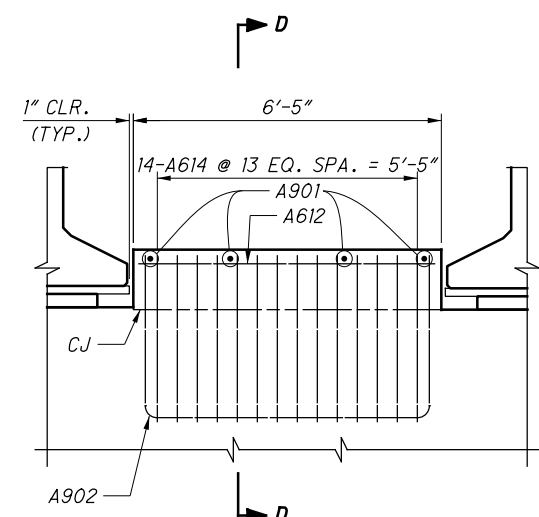
SECTION B-B

FROM SHEET 11/64

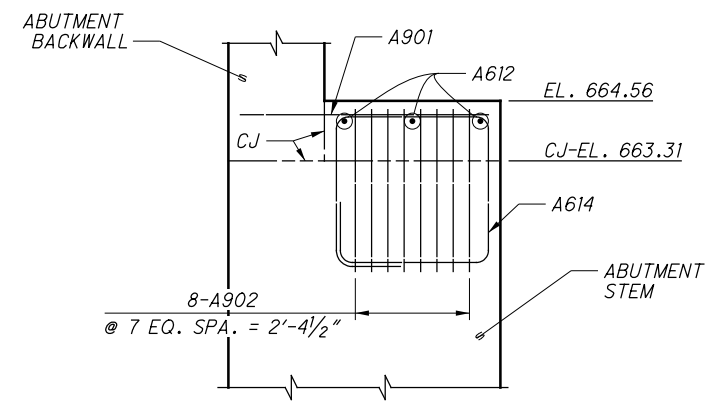


SECTION C-C

FROM SHEET 11/64



SEISMIC PEDISTAL



SECTION D-D

NOTES:

1. FOR WINGWALL DETAILS, SEE SHEET 14-15/64
2. FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET 35/64
3. FOR PILE LAYOUT, SEE SHEET 8/64
4. FOR MODULAR EXPANSION JOINT (NOT SHOWN) DETAILS, SEE SHEETS 54-56/64

ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

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REVIEWED

TLR

STRUCTURE FILE NUMBER

TBD

DATE

05/2015

INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00

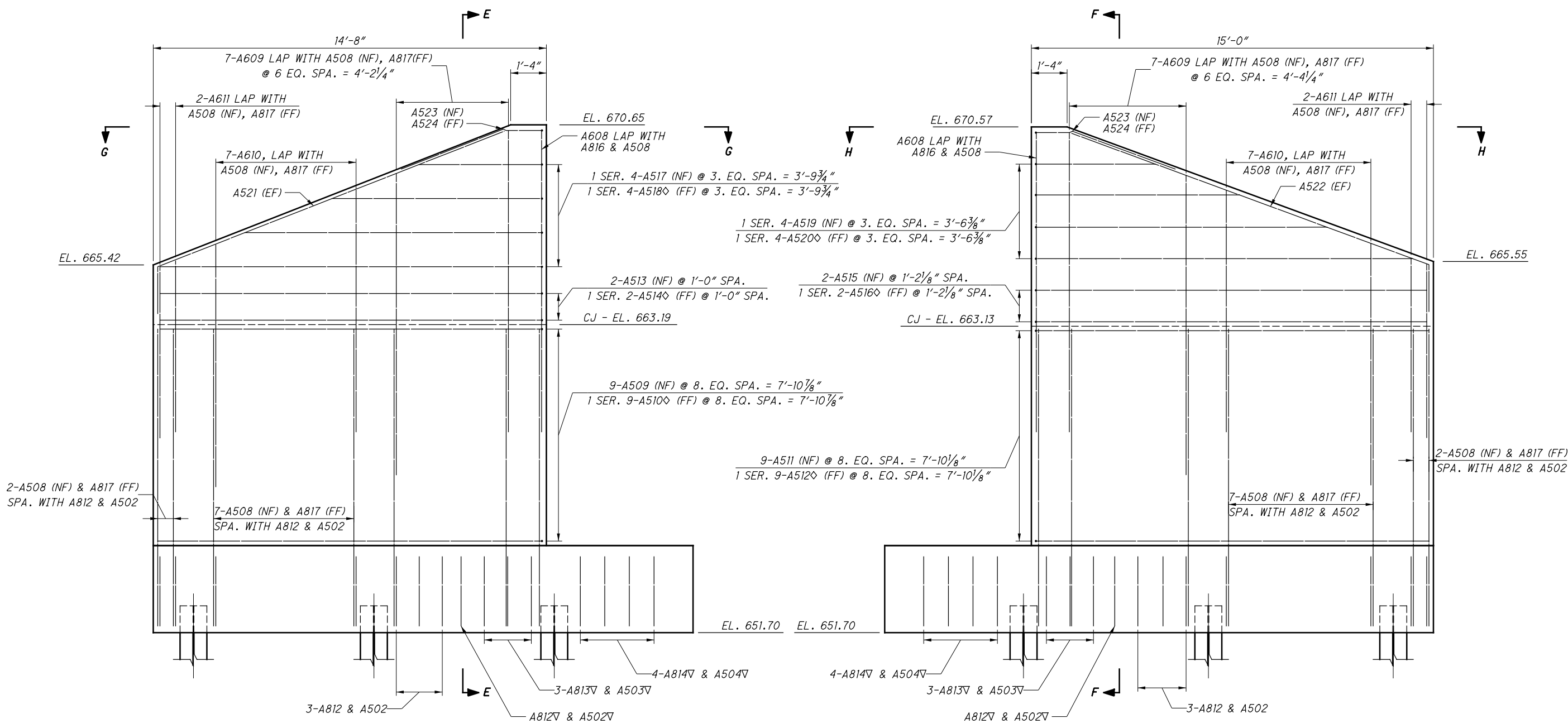
PID No. 22984

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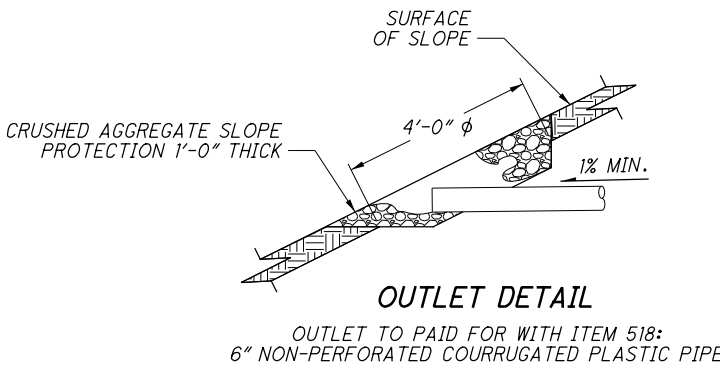


REAR RIGHT WINGWALL

REAR LEFT WINGWALL

▽ - INDICATES FANNED BAR
SEE SHEET 12/64 FOR DETAILS

◇ - INDICATES POSSIBLE NECESSITY
FOR FIELD BEND TO MATCH
BATTERED BACK FACE



- NOTES:**
- FOR ABUTMENT DETAILS, SEE SHEET 11/64
 - FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET 35/64
 - FOR PILE LAYOUT, SEE SHEET 8/64
 - FOR MODULAR EXPANSION JOINT DETAILS, SEE SHEETS 54-56/64
 - FOR SECTIONS E-E & F-F AND VIEWS G-G & H-H, SEE SHEET 15/64

ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

DESIGNED	CRH	CHECKED	SCT
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REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	05/2015		

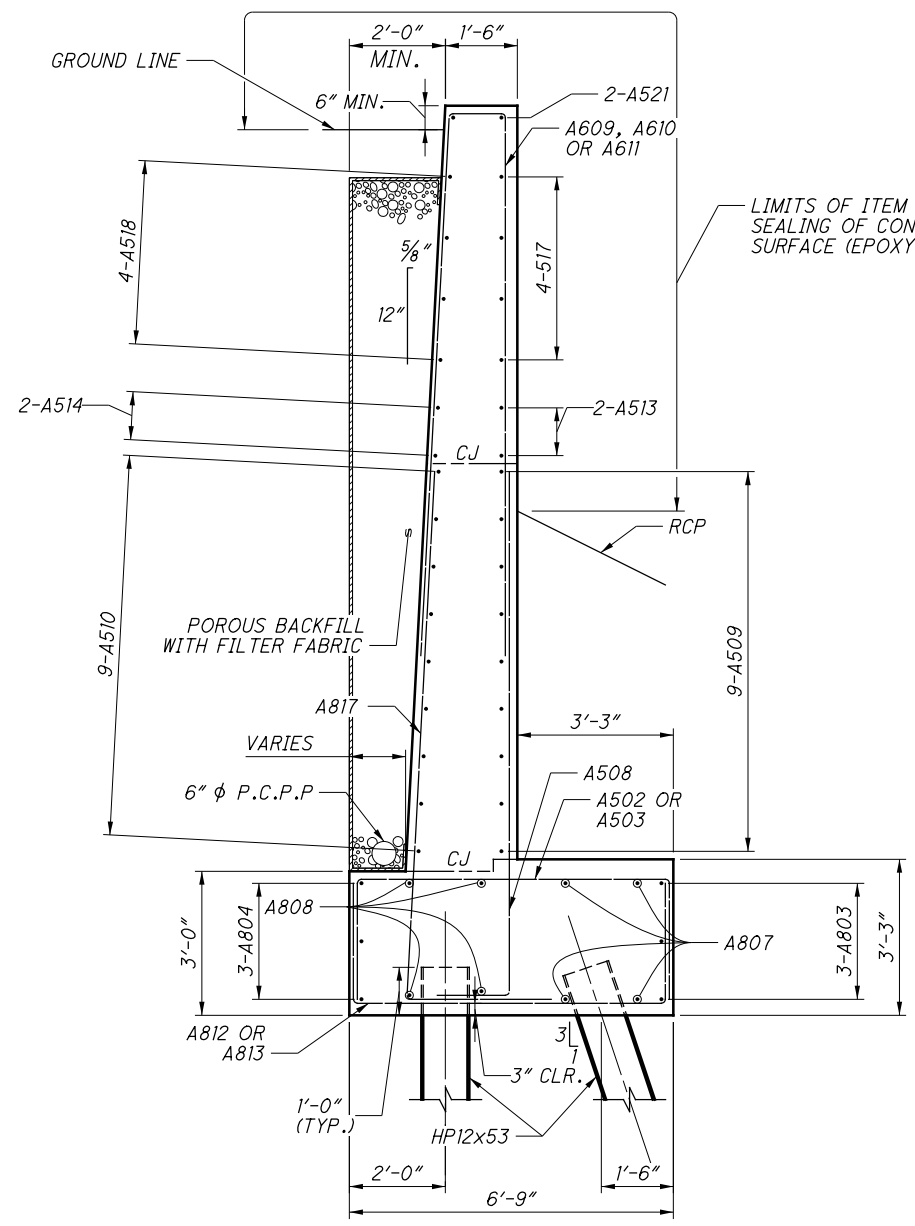
REAR WINGWALL DETAILS
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

14 / 64

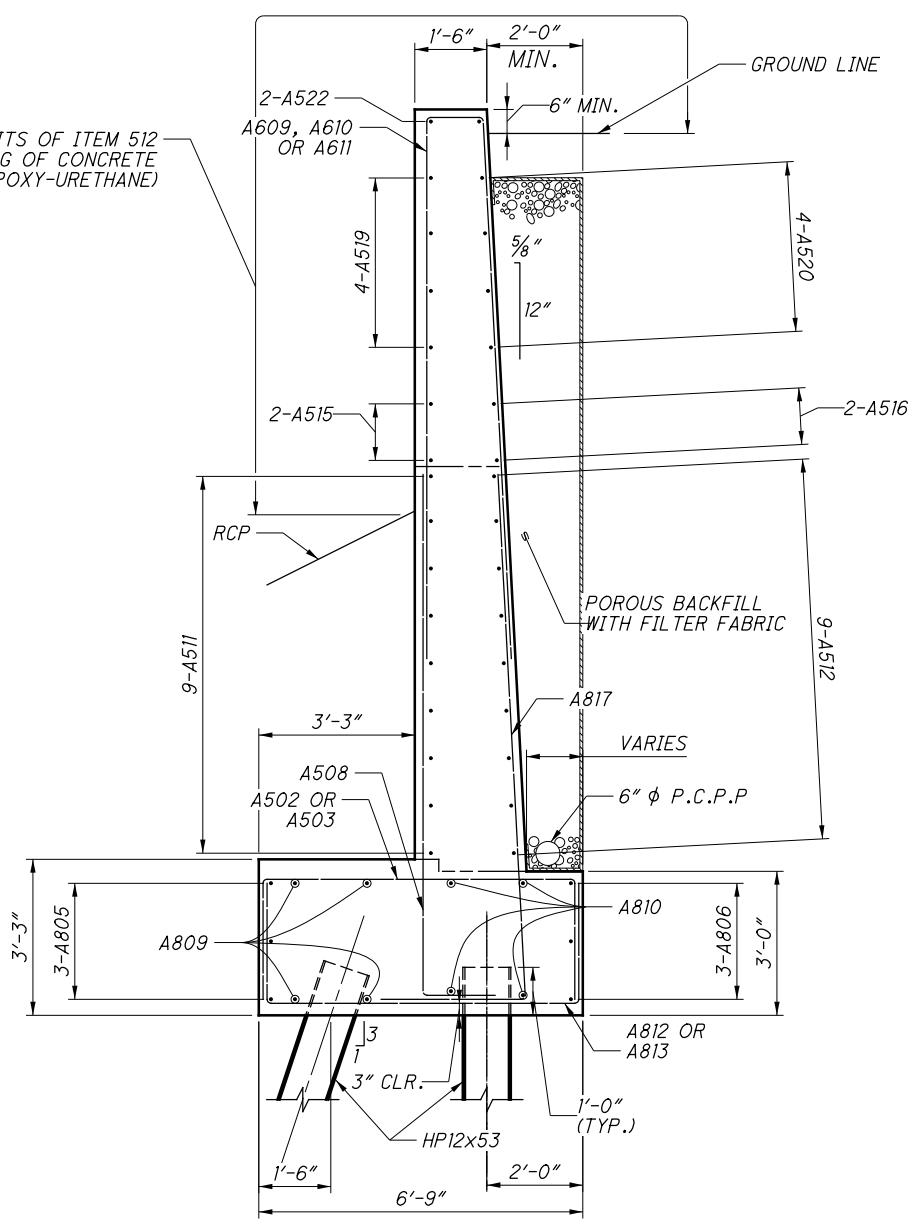
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SECTION E-E
FROM SHEET 14/64

LIMITS OF ITEM 512
SEALING OF CONCRETE
SURFACE (EPOXY-URETHANE)

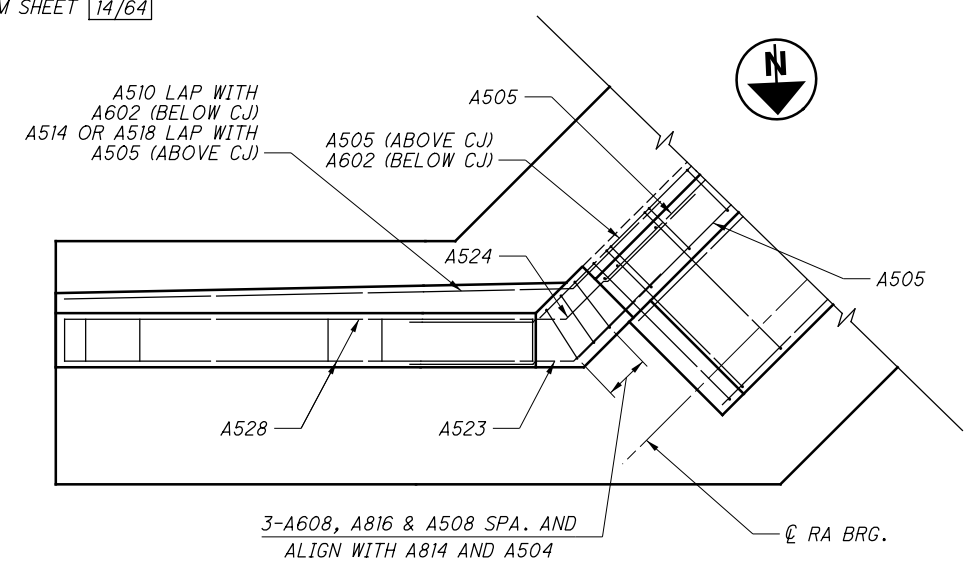


SECTION F-F
FROM SHEET 14/64

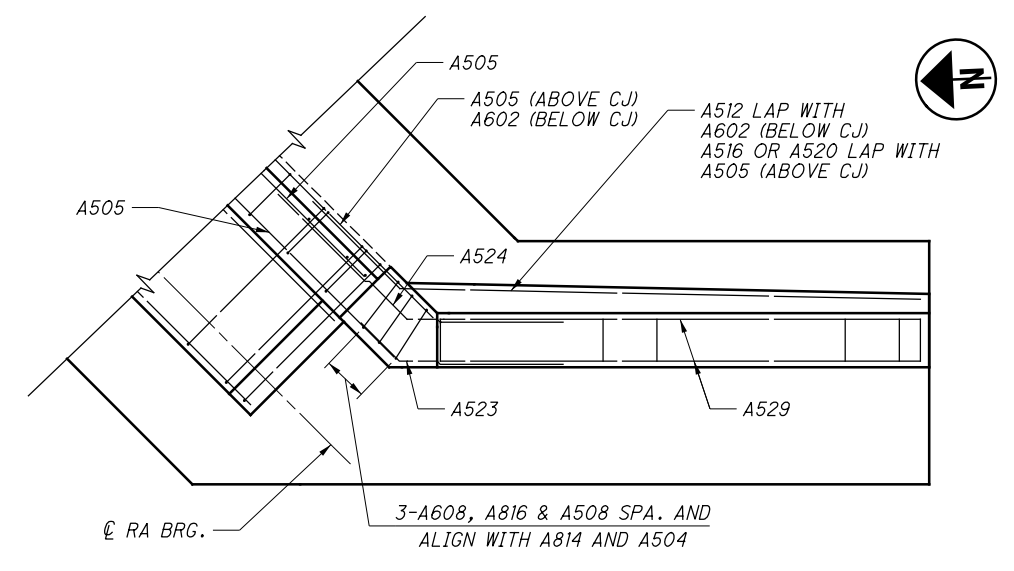
NOTES:

1. FOR ABUTMENT DETAILS, SEE SHEET 11/64
2. FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET 38/64
3. FOR PILE LAYOUT, SEE SHEET 8/64
4. FOR MODULAR EXPANSION JOINT DETAILS, SEE SHEETS 54-56/64

ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.



VIEW G-G
FROM SHEET 14/64



VIEW H-H
FROM SHEET 14/64

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

DESIGNED: KRH
CHECKED: SCT

DATE: 05/2015
FILE NUMBER: TBD

REAR WINGWALL DETAILS

HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00

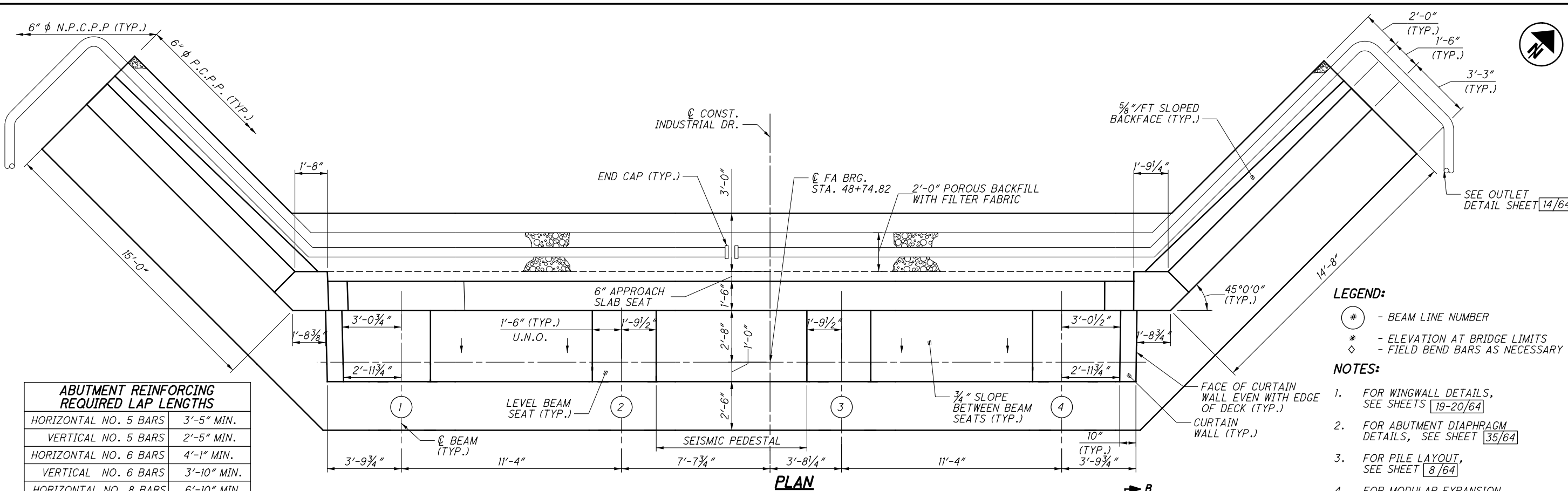
PID No. 22984

15/64

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180



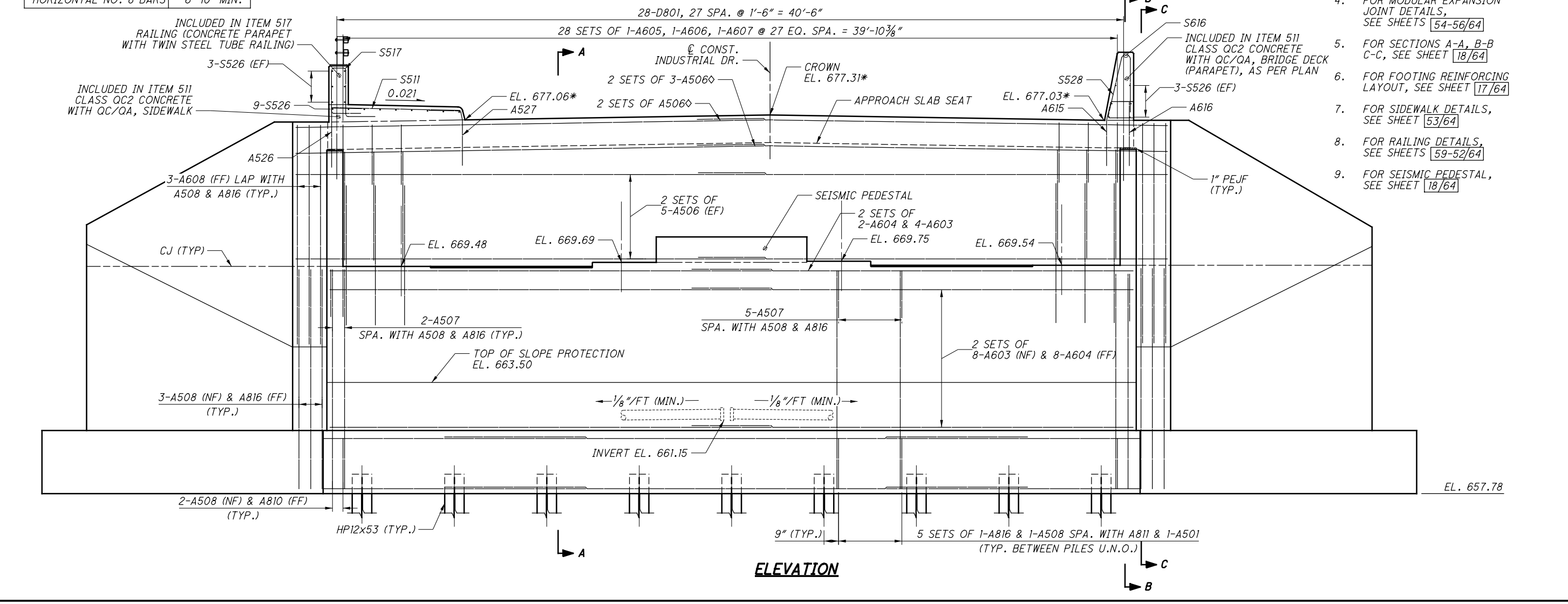
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ABUTMENT REINFORCING REQUIRED LAP LENGTHS

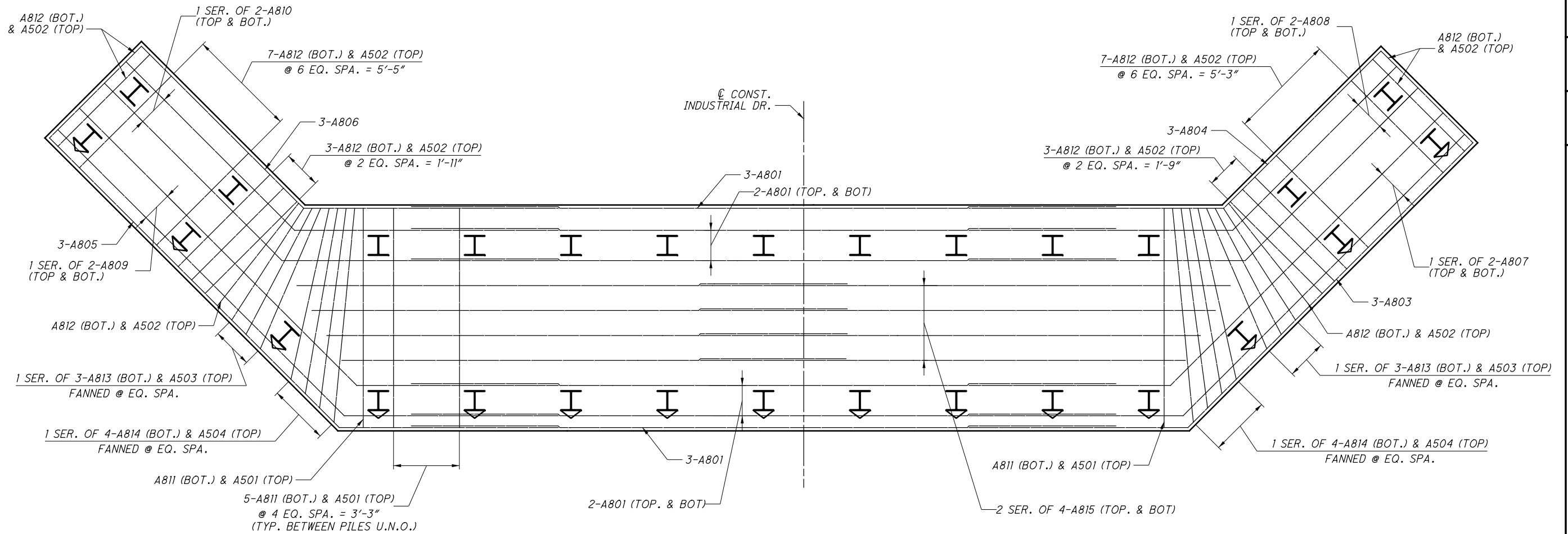
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

- LEGEND:**
- # - BEAM LINE NUMBER
 - * - ELEVATION AT BRIDGE LIMITS
 - ◇ - FIELD BEND BARS AS NECESSARY
- NOTES:**
- FOR WINGWALL DETAILS, SEE SHEETS 19-20/64
 - FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET 35/64
 - FOR PILE LAYOUT, SEE SHEET 8/64
 - FOR MODULAR EXPANSION JOINT DETAILS, SEE SHEETS 54-56/64
 - FOR SECTIONS A-A, B-B C-C, SEE SHEET 18/64
 - FOR FOOTING REINFORCING LAYOUT, SEE SHEET 17/64
 - FOR SIDEWALK DETAILS, SEE SHEET 53/64
 - FOR RAILING DETAILS, SEE SHEETS 59-52/64
 - FOR SEISMIC PEDESTAL, SEE SHEET 18/64



ELEVATION

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FORWARD ABUTMENT FOOTING REINFORCING PLAN

NOTES:

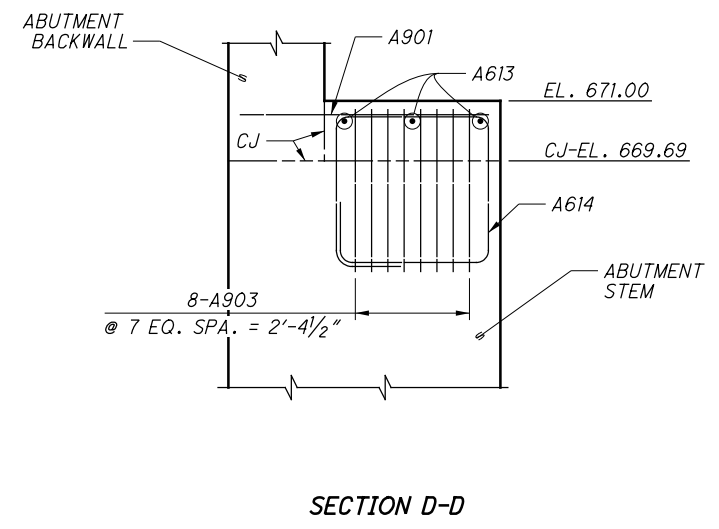
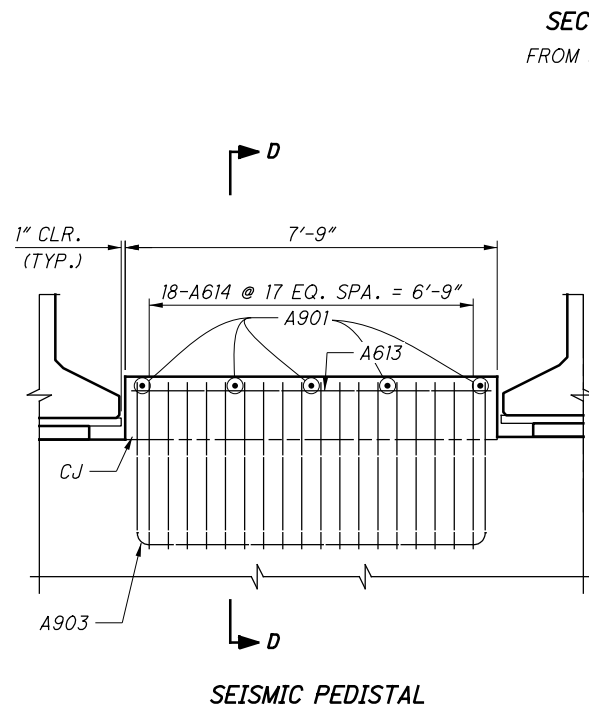
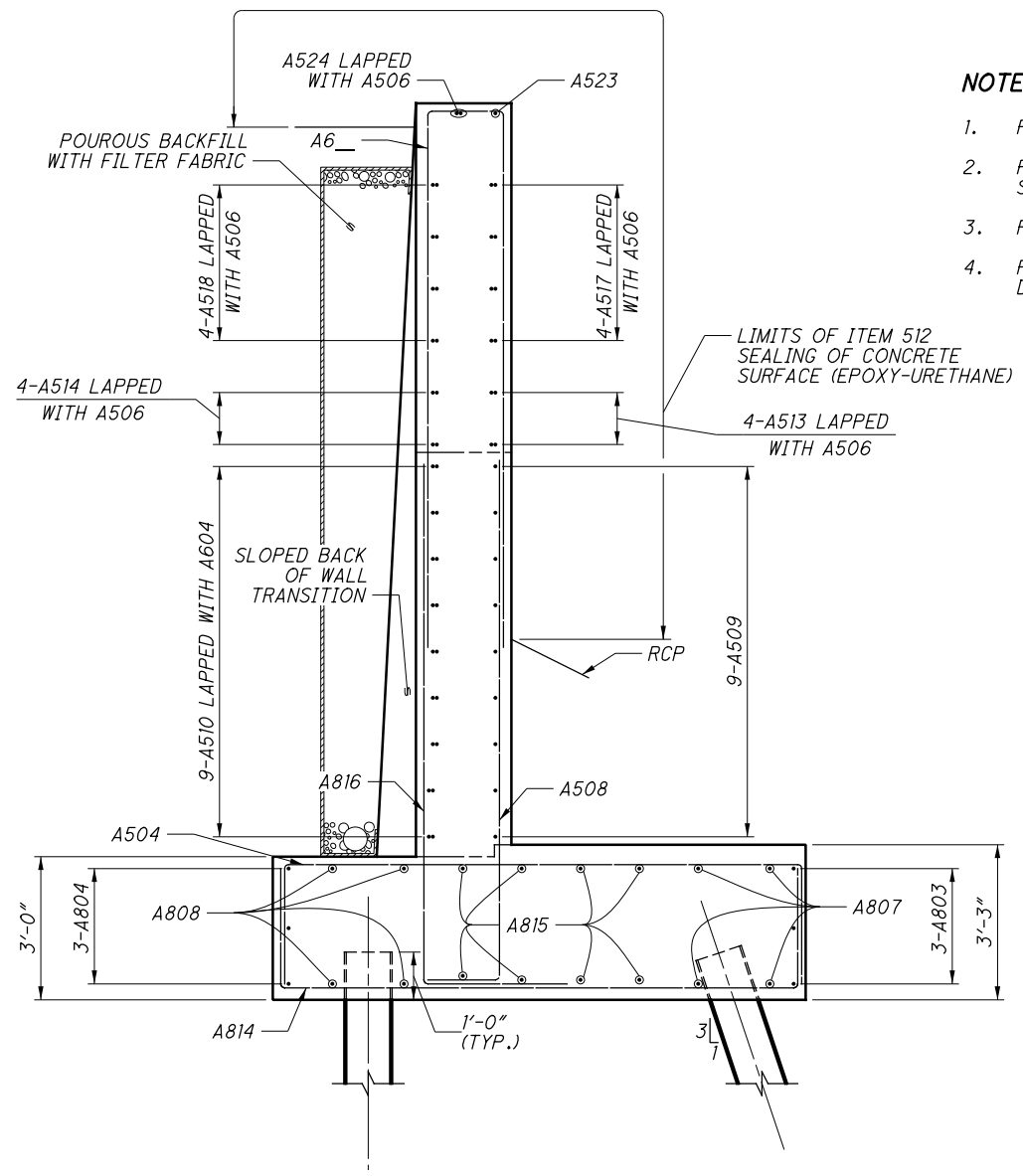
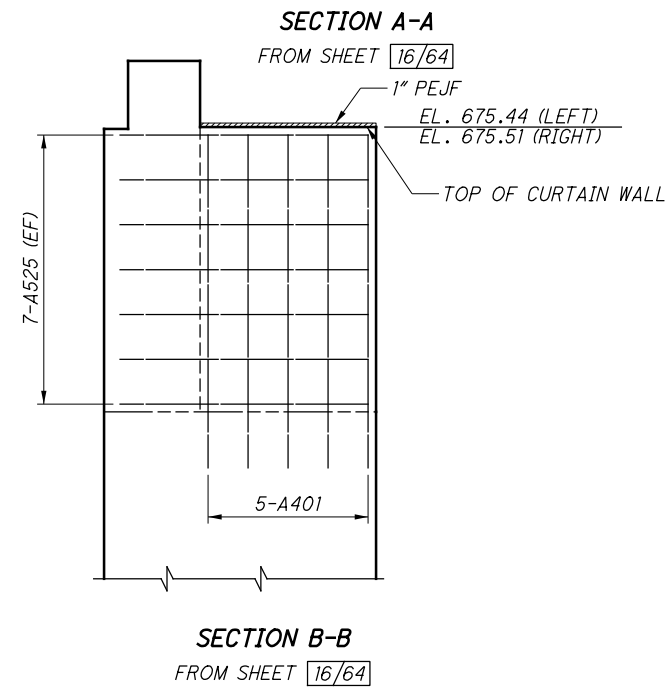
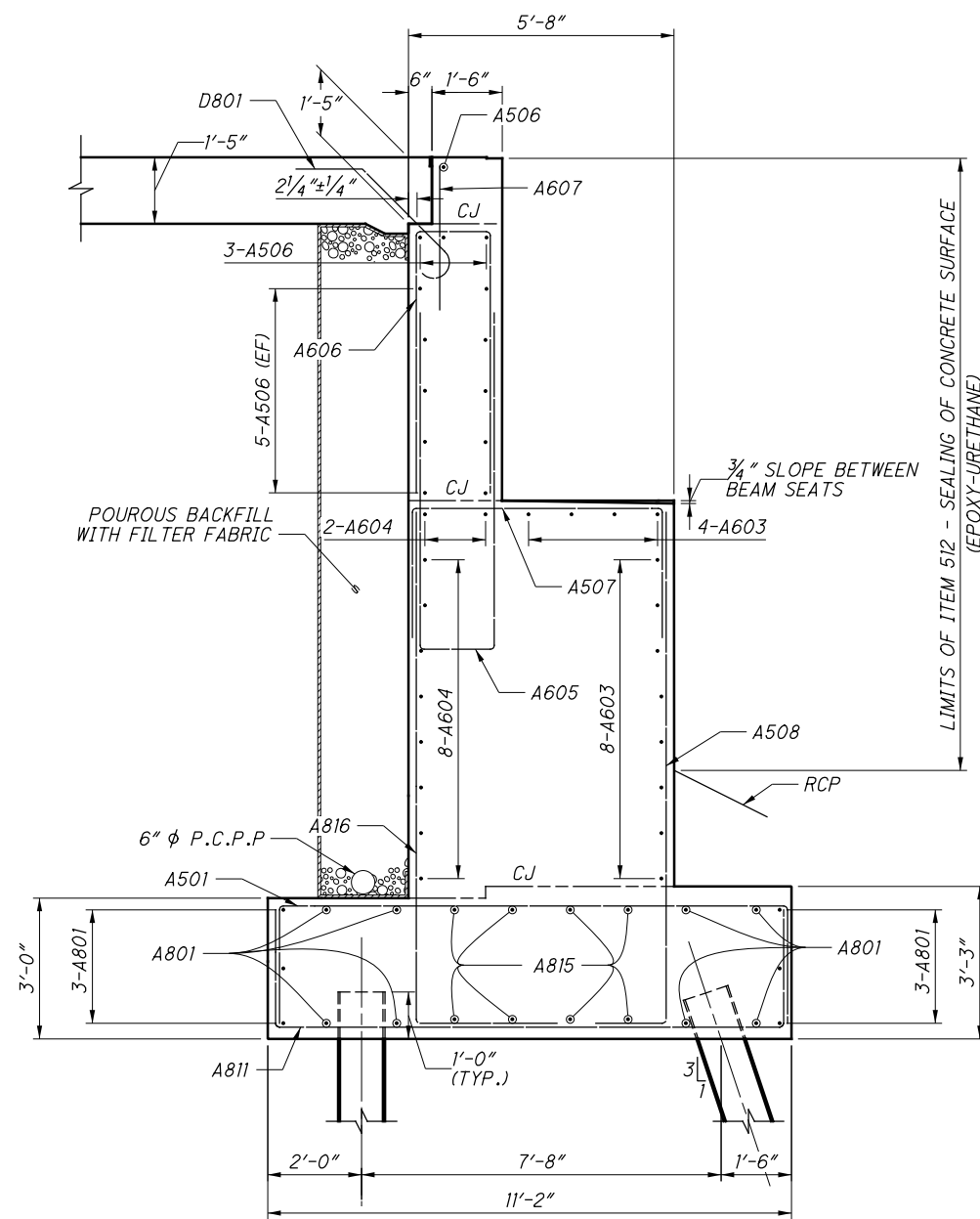
1. FOR ABUTMENT PLAN AND ELEVATION, SEE SHEET **16/64**
2. FOR PILE LAYOUT, SEE SHEET **8/64**

ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

FORWARD ABUTMENT FOOTING REINFORCING

HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

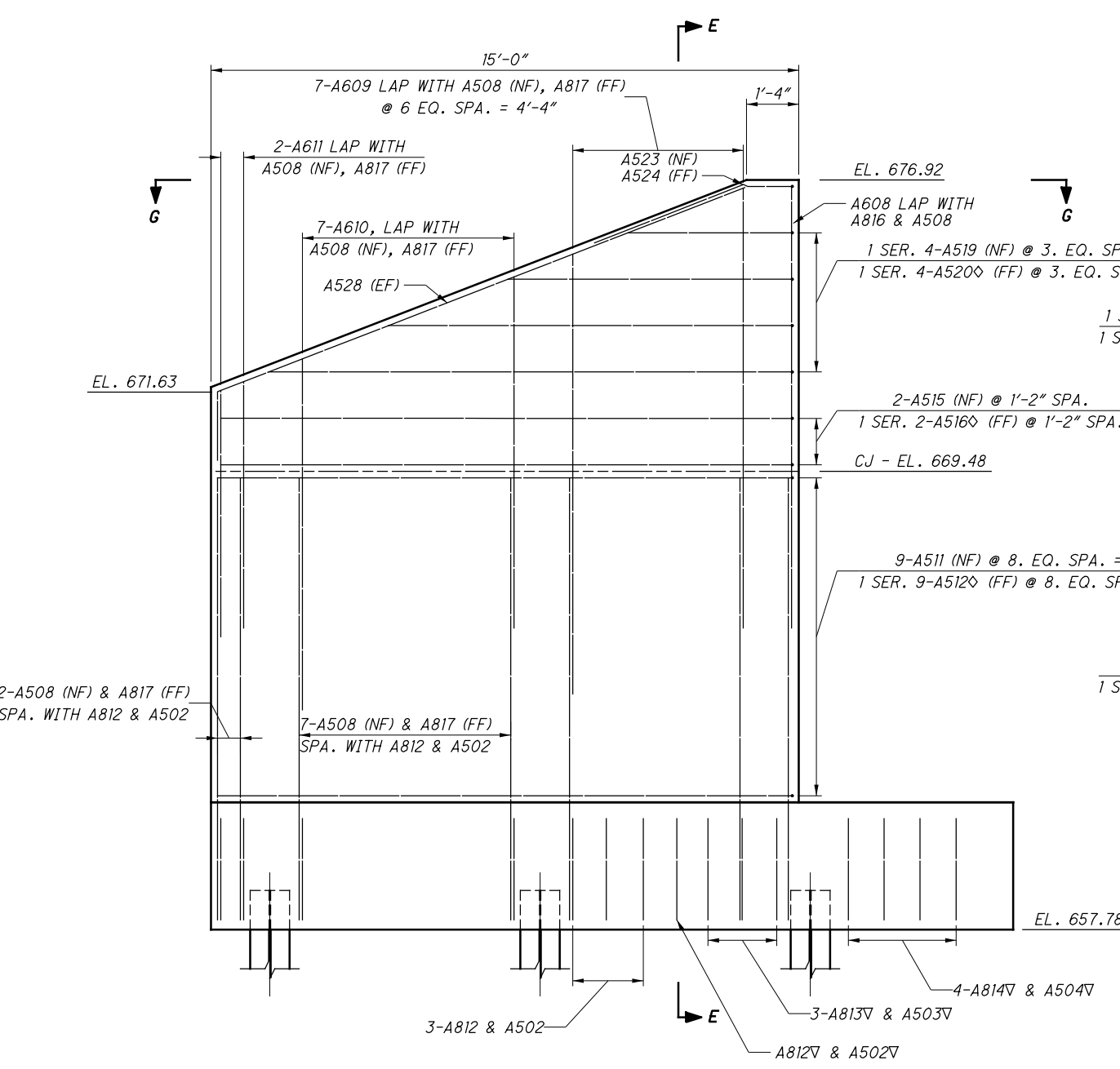


NOTES:

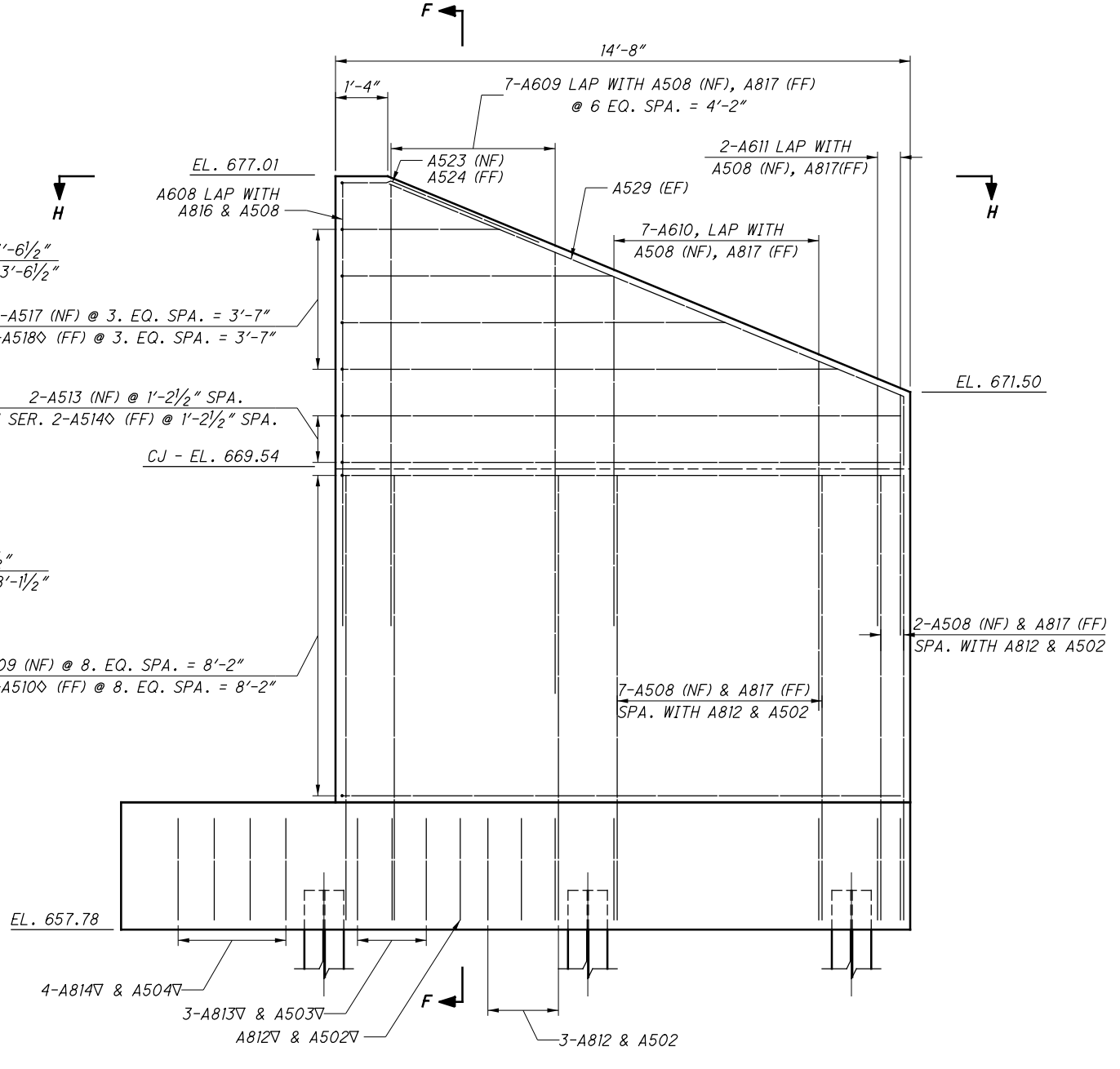
1. FOR WINGWALL DETAILS, SEE SHEETS 19-20/64
2. FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET 35/64
3. FOR PILE LAYOUT, SEE SHEET 8/64
4. FOR MODULAR EXPANSION JOINT (NOT SHOWN) DETAILS, SEE SHEETS 54-56/64

ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

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FORWARD LEFT WINGWALL



FORWARD RIGHT WINGWALL

▽ - INDICATES FANNED BAR
SEE SHEET 17/64 FOR DETAILS

◇ - INDICATES POSSIBLE NECESSITY
FOR FIELD BEND TO MATCH
BATTERED BACK FACE

- NOTES:**
1. FOR ABUTMENT DETAILS, SEE SHEET 16/64
 2. FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET 35/64
 3. FOR PILE LAYOUT, SEE SHEET 8/64
 4. FOR MODULAR EXPANSION JOINT DETAILS, SEE SHEETS 54-56/64
 5. FOR SECTIONS E-E & F-F AND VIEWS G-G & H-H, SEE SHEET 20/64

ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

DESIGNED

DRAWN

REVIEWED

DATE

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KRH

TLR

05/2015

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REVISED

STRUCTURE FILE NUMBER

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TBD

FORWARD WINGWALL DETAILS

HEN-INDUSTRIAL DRIVE-0000

INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00

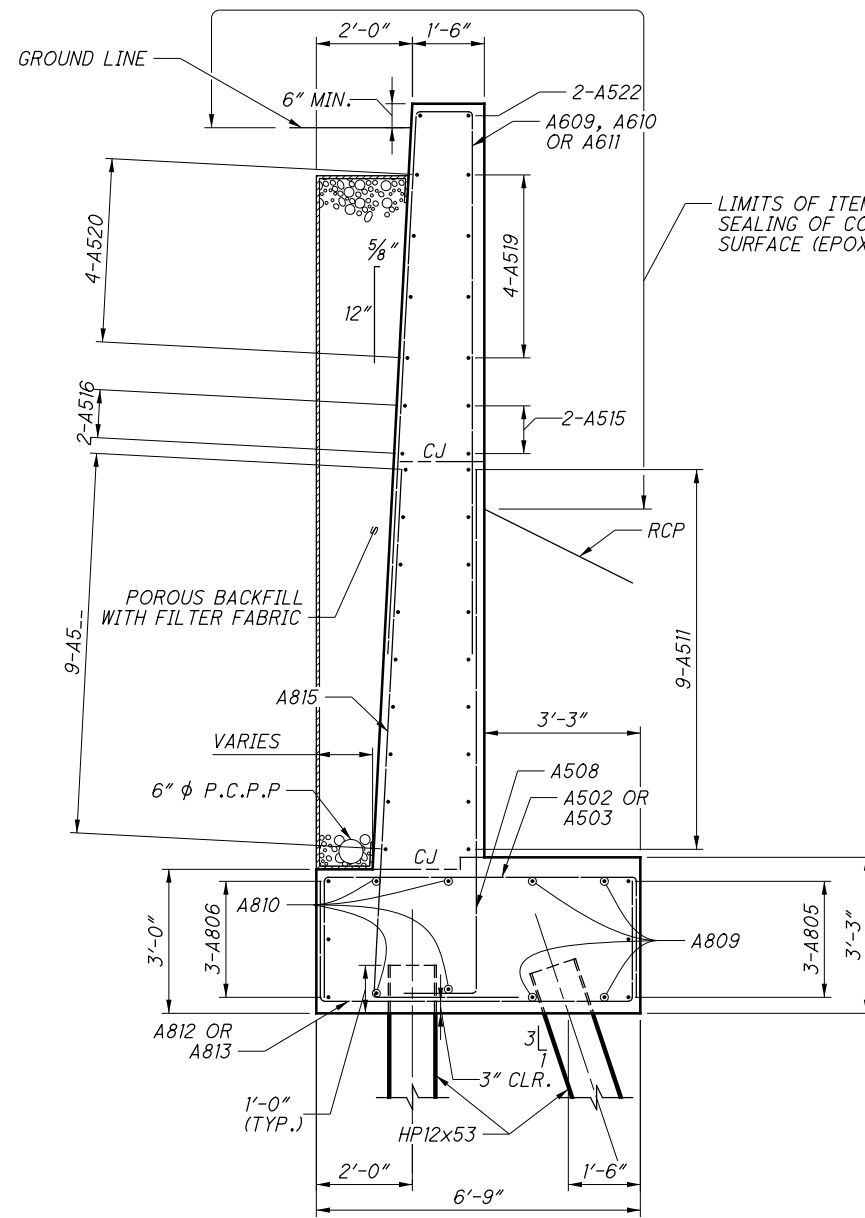
PID No. 22984

19/64

103

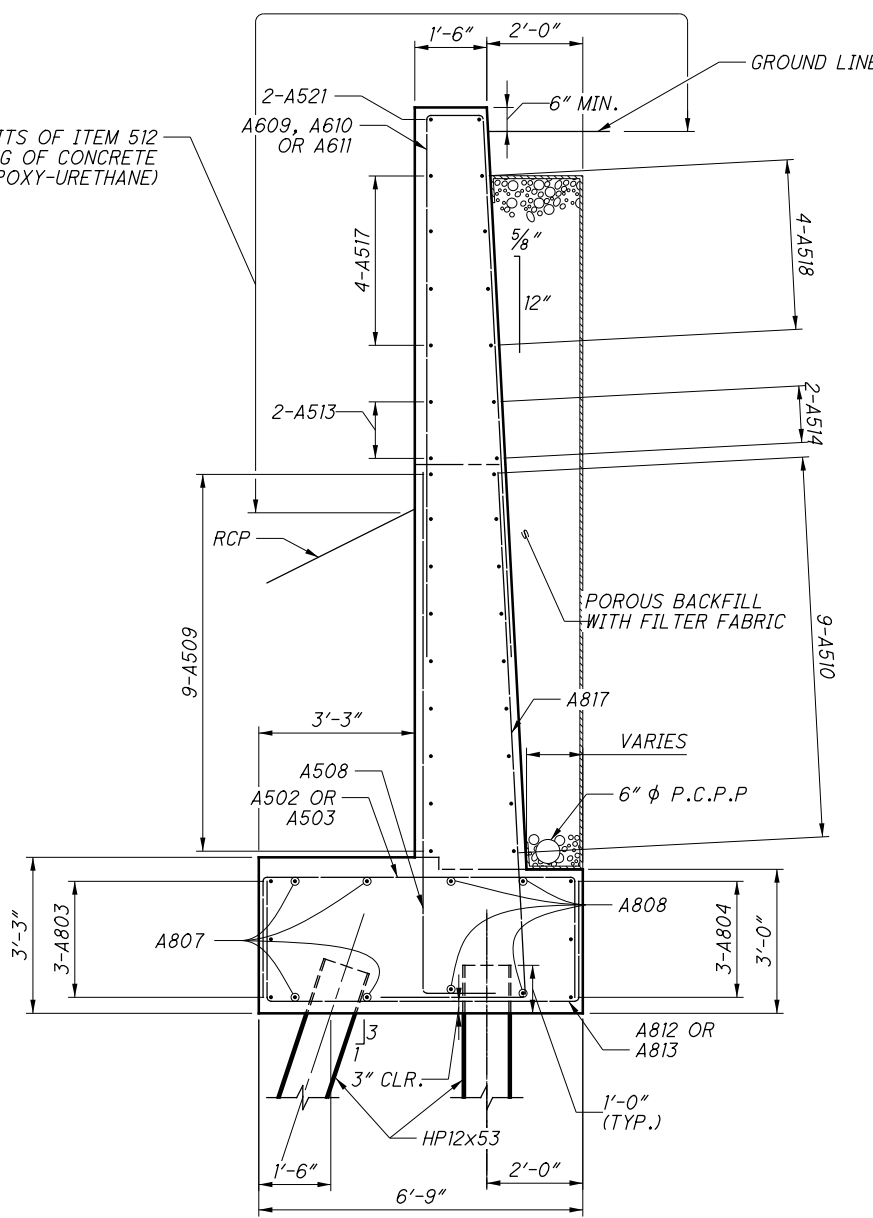
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SECTION E-E
FROM SHEET 19/64

LIMITS OF ITEM 512
SEALING OF CONCRETE
SURFACE (EPOXY-URETHANE)

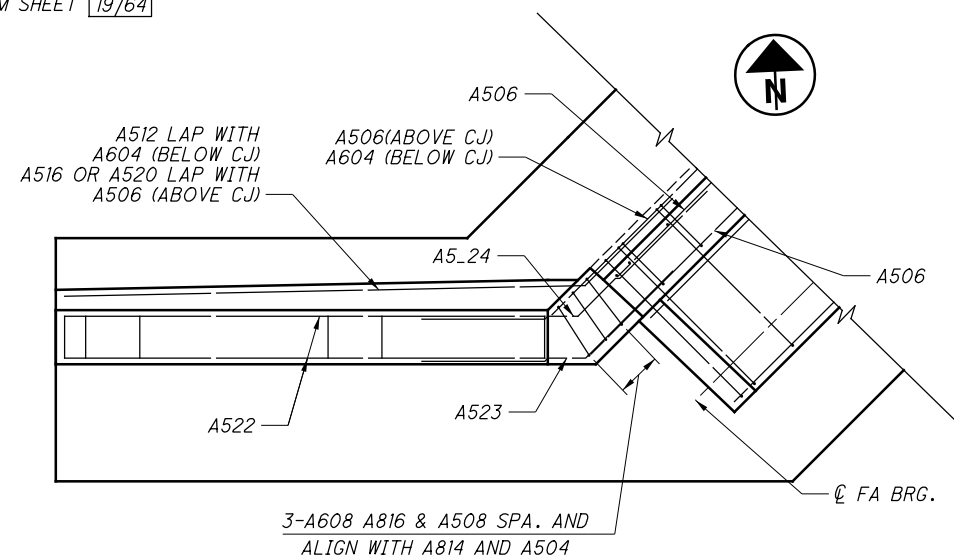


SECTION F-F
FROM SHEET 19/64

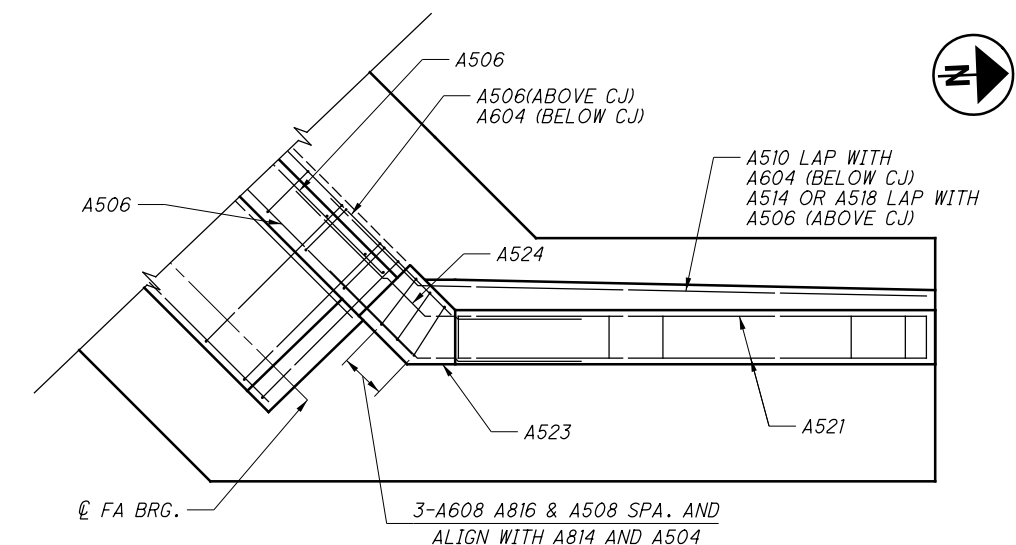
NOTES:

1. FOR ABUTMENT DETAILS, SEE SHEET 16/64
2. FOR ABUTMENT DIAPHRAGM DETAILS, SEE SHEET 35/64
3. FOR PILE LAYOUT, SEE SHEET 8/64
4. FOR MODULAR EXPANSION JOINT DETAILS, SEE SHEETS 54-56/64

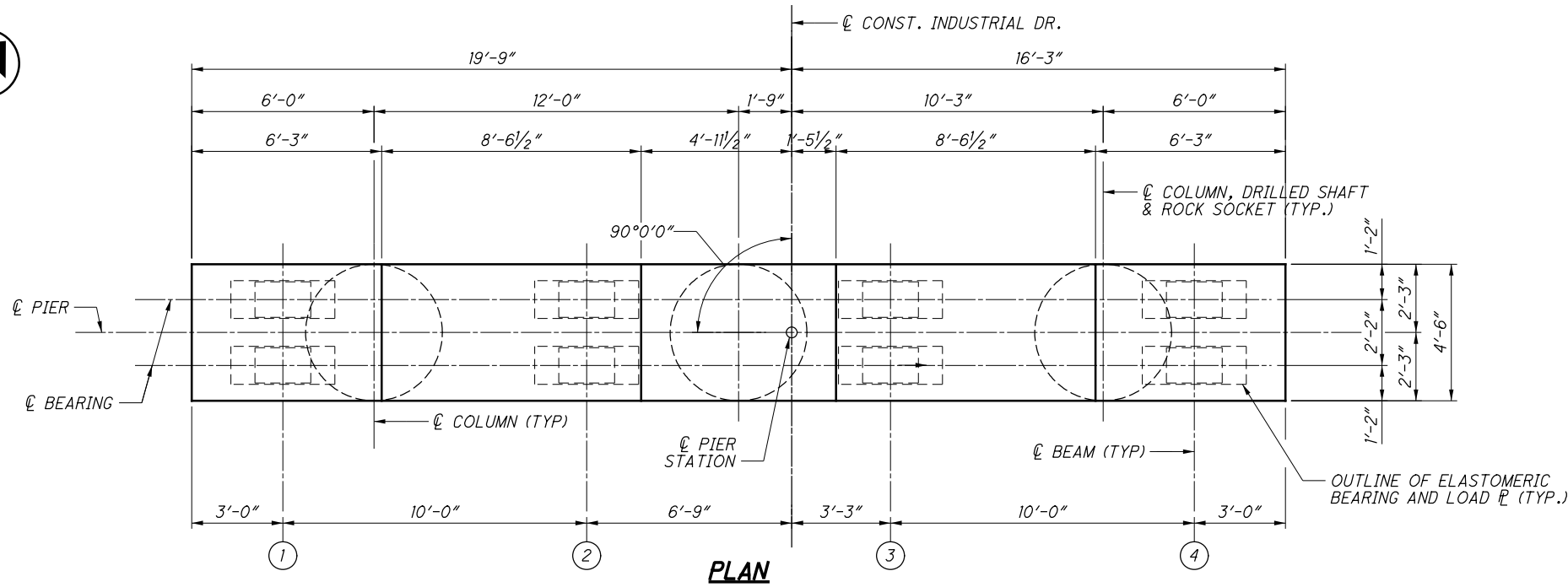
ABUTMENT REINFORCING REQUIRED LAP LENGTHS	
HORIZONTAL NO. 5 BARS	3'-5" MIN.
VERTICAL NO. 5 BARS	2'-5" MIN.
HORIZONTAL NO. 6 BARS	4'-1" MIN.
VERTICAL NO. 6 BARS	3'-10" MIN.
HORIZONTAL NO. 8 BARS	6'-10" MIN.



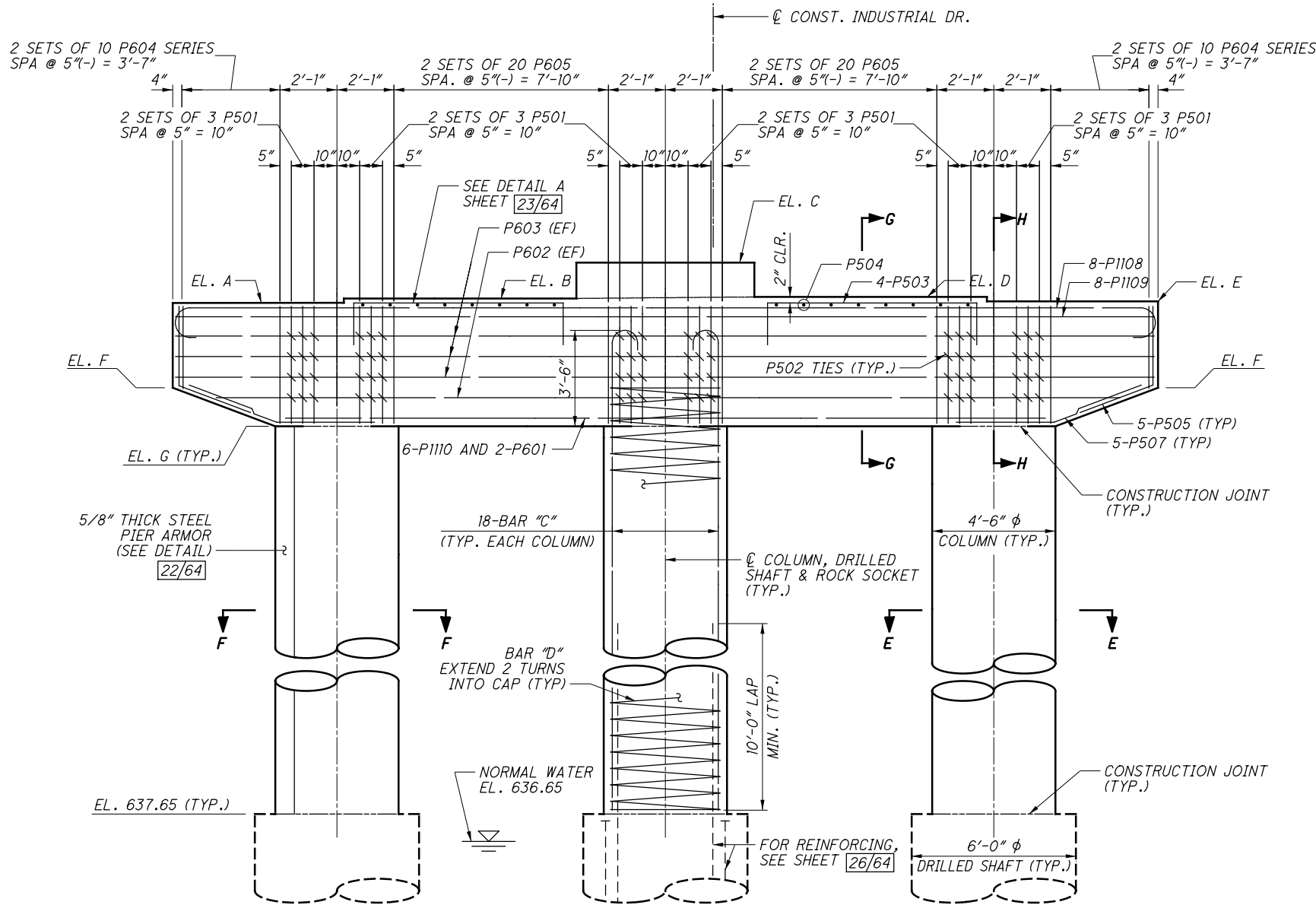
VIEW G-G
FROM SHEET 19/64



VIEW H-H
FROM SHEET 19/64



PLAN



ELEVATION

PIER NO.	STATION	EL. A	EL. B	EL. C	EL. D	EL. E	EL. F	EL. G
1	40+51.51	664.00	664.18	665.49	664.24	664.06	661.00	659.50
2	41+69.28	664.81	664.98	666.29	665.04	664.86	661.81	660.31
3	42+87.05	665.61	665.78	667.09	665.84	665.66	662.61	661.11
4	44+04.82	666.41	666.58	667.89	666.64	666.46	663.41	661.91
5	45+22.59	667.21	667.38	668.69	667.44	667.26	664.21	662.71
6	46+40.36	668.01	668.19	669.49	668.24	668.06	665.01	663.51
7	47+58.13	668.81	668.99	670.29	669.04	668.87	665.81	664.31

REINFORCING BAR	PIER 1	PIER 2	PIER 3	PIER 4	PIER 5	PIER 6	PIER 7
BAR "C"	P1101	P1102	P1103	P1104	P1105	P1106	P1107
BAR "D"	SP403	SP404	SP405	SP406	SP407	SP408	SP409

LEGEND:

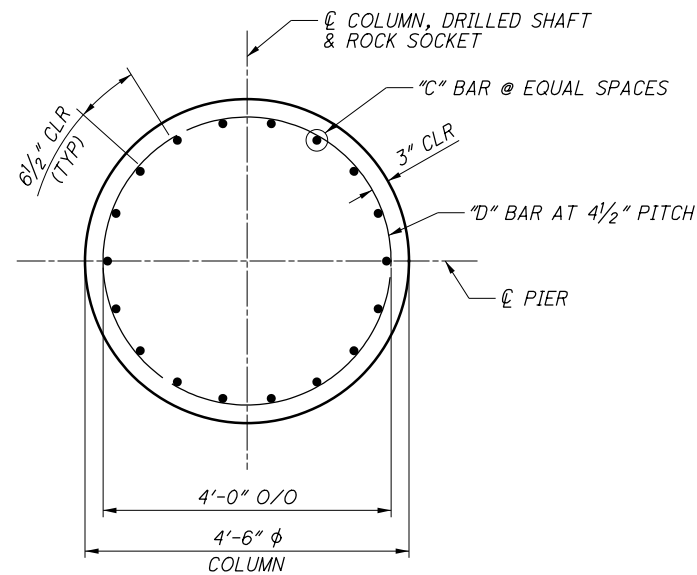
② DENOTES PROPOSED BEAM LINE

NOTES:

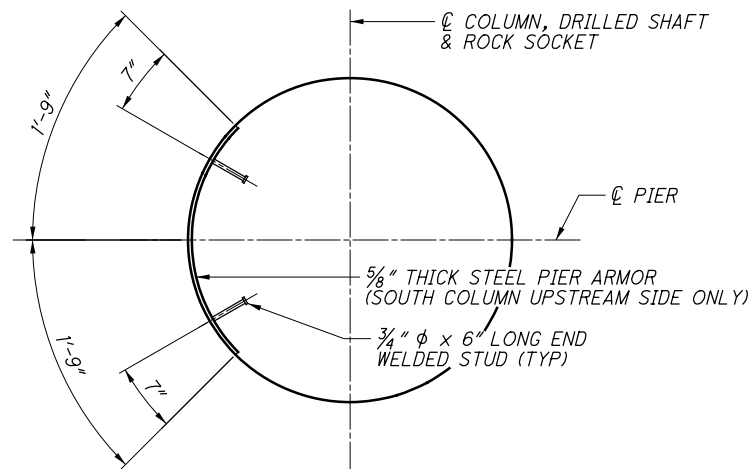
- FOR DRILLED SHAFT DETAILS, SEE SHEET [26/64].
- FOR FOUNDATION PLAN, SEE SHEET [9/64] AND [10/64].
- FOR ELASTOMERIC BEARING DETAILS, SEE SHEET [34/64].
- FOR SEISMIC PEDESTAL DETAILS, SEE SHEET [23/64].
- FOR FIXED PIER DOWEL BAR DETAILS, SEE SHEET [22/64].
- FOR SECTIONS E-E, F-F, G-G, AND H-H, SEE SHEET [22/64].
- ALL MATERIAL, LABOR, AND INCIDENTALS ASSOCIATED WITH THE STEEL PIER ARMOR SHALL BE INCLUDED IN PAYMENT FOR ITEM 511, CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS, AS PER PLAN.

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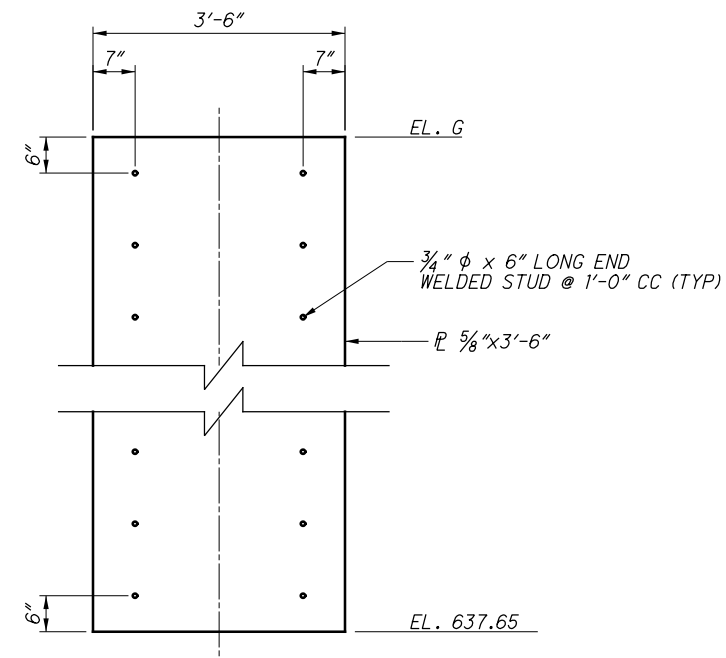
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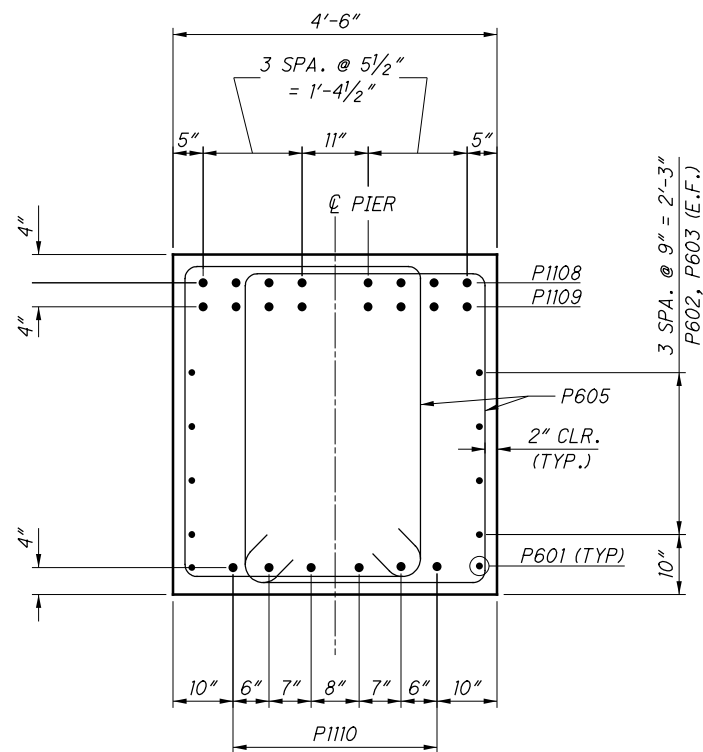
SECTION E-E



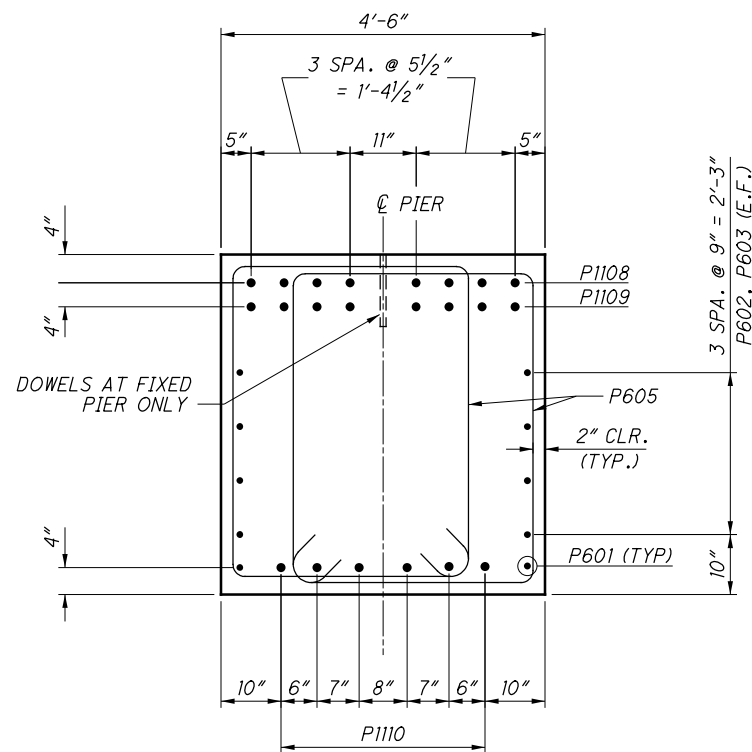
SECTION F-F
(COLUMN REINFORCEMENT NOT SHOWN)



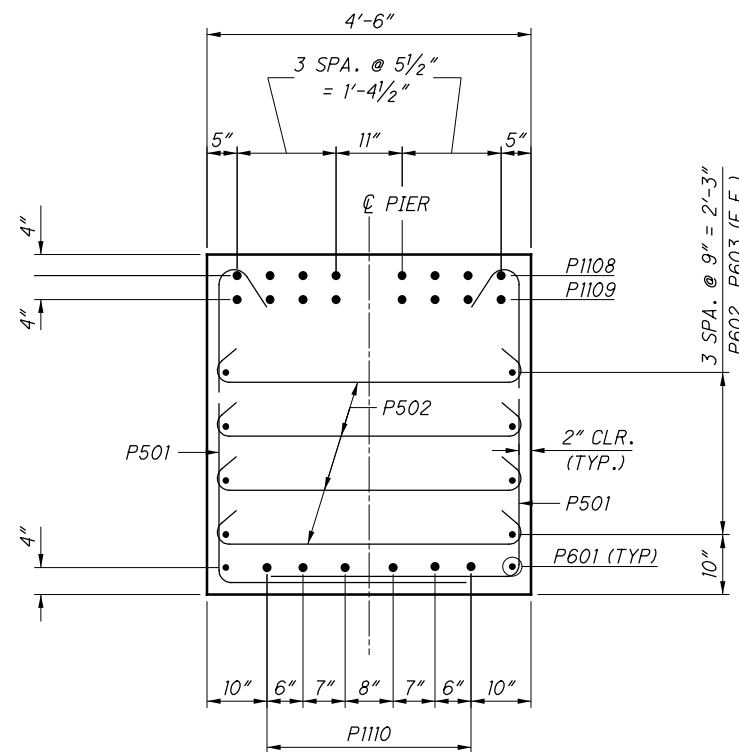
STEEL PIER ARMOR
(SHOWN DEVELOPED)



SECTION G-G



SECTION G-G
(FIXED PIER)
(PIER 4)



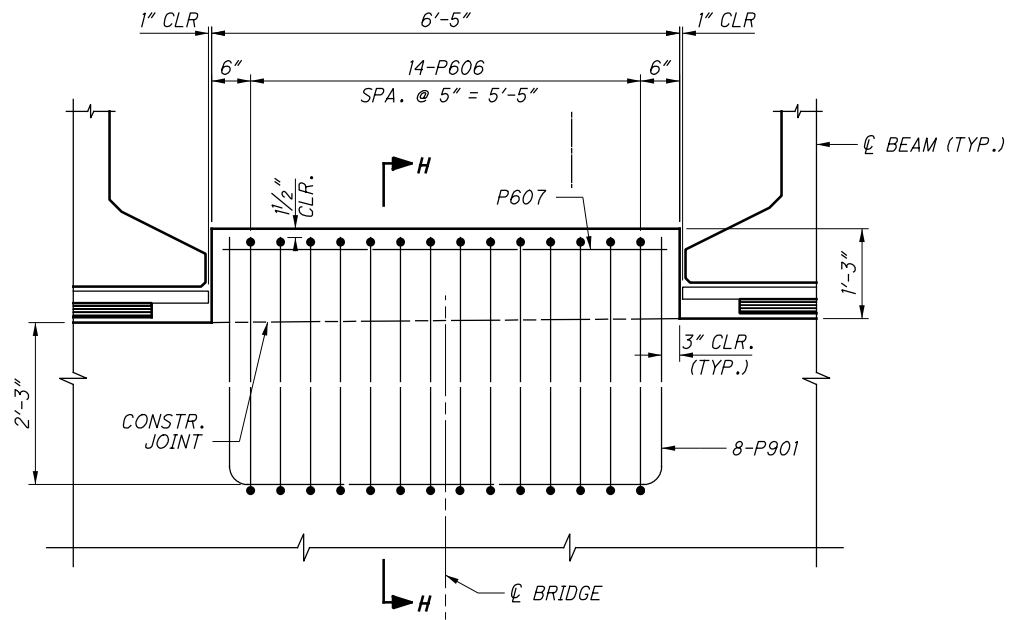
SECTION H-H

NOTES:

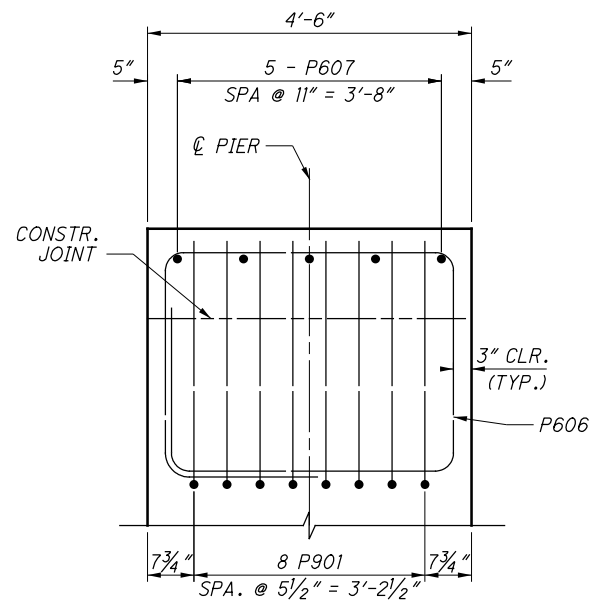
- SEE SHEET 21/64 FOR NOTES.

	1800 INDIAN WOOD CIRCLE MAUMEE, OHIO 43537	DATE: 05/2015 REVIEWED TLR: STRUCTURE FILE NUMBER TBD	DESIGNED: CWE CHECKED: SCT	DRAWN: JEC REVISED:
PIER DETAILS HEN-INDUSTRIAL DRIVE-0000 INDUSTRIAL DRIVE OVER MAUMEE RIVER				
HEN-IND-00.00 PID No. 22984				
22 / 64				
106 180				

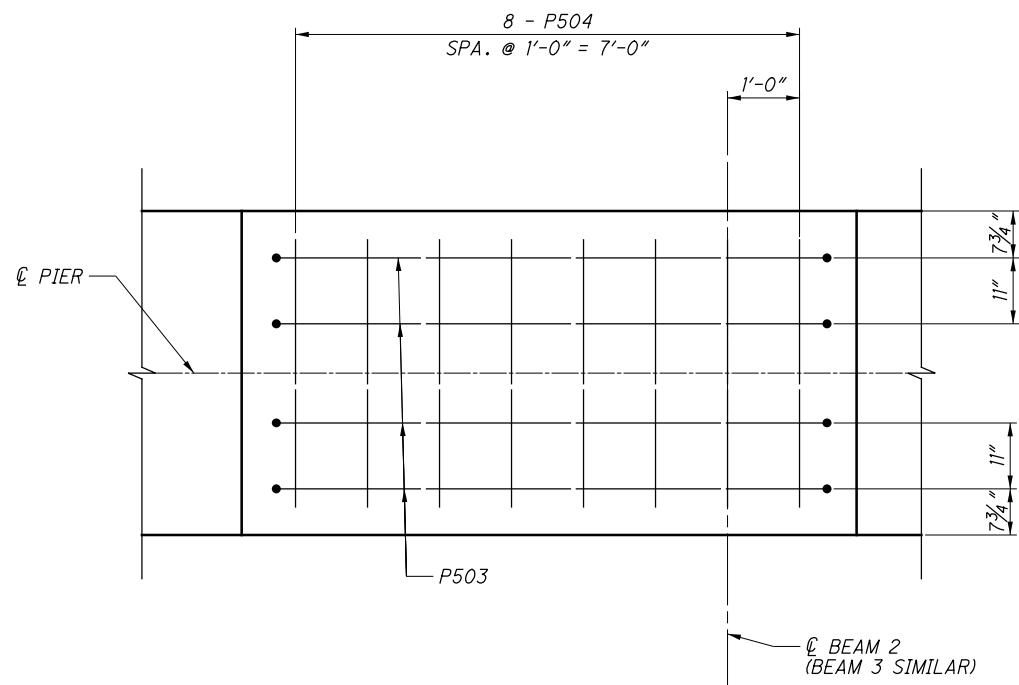
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FRONT VIEW OF SEISMIC PEDESTAL



SECTION H-H

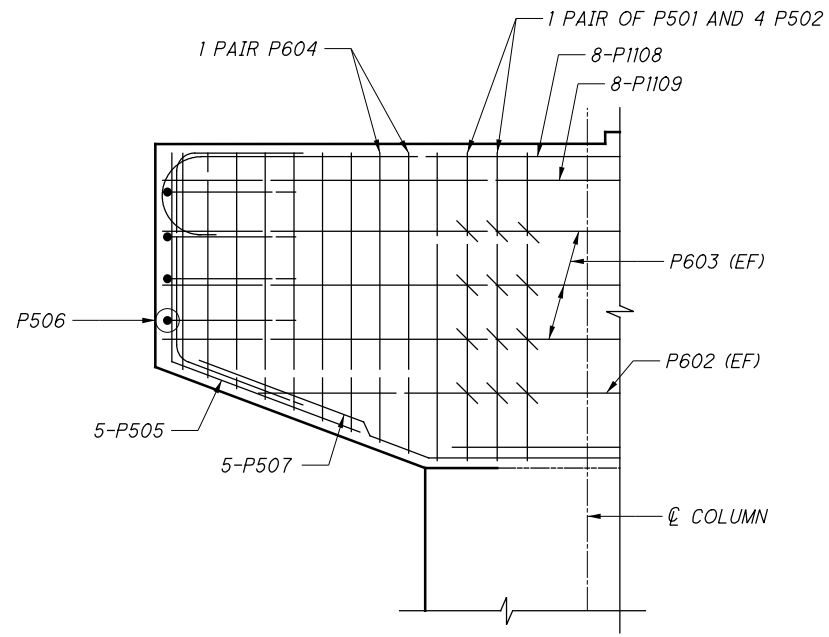


DETAIL A
PIER 4 DOWELS NOT SHOWN

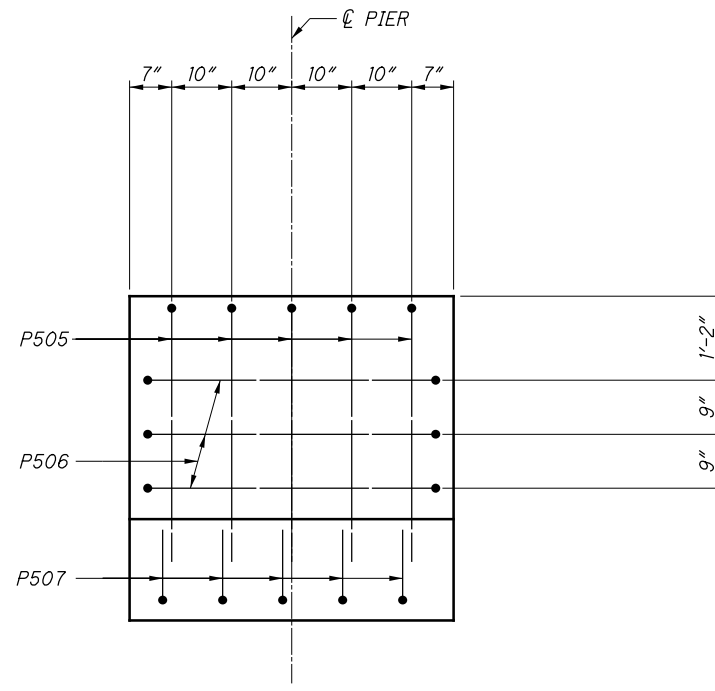
NOTES:

- SEE SHEET 21/64 FOR NOTES.

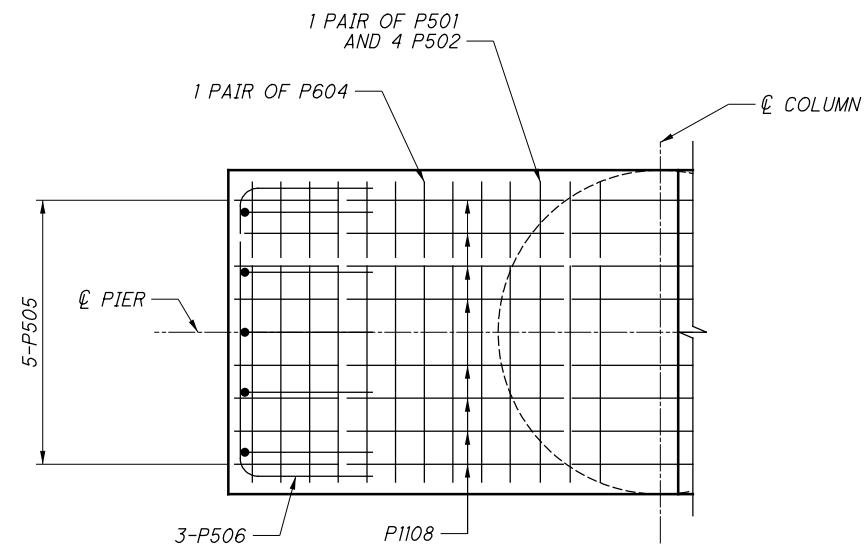
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DRAWN	JEC	REVISED	
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	05/2015		



PIER CAP ELEVATION
(COLUMN REINFORCING STEEL AND PIER 4 DOWELS NOT SHOWN)



PIER CAP END DETAIL

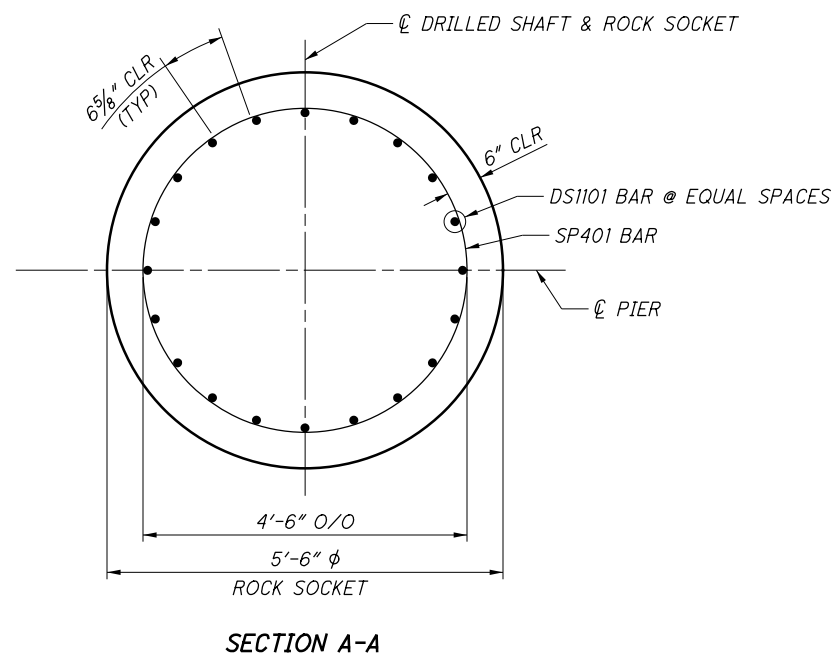
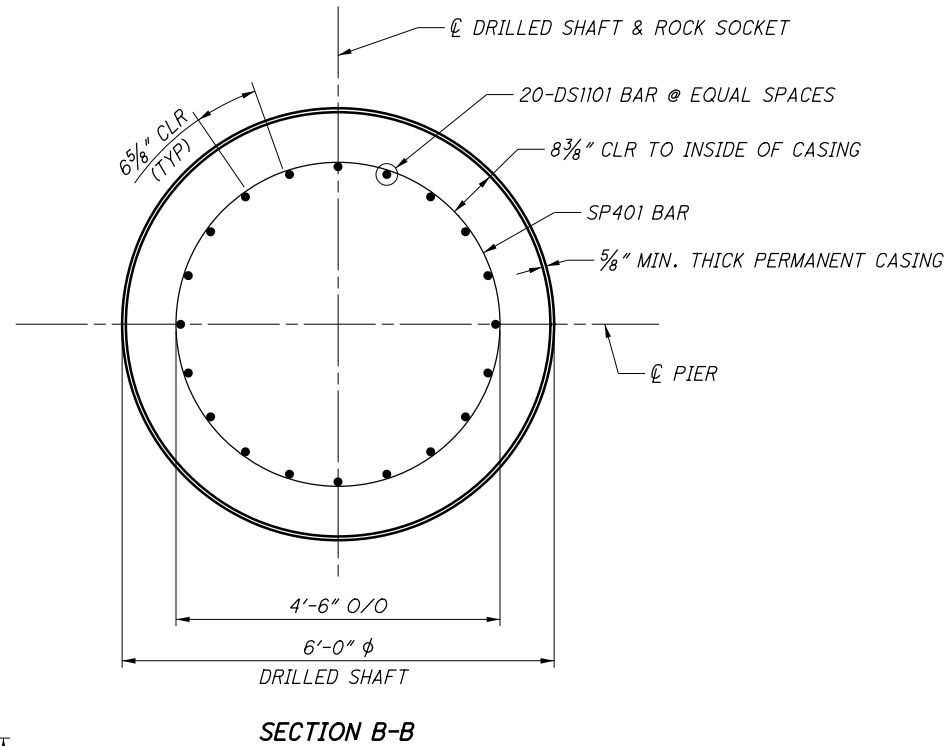
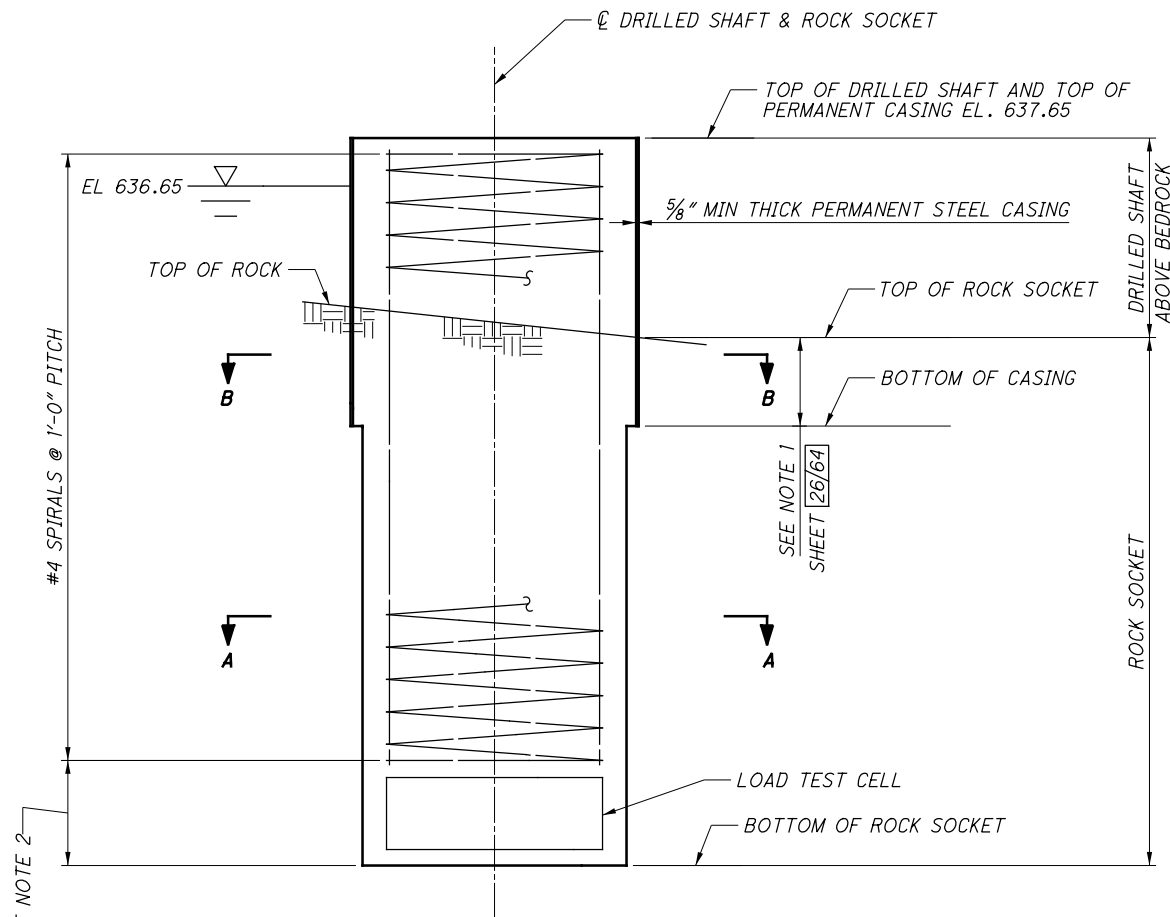


PIER CAP SECTION LEFT

DESIGNED	CWE	CHECKED	SCT
DRAWN	JEC	REVISED	
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	05/2015	FILE NUMBER	TBD

PIER DIAPHRAGM DETAILS
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

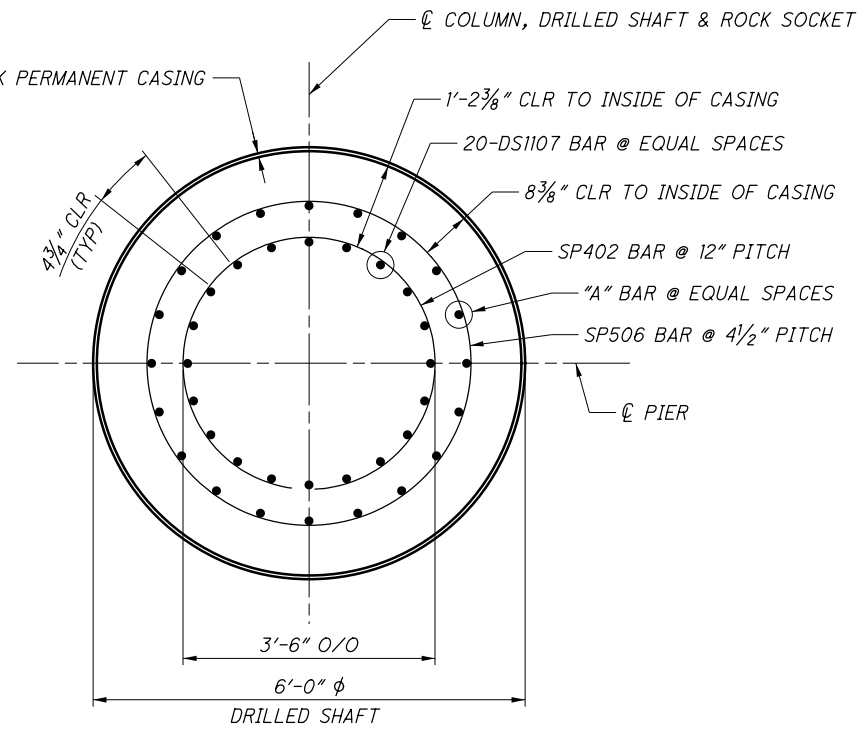
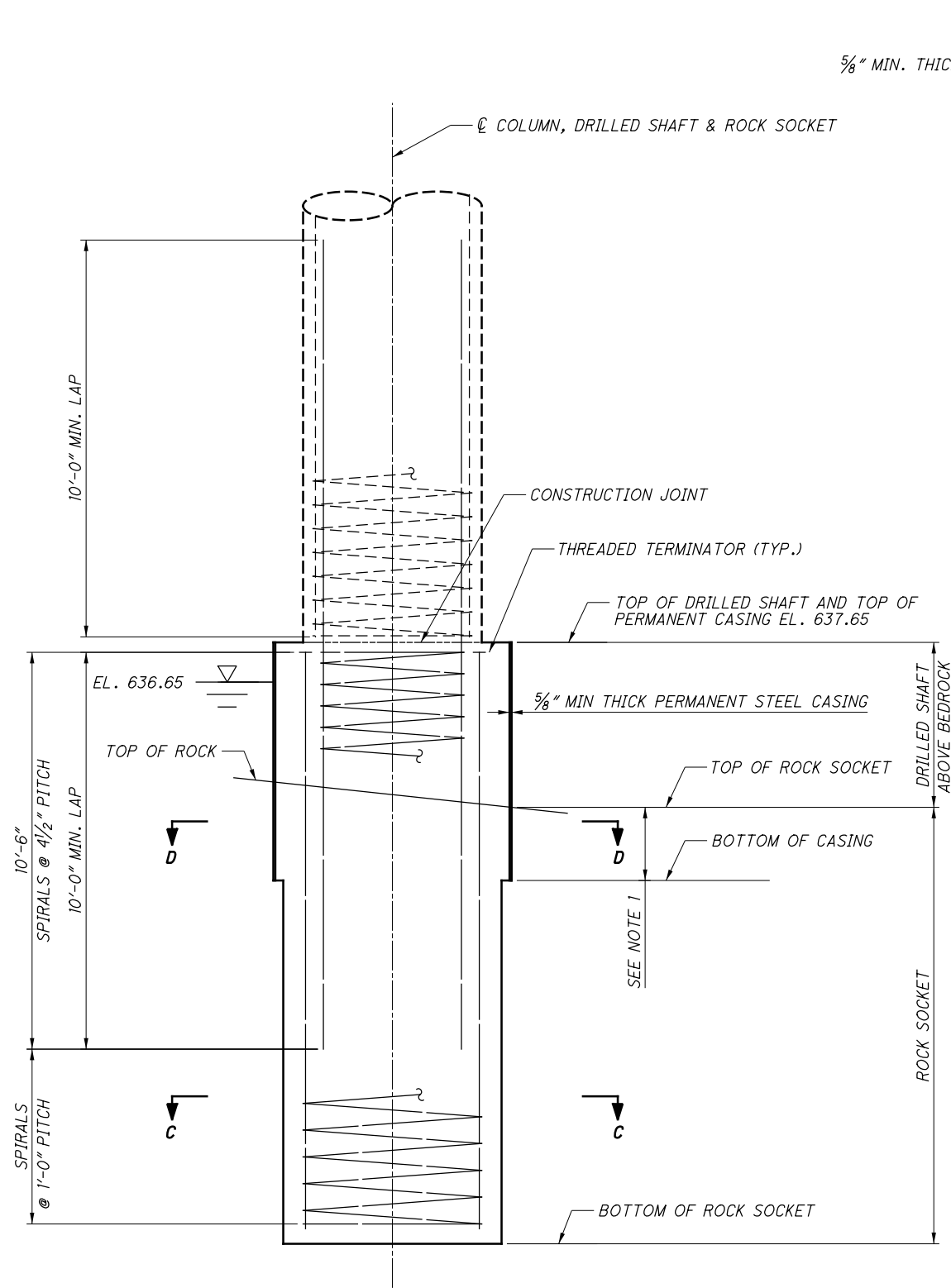
HEN-IND-00.00
PID No. 22984



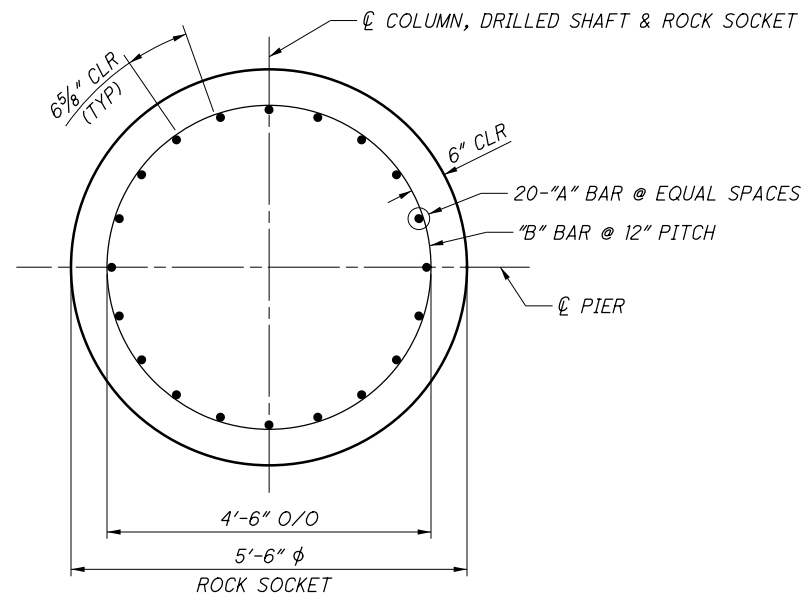
NOTES:

1. THE TESTING AT THE TEST SHAFT LOCATION SHALL BE PERFORMED PRIOR TO COMMENCEMENT OF DRILLING FOR THE PRODUCTION SHAFTS.
2. THE TESTING SHALL BE PERFORMED AS OUTLINED IN SECTION 17.2.2.2 OF PUBLICATION NO. FHWA-NHI-10-016 (FHWA GEC 010, MAY 2010). THE USE OF THE CHICAGO METHOD OF TESTING MAY BE WARRANTED. THE CROSS SECTION OF THE TEST DRILLED SHAFT SHOWN IS FOR SCHEMATIC PURPOSES ONLY. SUITABLE MODIFICATIONS TO THE PLACEMENT OF THE LOAD CELL MAY BE MADE TO OBTAIN ACCURATE RESULTS.
3. THE TEST SHAFT WILL BE CONSIDERED TO HAVE PERFORMED SATISFACTORILY IF THE TOTAL (NOMINAL) END BEARING AND SIDE RESISTANCE OF THE DRILLED SHAFT IS EQUAL TO OR GREATER THAN 2,700 KIPS. THE CONTRACTOR SHALL PROVIDE THE RESULTS OF THE TESTS TO THE DESIGN ENGINEER.
4. FOR ADDITIONAL NOTES, SEE SHEET 6, 26/64.
5. FOR DRILLED SHAFT RECORD, SEE SHEET 27/64.

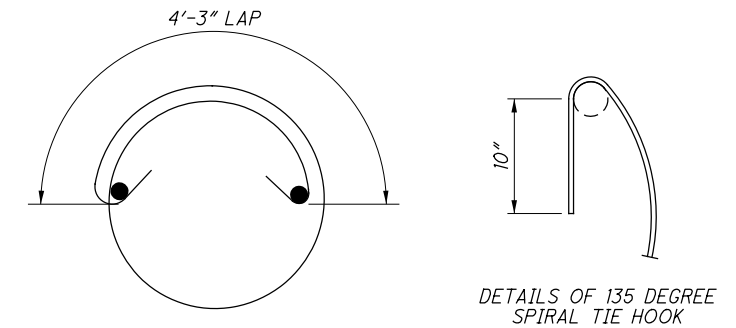
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SECTION D-D



SECTION C-C



SPIRAL ANCHOR SPLICE

NOTES:

- PERMANENT CASING IS REQUIRED IN WATER AND IN THE OVERBURDEN, AND IT SHALL BE EMBEDDED INTO ROCK TO CREATE AND MAINTAIN A CONCRETE TIGHT SEAL FOR CONSTRUCTION OF THE DRILLED SHAFT. DIMENSION TO BE FIELD VERIFIED.
- ELEVATIONS FOR THE BOTTOM OF THE DRILLED SHAFT AND BOTTOM OF DRILLED SHAFT ROCK SOCKET WILL BE DETERMINED BY THE OHIO DEPARTMENT OF TRANSPORTATION BASED ON THE RESULTS OF ROCK SOUNDING AND ROCK CORING. QUANTITIES FOR THE DRILLED SHAFTS SHOWN ON THE ESTIMATED QUANTITIES SHEET ARE ESTIMATES. THE PAID QUANTITIES WILL BE THAT OF THE ACTUAL INSTALLED QUANTITY.
- MECHANICAL COUPLERS SHALL COMPLY WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SEVENTH EDITION, ARTICLE 5.11.5.2.2. WHEN A DRILLED SHAFT IS LENGTHENED IN THE FIELD, 100% OF THE VERTICAL REINFORCEMENT MAY BE SPLICED AT THE BOTTOM OF THE REINFORCEMENT CAGE.
- SPIRAL SPLICES SHALL BE MECHANICALLY COUPLED, WELDED, OR HOOKED LAPPED SPLICED. ENDS OF SPIRAL REINFORCING SHALL BE HOOKED 135 DEGREES AROUND A VERTICAL REINFORCING BAR. SEE SPLICE DETAIL.
- 5/8 INCH CASING THICKNESS SHOWN IS A MINIMAL STRUCTURAL REQUIREMENT FOR THE DRILLED SHAFTS IN THEIR FINAL IN-SERVICE CONDITION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SIZE THE CASING TO SATISFY CONSTRUCTION INSTALLATION REQUIREMENTS.
- FOR DRILLED SHAFT RECORD, SEE SHEET 27/64.
- DIMENSIONS SHOWN IN THE DRAWINGS ARE BASED ON THE SATISFACTORY PERFORMANCE OF THE TEST SHAFT. IN THE EVENT OF UNSATISFACTORY PERFORMANCE OF THE TEST SHAFT, THE DESIGN ENGINEER RESERVES THE RIGHT TO REVISE THE DIMENSIONS OF THE PRODUCTION SHAFTS.
- REINFORCING BARS ARE INCLUDED IN PAYMENT FOR ITEM 524 - DRILLED SHAFT

REINFORCING BAR	PIER 1	PIER 2	PIER 3	PIER 4	PIER 5	PIER 6	PIER 7
BAR "A" *	DS1102	DS1102	DS1103	DS1102	DS1104	DS1105	DS1106
BAR "B"	SP501	SP501	SP502	SP501	SP503	SP504	SP505

* THREADED TERMINATOR REQUIRED

PIER NUMBER	DRILLED SHAFT NUMBER	REQUIRED FACTORED RESISTANCE KIPS		ELEVATIONS				DRILLED SHAFT ROCK SOCKET LENGTH	DRILLED SHAFT ABOVE BEDROCK LENGTH
				TOP OF DRILLED SHAFT	TOP OF ROCK ②	BOTT OF ROCK SOCKET ELEVATION PLAN ③	TOP OF ROCK SOCKET ④		
		BEARING	LATERAL					PLAN	PLAN
1	1	1303	120	637.65	632.40	621.40	632.40	11.00	5.25
	2	1303	120	637.65	632.40	621.40	632.40	11.00	5.25
	3	1303	120	637.65	632.40	621.40	632.40	11.00	5.25
*	4	2700	-	637.65	632.40	621.40	632.40	11.00	5.25
2	5	1303	120	637.65	632.30	621.30	632.30	11.00	5.35
	6	1303	120	637.65	632.30	621.30	632.30	11.00	5.35
	7	1303	120	637.65	632.30	621.30	632.30	11.00	5.35
3	8	1303	120	637.65	631.00	620.00	631.00	11.00	6.65
	9	1303	120	637.65	631.00	620.00	631.00	11.00	6.65
	10	1303	120	637.65	631.00	620.00	631.00	11.00	6.65
4	11	1303	120	637.65	632.20	621.20	632.20	11.00	5.45
	12	1303	120	637.65	632.20	621.20	632.20	11.00	5.45
	13	1303	120	637.65	632.20	621.20	632.20	11.00	5.45
5	14	1303	120	637.65	634.30	623.30	634.30	11.00	3.35
	15	1303	120	637.65	634.30	623.30	634.30	11.00	3.35
	16	1303	120	637.65	634.30	623.30	634.30	11.00	3.35
6	17	1303	120	637.65	631.30	620.30	631.30	11.00	6.35
	18	1303	120	637.65	631.30	620.30	631.30	11.00	6.35
	19	1303	120	637.65	631.30	620.30	631.30	11.00	6.35
7	20	1303	145	637.65	634.80	623.80	634.80	11.00	2.85
	21	1303	145	637.65	634.80	623.80	634.80	11.00	2.85
	22	1303	145	637.65	634.80	623.80	634.80	11.00	2.85

NOTES:

- *1. DRILLED SHAFT NUMBER 4 IS A TEST SHAFT.
- 2. TOP OF ROCK ELEVATIONS BASED ON 2014 BORINGS / ROCK CORE.
- 3. BOTTOM OF ROCK SOCKET ELEVATIONS WERE SELECTED BASED ON THE TOP OF ROCK ELEVATIONS AT EACH SHAFT AND FOR CONSISTANCY OF TIP ELEVATIONS IN A GIVEN ROW.
- 4. TOP OF ROCK SOCKET ELEVATION IS ESTIMATED FROM THE LOWEST TOP OF ROCK ELEVATION NEAR EACH SHAFT. TOP OF ROCK SOCKET ELEVATION SHALL BE VERIFIED BY THE CONTRACTOR PER C&MS 524.01.
- 5. THE FOLLOWING RESISTANCE FACTORS WERE CONSIDERED IN CALCULATING REQUIRED NOMINAL GEOTECHNICAL RESISTANCE PER AASHTO LRFD TABLE 10.5.5.2.4-1 AND ARTICLE 10.5.5.3.3:
 - RESISTANCE FACTOR FOR AXIAL COMPRESSION RESISTANCE (SIDE RESISTANCE IN ROCK) = 0.55
 - RESISTANCE FACTOR FOR AXIAL COMPRESSION RESISTANCE (TIP RESISTANCE IN ROCK) = 0.50
 - RESISTANCE FACTOR FOR HORIZONTAL (LATERAL) GEOTECHNICAL RESISTANCE OF SINGLE SHAFT OR SHAFT GROUP = 1.0
 - RESISTANCE FACTOR FOR AXIAL COMPRESSION RESISTANCE (SIDE RESISTANCE IN ROCK) FOR EXTREME EVENT = 1.0
 - RESISTANCE FACTOR FOR AXIAL COMPRESSION RESISTANCE (TIP RESISTANCE IN ROCK) FOR EXTREME EVENT = 1.0



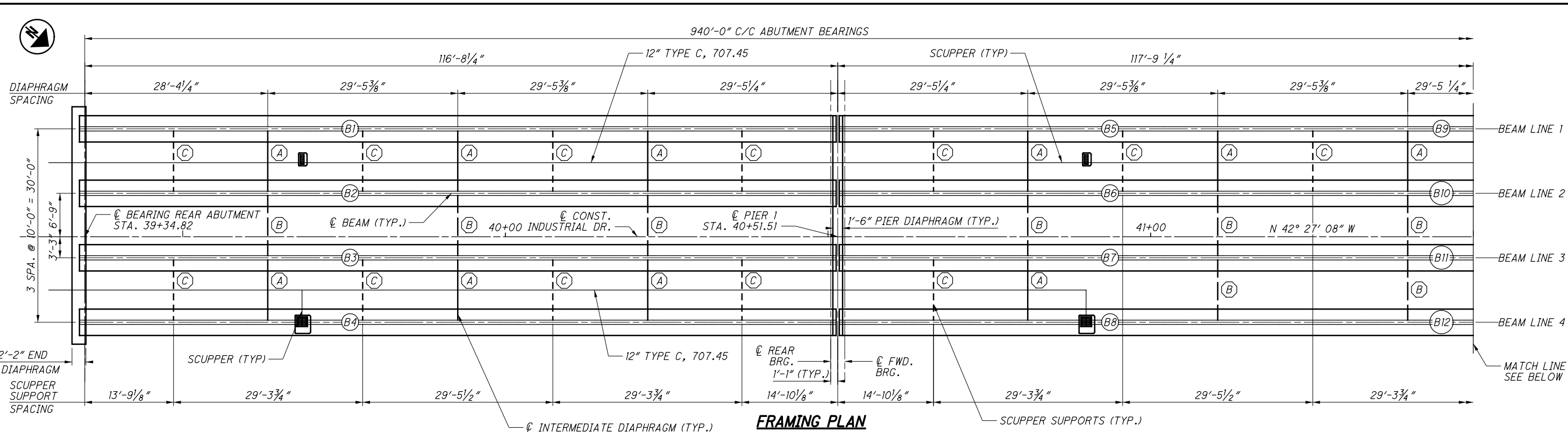
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DATE 05/2015
STRUCTURE FILE NUMBER TBD

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DESIGNED CWE
CHECKED CMZ
REVISED

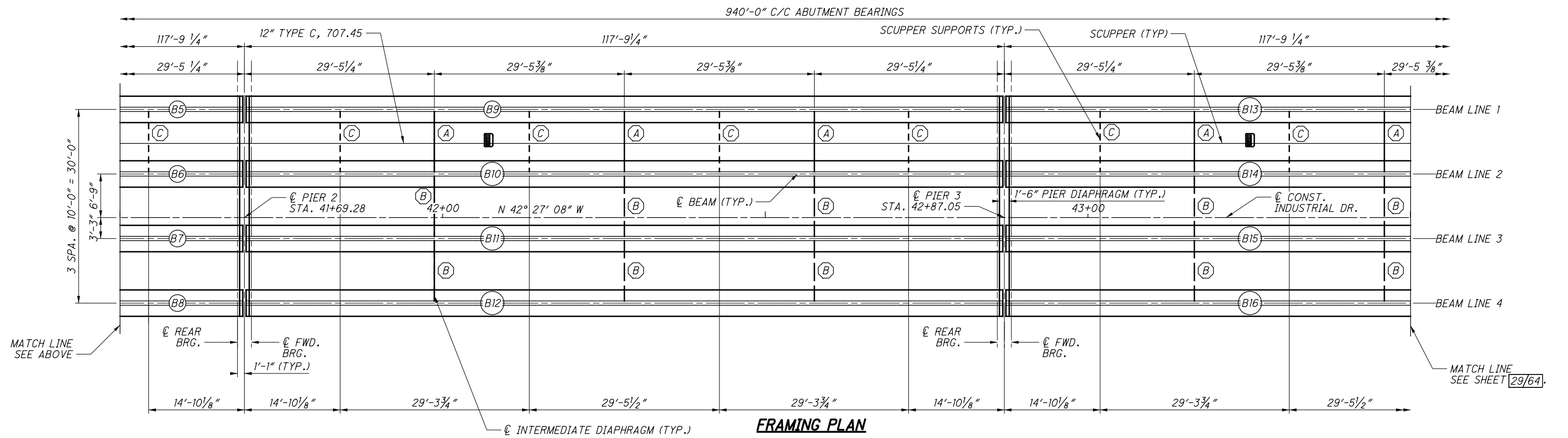
PIER DETAILS
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

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FRAMING PLAN



FRAMING PLAN

CAMBER (SPANS 1-7)

CALCULATED CAMBER AT DAY 0 (D30) IS 2.002 INCHES.

CALCULATED CAMBER AT DAY 30 (D30) IS 2.899 INCHES.

DEFLECTION DUE TO REMAINING DEAD LOAD (E.G. CONCRETE DECK, CROSSFRAMES, DIAPHRAGMS, BARRIERS, UTILITIES, ETC.) IS 1.516 INCHES (INTERIOR) AND 1.317 INCHES (FASCIA).

THE BEAM SEAT ELEVATIONS ASSUME ESTIMATED CAMBER D30 WITH A SACRIFICIAL HAUNCH THICKNESS OF 2-INCHES.

LEGEND:

- (BX) - BEAM NUMBER
- (A) - TYPE A INTERMEDIATE DIAPHRAGM
- (B) - TYPE B INTERMEDIATE DIAPHRAGM
- (C) - SCUPPER SUPPORT

NOTES:

1. FOR ADDITIONAL DETAILS AND NOTES, SEE ODOT STANDARD DRAWING PSID-1-13.
2. FOR BEARING DETAILS, SEE SHEETS [33/64](#) THRU [34/64](#).
3. FOR HAUNCH AND DECK INFORMATION, SEE SHEETS [43/64](#) THRU [48/64](#).
4. FOR SCREED ELEVATIONS, SEE SHEETS [42/64](#).
5. FOR PRESTRESSED I-BEAM DETAILS, SEE SHEETS [31/64](#) THRU [32/64](#).
6. FOR END DIAPHRAGM DETAILS, SEE SHEETS [35/64](#).
7. FOR PIER DIAPHRAGM DETAILS, SEE SHEETS [36/64](#).
8. INTERMEDIATE DIAPHRAGMS: DO NOT PLACE THE DECK CONCRETE UNTIL ALL INTERMEDIATE DIAPHRAGMS HAVE BEEN PROPERLY INSTALLED.
9. THE THREADED INSERTS AND THE THREADED RODS FOR THE DIAPHRAGMS SHALL BE GALVANIZED ACCORDING TO CMS 711.02.

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

Mannik Smith GROUP

DESIGNED: KRH
CHECKED: SCT

DRAWN: RJS
REVISED:

REVIEWED: TLR
STRUCTURE FILE NUMBER: TBD

DATE: 05/20/15

FRAMING PLAN (1 OF 3)

HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00

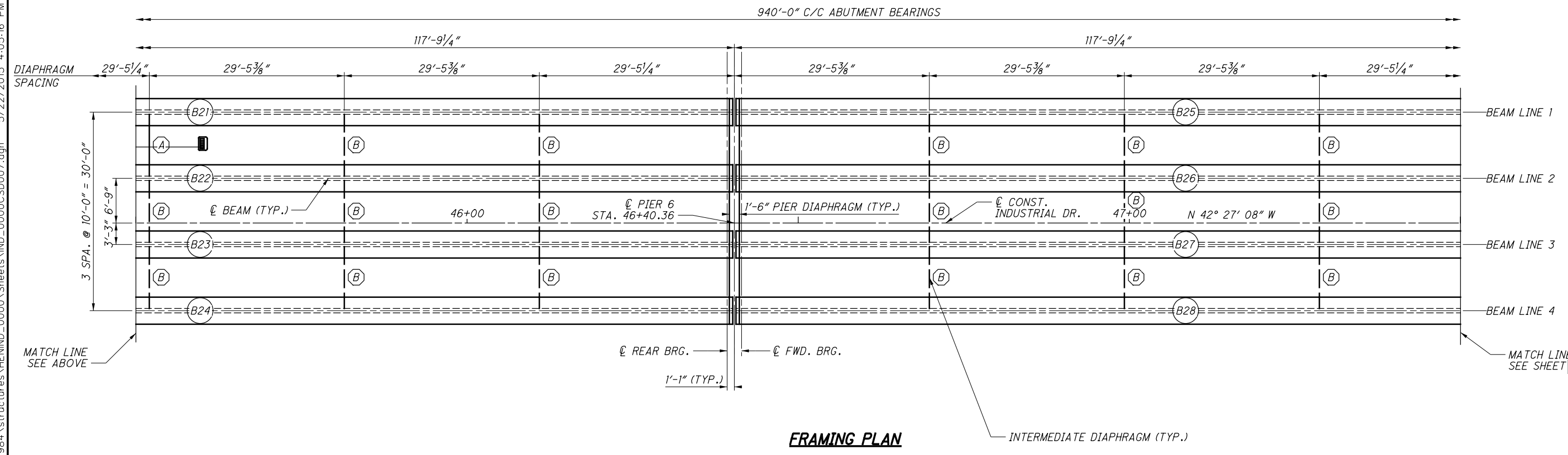
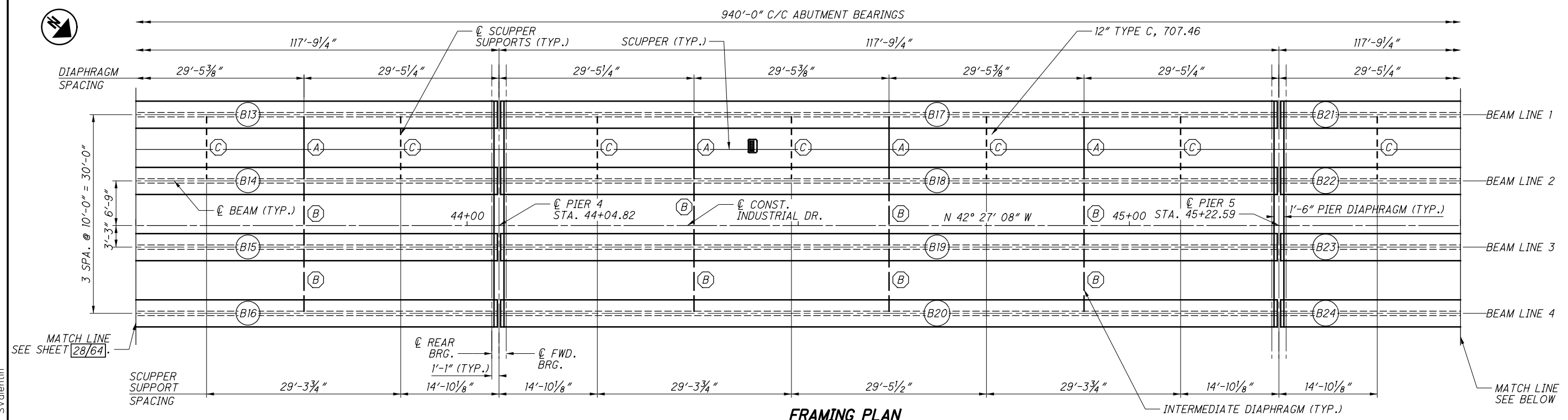
PID No. 22984

28/64

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180



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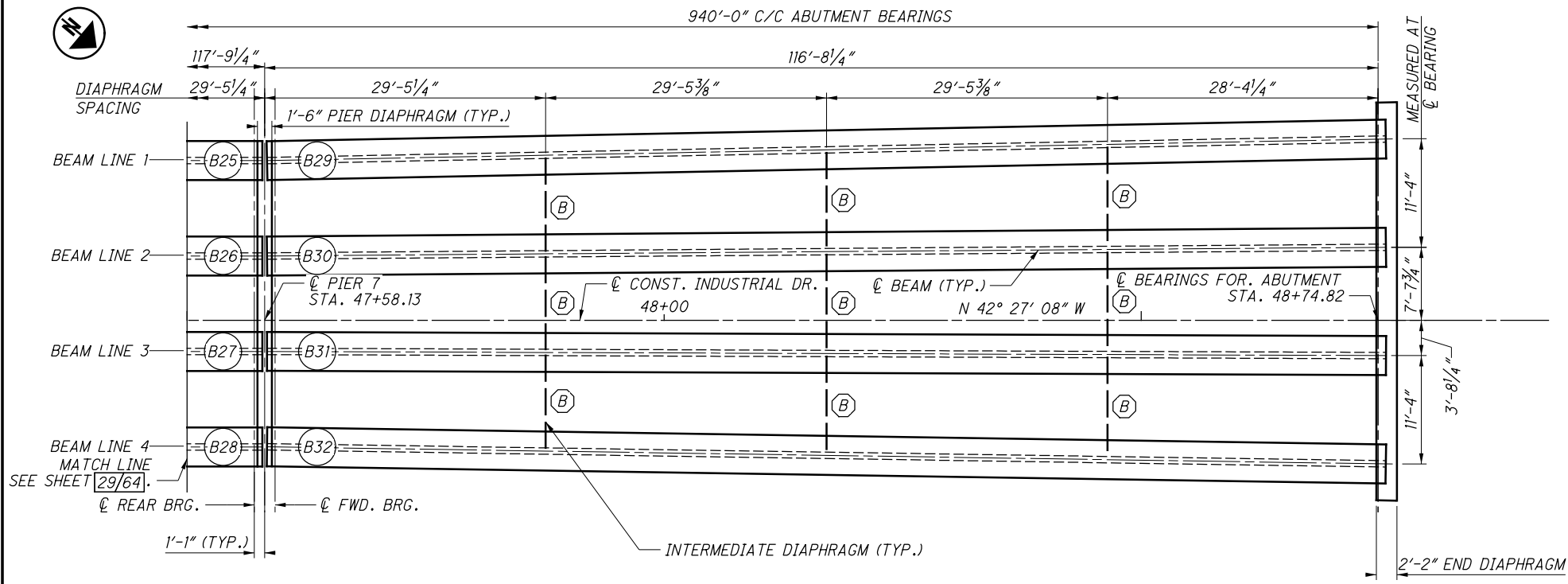
DESIGNED	DRAWN	REVIEWED	DATE
KRH	RJS	TLR	05/2015
CHECKED	REVISED	STRUCTURE FILE NUMBER	TBD
SCT			

FRAMING PLAN (2 OF 3)
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
 PID No. 22984

NOTES:
 1. FOR NOTES, SEE SHEETS 28/64.

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FRAMING PLAN

CAMBER (SPANS 8)

CALCULATED CAMBER AT DAY 0 (D30) IS 2.002 INCHES.
 CALCULATED CAMBER AT DAY 30 (D30) IS 2.899 INCHES.

DEFLECTION DUE TO REMAINING DEAD LOAD (E.G. CONCRETE DECK, CROSSFRAMES, DIAPHRAGMS, BARRIERS, UTILITIES, ETC.) IS 1.8 INCHES (INTERIOR) AND 1.412 INCHES (FASCIA).

THE BEAM SEAT ELEVATIONS ASSUME ESTIMATED CAMBER D30 WITH A SACRIFICIAL HAUNCH THICKNESS OF 2-INCHES.

NOTES:
 1. FOR NOTES, SEE SHEETS 28/64.

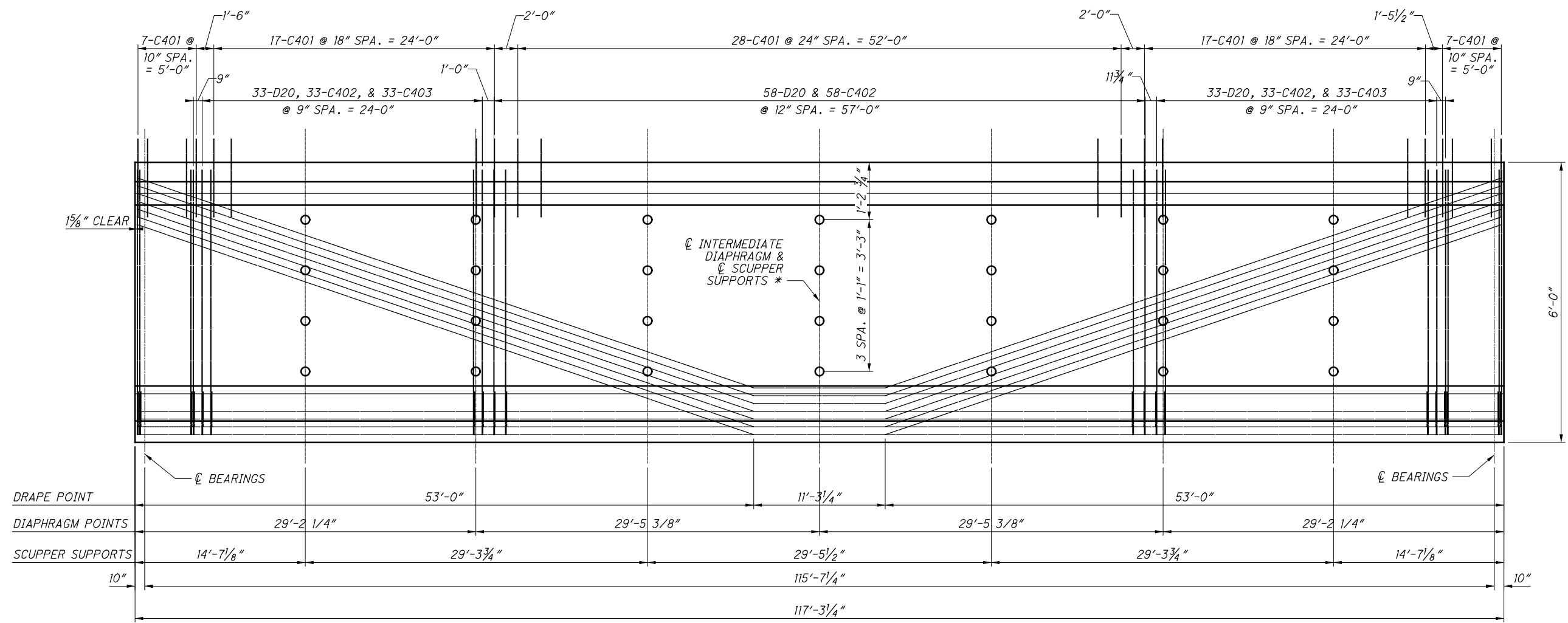
	1800 INDIAN WOOD CIRCLE MAUMEE, OHIO 43537	DATE: 05/2015 REVIEWED: SCT DRAWN: RJS DESIGNED: KRH	STRUCTURE FILE NUMBER: TBD REVISIONS:	FRAMING PLAN (3 OF 3) HEN-INDUSTRIAL DRIVE-0000 INDUSTRIAL DRIVE OVER MAUMEE RIVER
HEN-IND-00.00 PID No. 22984		30/64		114 180

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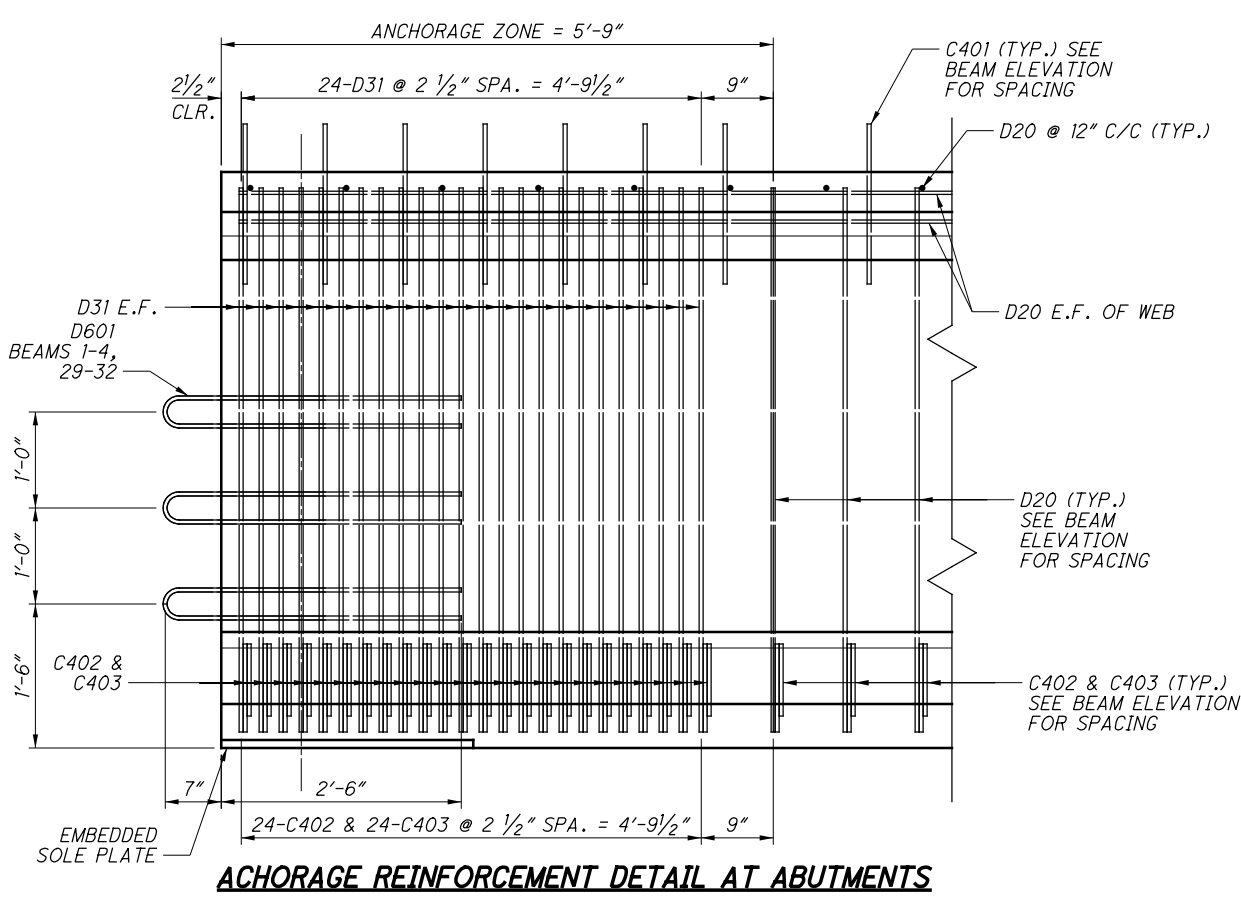
DESIGNED	DATE
CHKD	05/2015
DRAWN	FILE NUMBER
RJS	TBD
REVISED	STRUCTURE
SCT	FILE NUMBER

PRESTRESSED I-BEAM DETAILS
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

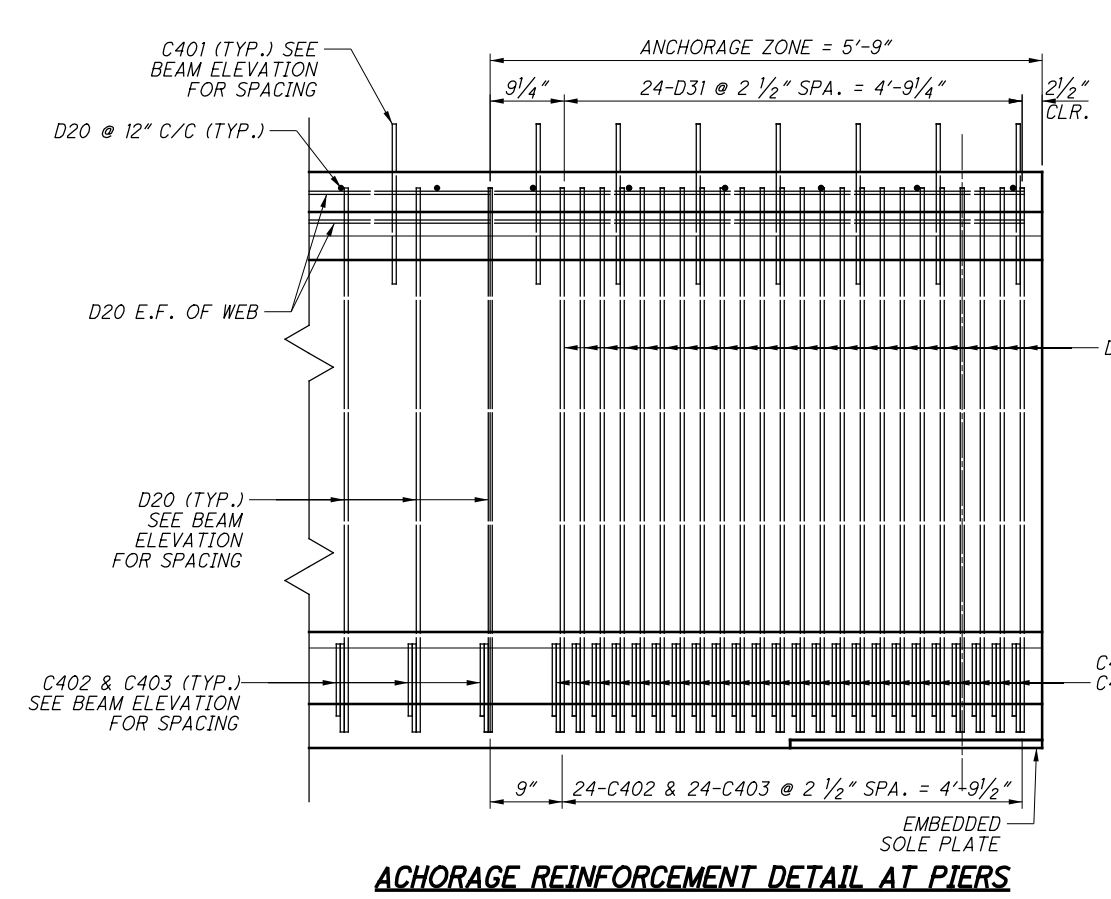
HEN-IND-00.00
PID No. 22984
31 / 64
115
180



BEAM ELEVATION



ANCHORAGE REINFORCEMENT DETAIL AT ABUTMENTS

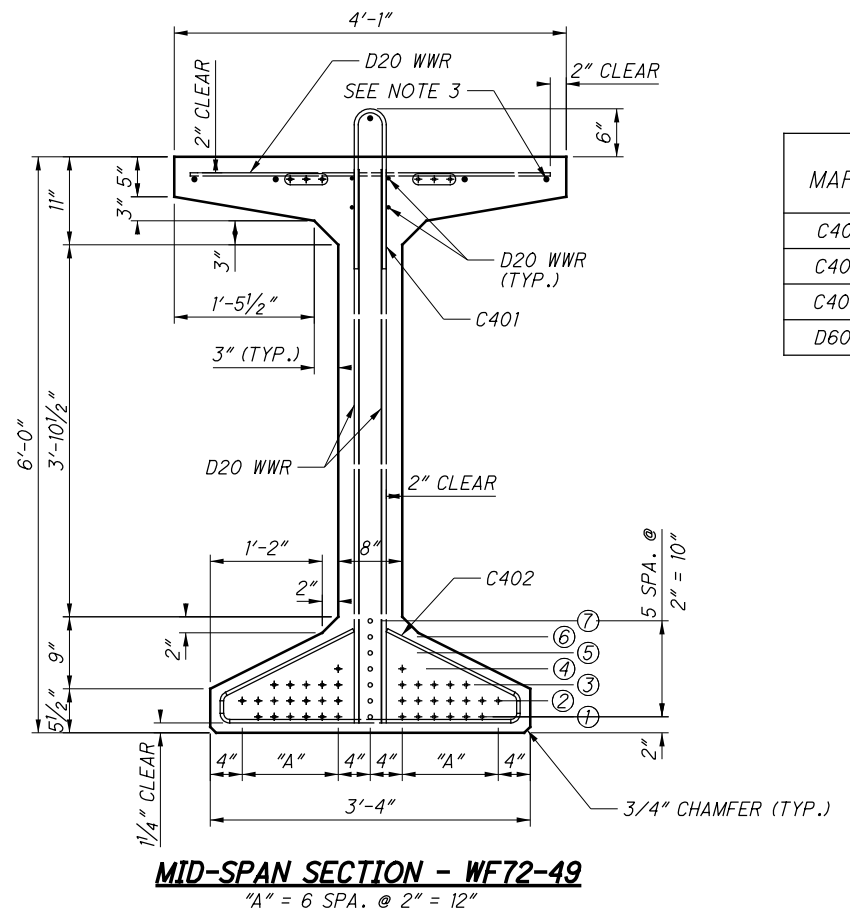
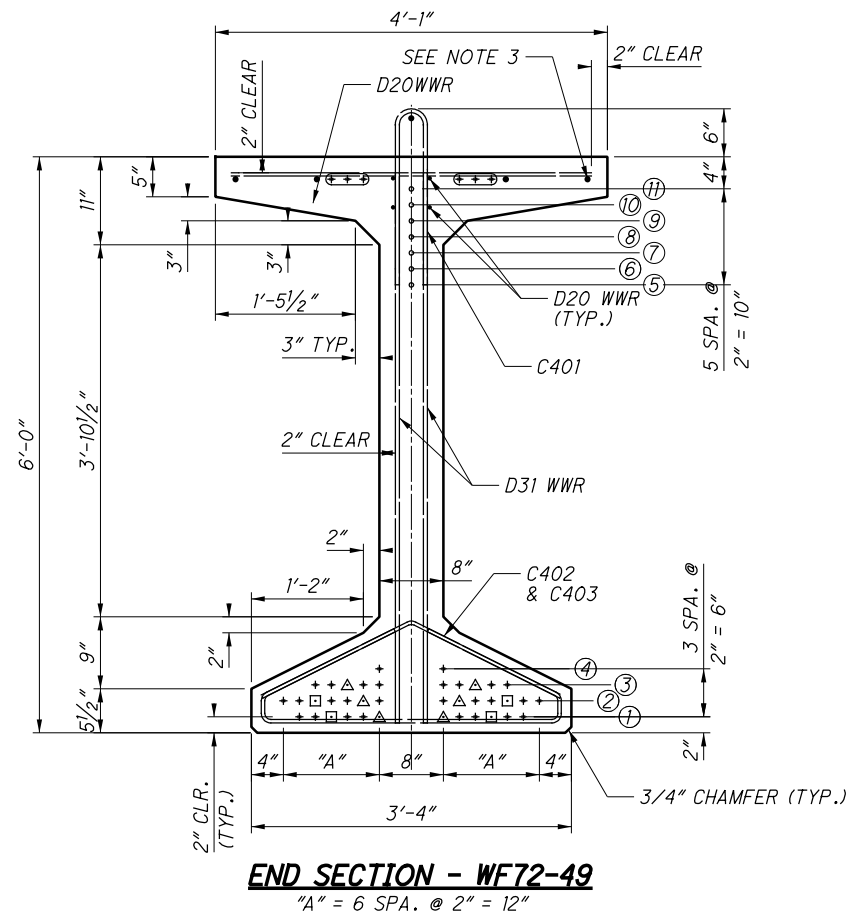


ANCHORAGE REINFORCEMENT DETAIL AT PIERS

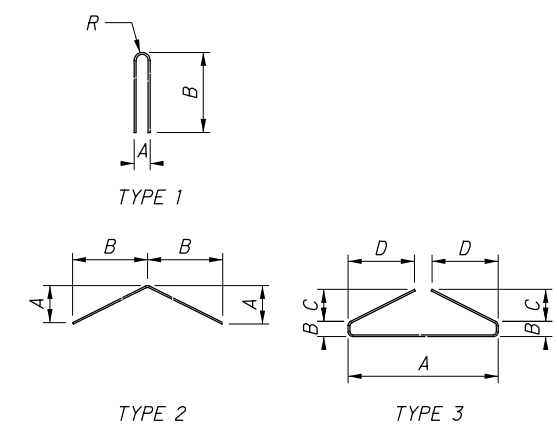
LEGEND:
* - LOCATE BOLT HOLES IN THE I-BEAM WEBS TO AVOID PRESTRESS STRANDS. THE MINIMUM CLEAR DISTANCE SHALL BE 1 1/2"

NOTES:
1. FOR ADDITIONAL DETAILS AND NOTES, SEE ODOT STANDARD DRAWING PSID-1-13.
2. ALL PRESTRESSING STRANDS SHALL BE GRADE 270 KIPS SEVEN WIRE, UNCOATED, LOW RELAXATION STRAND, WITH A DIAMETER OF 0.6 INCHES AND A NOMINAL AREA OF 0.217 SQUARE INCHES.
3. ONLY THE C401 REINFORCING BARS SHALL BE EPOXY COATED, GRADE 60.

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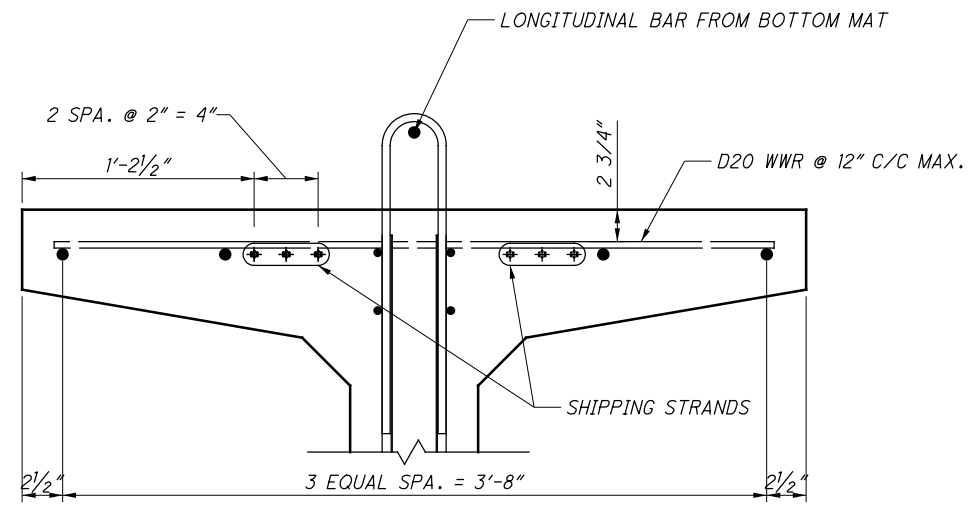


MARK	TYPE	DIMENSIONS					
		A	B	C	D	E	R
C401	1	4"	1'-8"				2"
C402	2	9"	1'-6"				
C403	3	3'-1 1/2"	3 1/2"	7 1/2"	1'-3"		
D601	1	6"	3'-1"				



PRESTRESSED I-BEAM DESIGN TABLE																							
BEAM	NUMBER OF STRANDS PER ROW																TOTAL STRANDS	CONCRETE STRENGTHS			B401 BARS REQ'D	B402 BARS REQ'D	B403 BARS REQ'D
	END SECTION								MID-SPAN SECTION									f'ci	f'c				
	1	2	3	4	5*	6*	7*	8*	9*	10*	11*	1*	2*	3*	4*	5*				6*			
	12	14	10	2	1	1	1	1	1	1	13	15	11	3	1	1	1	45	5.0 KSI	7 KSI	76	114	172

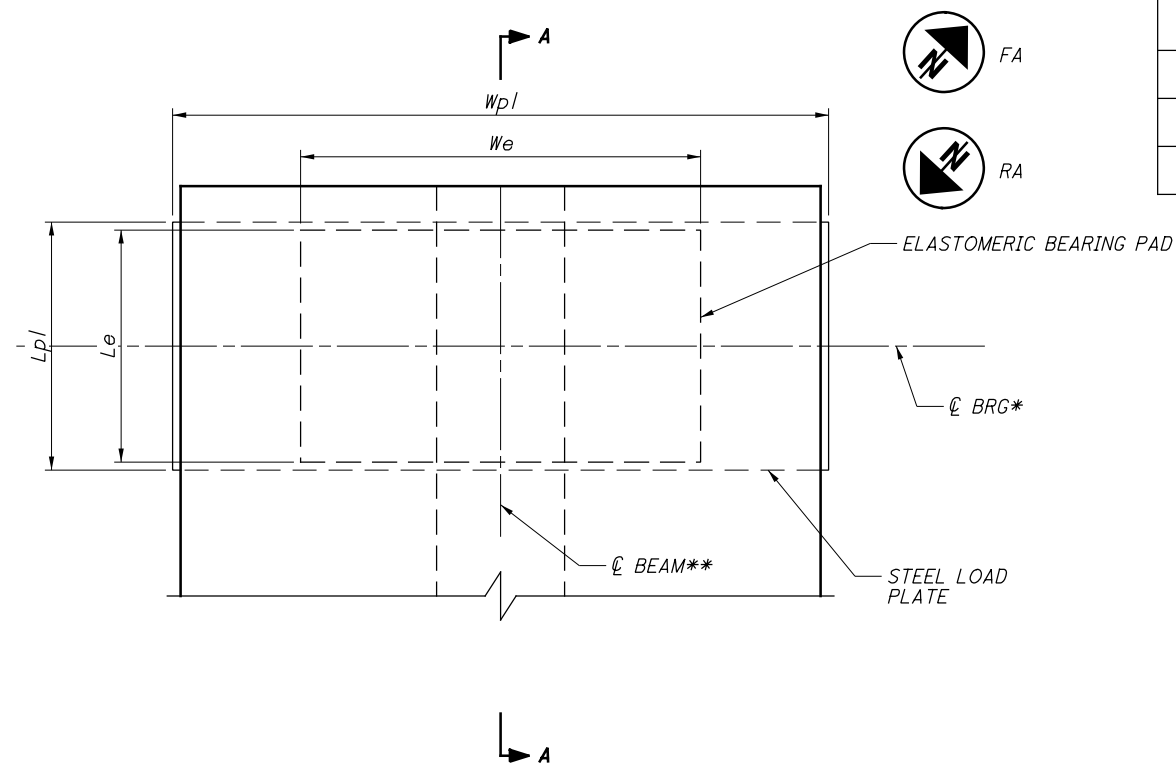
- THE INFORMATION IN THE ABOVE TABLE IS TYPICAL FOR ALL BEAMS.
 - SEE GENERAL NOTES, SHEET 4/64, FOR PRESTRESSING STRAND TYPE AND DESIGN DATA.
 * DENOTES ROWS WHERE DRAPED STRANDS ARE PRESENT



- NOTES:**
- FOR ADDITIONAL DETAILS AND NOTES, SEE ODOT STANDARD DRAWING PSID-1-13.
 - ALL PRESTRESSING STRANDS SHALL BE GRADE 270 KIPS SEVEN WIRE, UNCOATED, LOW RELAXATION STRAND, WITH A DIAMETER OF 0.6 INCHES AND A NOMINAL AREA OF 0.217 SQUARE INCHES.
 - FOUR CONTINUOUS D31 BARS SHALL BE PROVIDED IN THE TOP FLANGE AS SHOWN FOR THE FULL LENGTH OF THE BEAMS PER PSID-1-13.
 - ONLY THE C401 REINFORCING BARS SHALL BE EPOXY COATED, GRADE 60.

- LEGEND:**
- ① - INDICATES STRAND ROW
 - ▲ - STRAND DEBONDED 15'-0"
 - ▣ - STRAND DEBONDED 25'-0"
 - - INDICATED DRAPED STRAND

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BEARING LAYOUT

LOCATION	TYPE	ELASTOMER							STEEL LAMINATES		LOAD PLATE			LOAD (KIPS)		
		DUROMETER	Le	We	Te	ti	te	N	P	t	Lpl	Wpl	Tpl	DL	LL	TOTAL
REAR ABUTMENT	EXP	50	14.5"	25"	3.39"	0.44"	0.3"	6	6	0.0747"	15.5"	41"	2"	215	110	325
FORWARD ABUTMENT	EXP	50	14.5"	25"	3.39"	0.44"	0.3"	6	6	0.0747"	15.5"	41"	2"	230	110	340

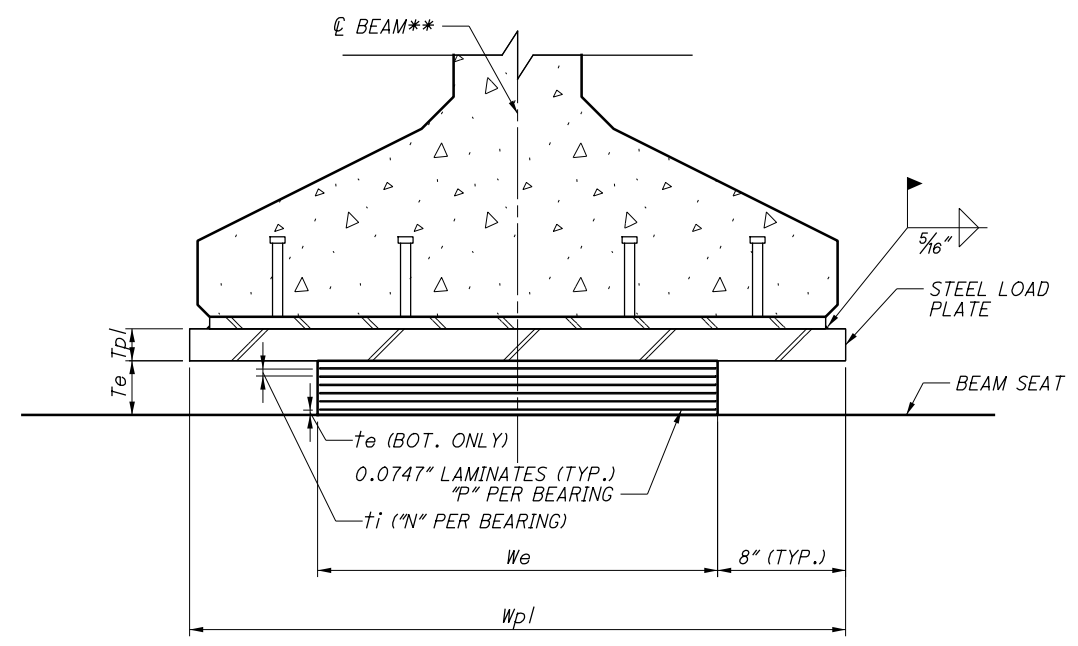
L_e - LENGTH OF LAMINATED ELASTOMERIC BEARING
 W_e - WIDTH OF LAMINATED ELASTOMERIC BEARING
 T_e - TOTAL THICKNESS OF LAMINATED ELASTOMERIC BEARING
 t_i - THICKNESS OF INTERNAL ELASTOMER LAYER
 t_e - THICKNESS OF EXTERNAL ELASTOMER LAYER
 N - NUMBER OF INTERNAL ELASTOMER LAYERS
 t - THICKNESS OF STEEL LAMINATES
 P - NUMBER OF STEEL LAMINATES

NOTES

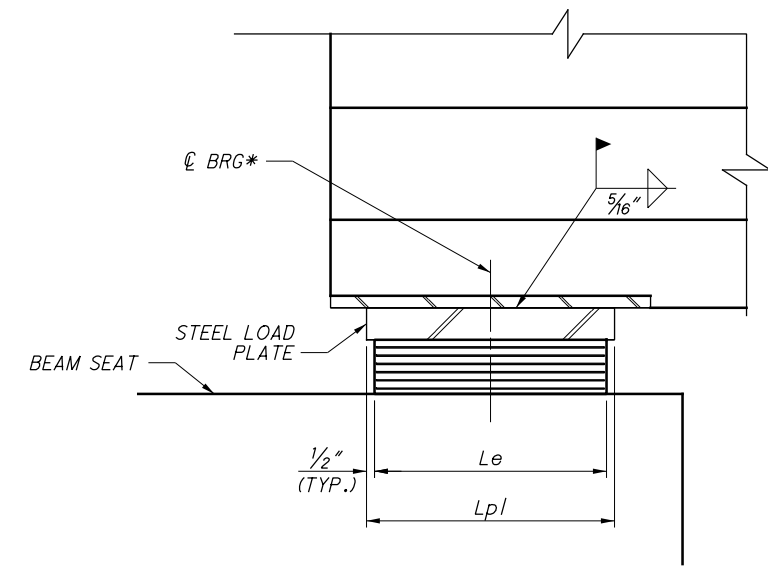
- LOAD PLATE:**
THE STEEL LOAD PLATE SHALL BE BONDED BY VULCANIZATION TO THE ELASTOMER DURING THE MOLDING PROCESS.
- ELASTOMERIC BEARING:**
THE ELASTOMER SHALL HAVE A HARDNESS OF 50 DUROMETER. THE BEARINGS WERE DESIGNED UNDER DIVISION I, SECTION 14.7.6 (METHOD A) OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THE LONG TERM COMPRESSION PROOF LOAD TEST (AASHTO STANDARD SPECIFICATION FOR HIGHWAY BRIDGES, DIVISION II, SECTION 18.7.2.6) IS NOT REQUIRED.
- STEEL FOR LOAD PLATES AND PEDESTAL SHALL BE ASTM A709 GRADE 50.
- ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE, AND A DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND BE VISIBLE AFTER THE BEARING IS INSTALLED.

LEGEND

- * - DIMENSIONS SYMMETRICAL ABOUT CL BRG (& BRG ASSEMBLY)
- ** - DIMENSIONS SYMMETRICAL ABOUT CL BEAM



BEARING ELEVATION



SECTION A-A

1800 INDIAN WOOD CIRCLE
 MAUMEE, OHIO 43537

REVIEWED TLR
 DATE 05/2015
 STRUCTURE FILE NUMBER TBD

DRAWN KRH
 REVISIONS

DESIGNED KRH
 CHECKED SCT

ABUTMENT BEARING
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
 PID No. 22984

33/64

117
 180

		ELASTOMER								STEEL LAMINATES		LOAD PLATE			LOAD (KIPS)		
LOCATION		TYPE	DUROMETER	Le	We	Te	ti	te	N	P	t	Lpl	Wpl	Tpl	DL	LL	TOTAL
PIERS 1 THRU 6	BRG 1	EXP	50	14"	22"	2.36"	0.44"	0.3"	4	4	0.0747"	15"	41"	2"	203	110	313
	BRG 2	EXP	50	14"	22"	2.36"	0.44"	0.3"	4	4	0.0747"	15"	41"	2 3/16"	203	110	313
PIERS 7	BRG 1	EXP	50	14"	22"	2.36"	0.44"	0.3"	4	4	0.0747"	15"	41"	2"	203	110	313
	BRG 2	EXP	50	14"	22"	2.36"	0.44"	0.3"	4	4	0.0747"	15"	41"	2"	209	110	319

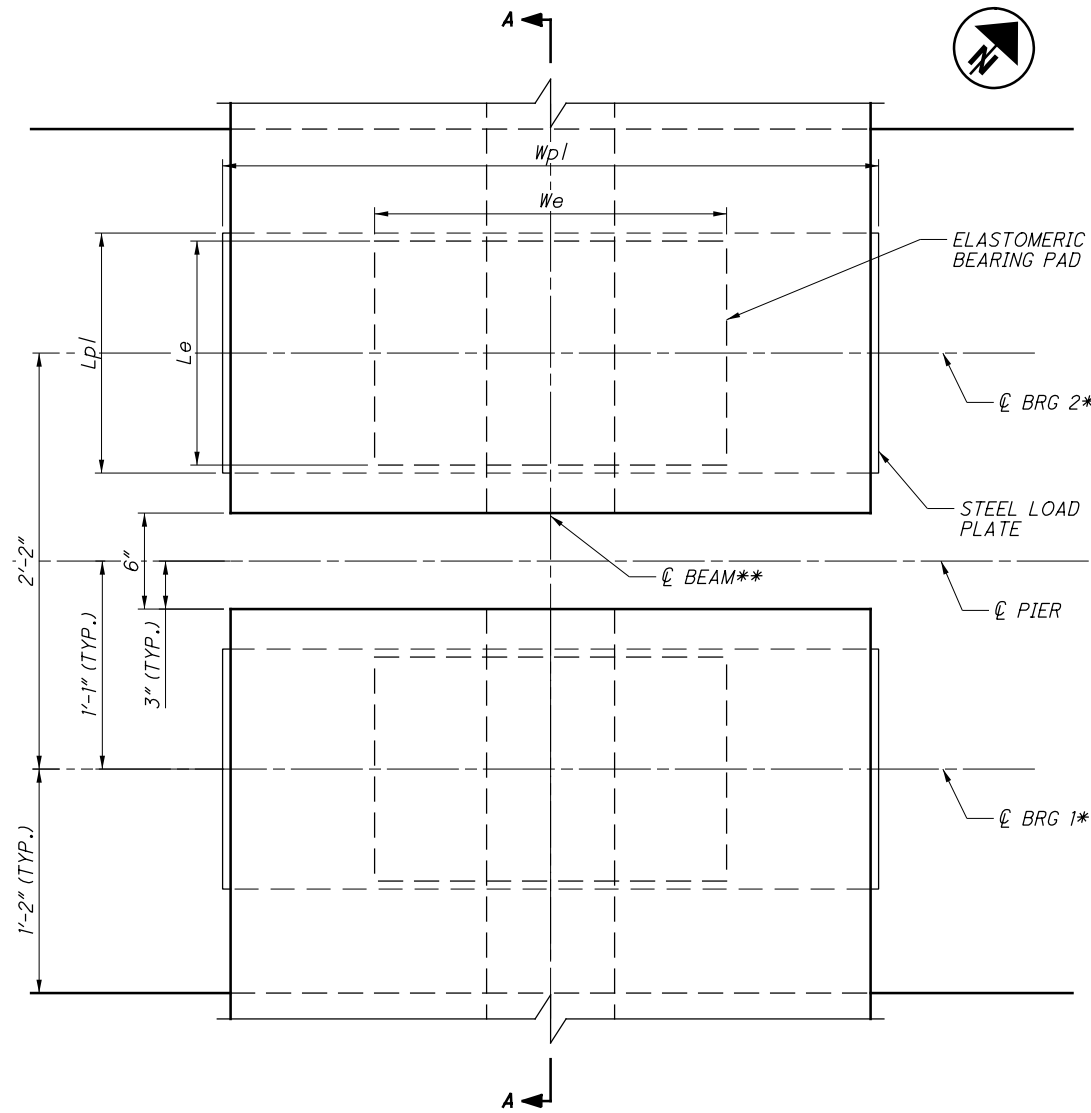
Le - LENGTH OF LAMINATED ELASTOMERIC BEARING
 We - WIDTH OF LAMINATED ELASTOMERIC BEARING
 Te - TOTAL THICKNESS OF LAMINATED ELASTOMERIC BEARING
 ti - THICKNESS OF INTERNAL ELASTOMER LAYER
 te - THICKNESS OF EXTERNAL ELASTOMER LAYER
 N - NUMBER OF INTERNAL ELASTOMER LAYERS
 t - THICKNESS OF STEEL LAMINATES
 P - NUMBER OF STEEL LAMINATES

NOTES

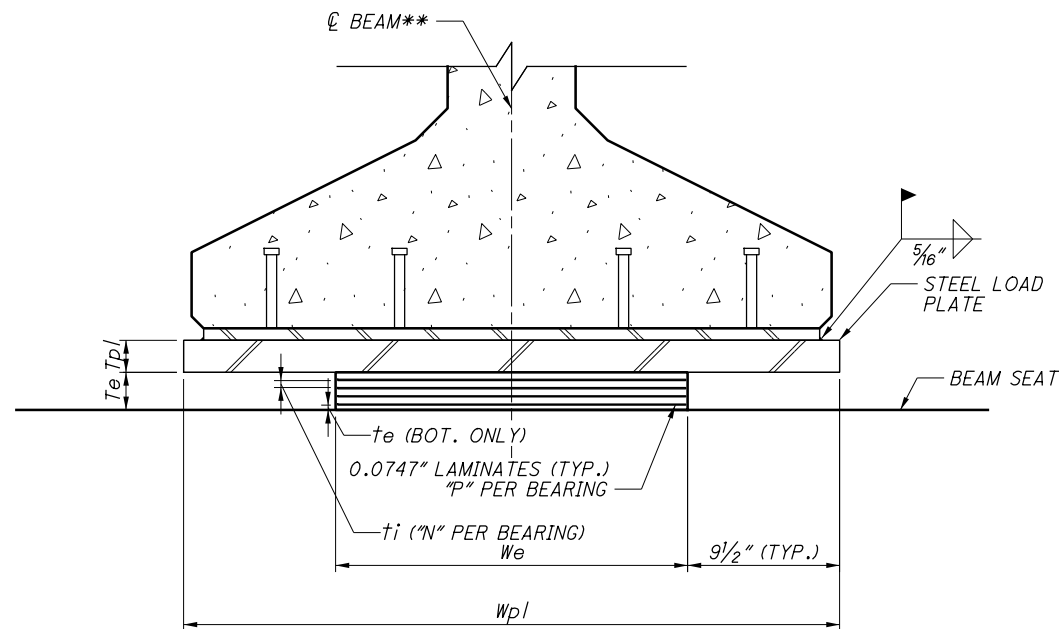
- LOAD PLATE:**
THE STEEL LOAD PLATE SHALL BE BONDED BY VULCANIZATION TO THE ELASTOMER DURING THE MOLDING PROCESS.
- ELASTOMERIC BEARING:**
THE ELASTOMER SHALL HAVE A HARDNESS OF 50 DUROMETER. THE BEARINGS WERE DESIGNED UNDER DIVISION I, SECTION 14.7.6 (METHOD A) OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THE LONG TERM COMPRESSION PROOF LOAD TEST (AASHTO STANDARD SPECIFICATION FOR HIGHWAY BRIDGES, DIVISION II, SECTION 18.7.2.6) IS NOT REQUIRED.
- STEEL FOR LOAD PLATES AND PEDESTAL SHALL BE ASTM A709 GRADE 50W.
- ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE, AND A DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND BE VISIBLE AFTER THE BEARING IS INSTALLED.

LEGEND

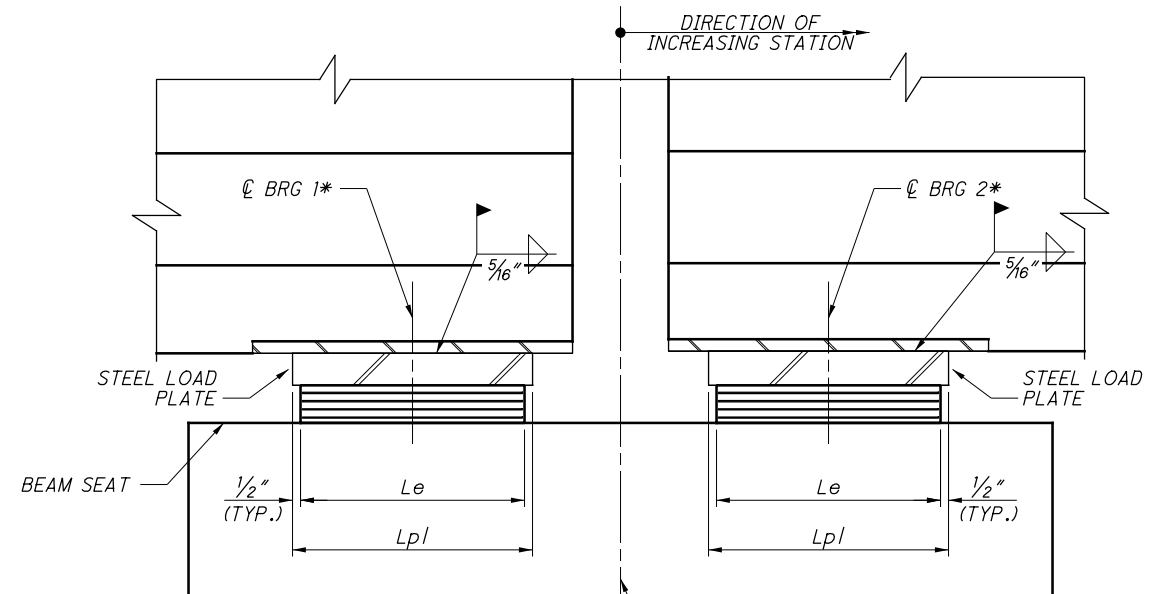
- * - DIMENSIONS SYMMETRICAL ABOUT \varnothing BRG (& BRG ASSEMBLY)
- ** - DIMENSIONS SYMMETRICAL ABOUT \varnothing BEAM



BEARING LAYOUT



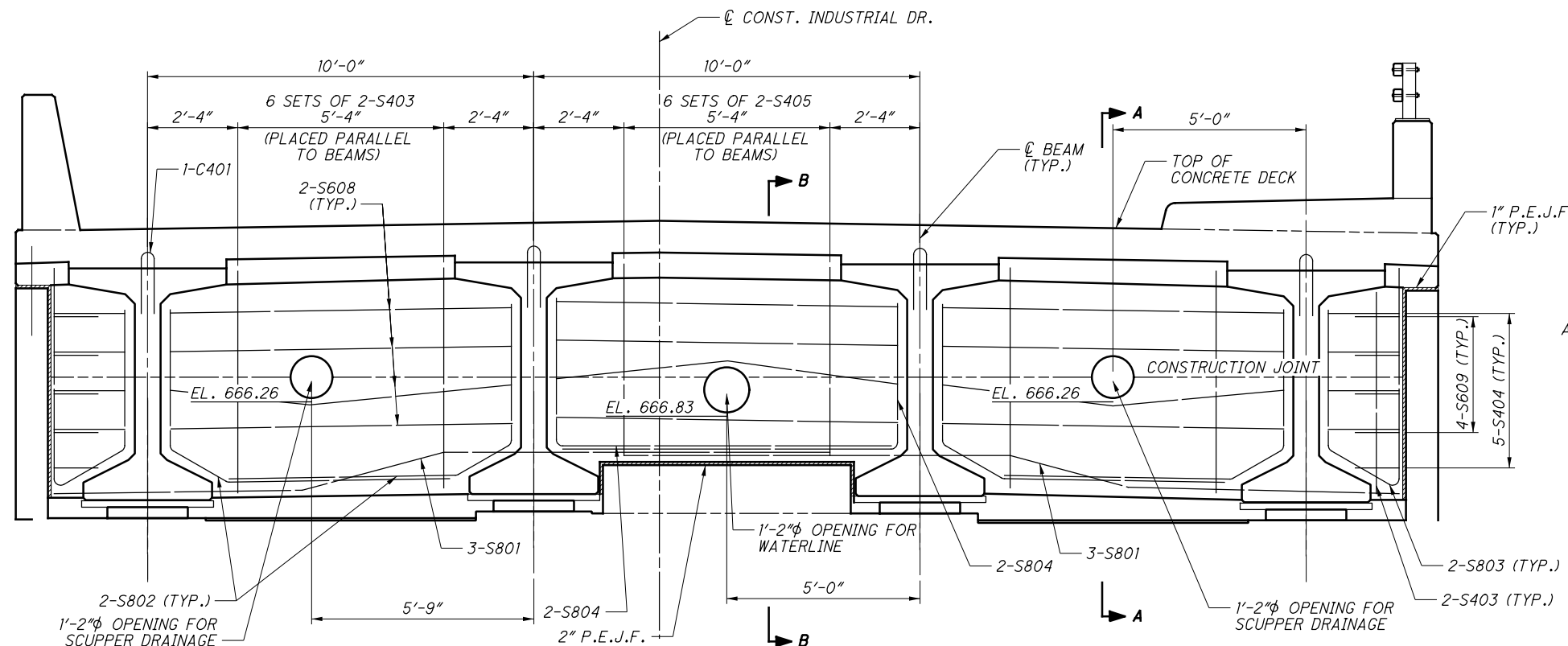
BEARING ELEVATION



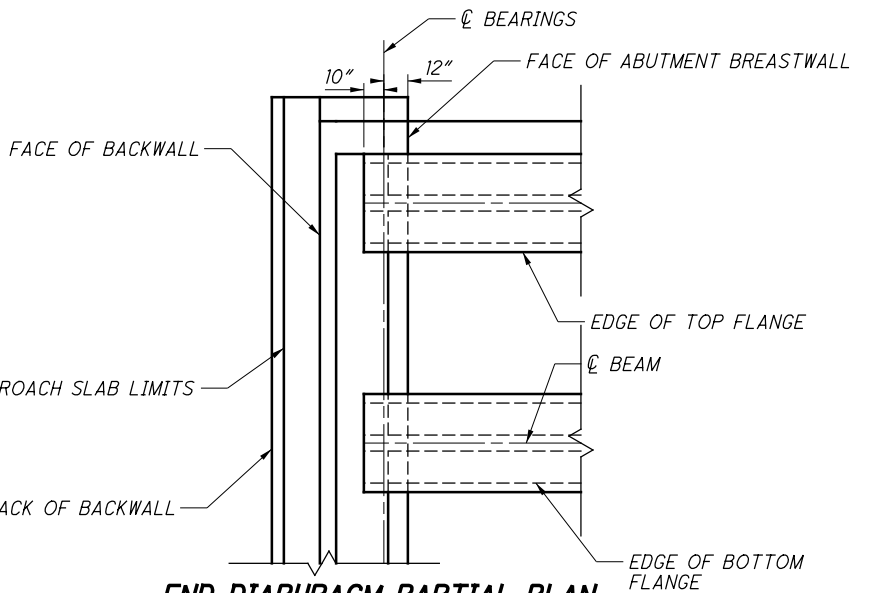
SECTION A-A

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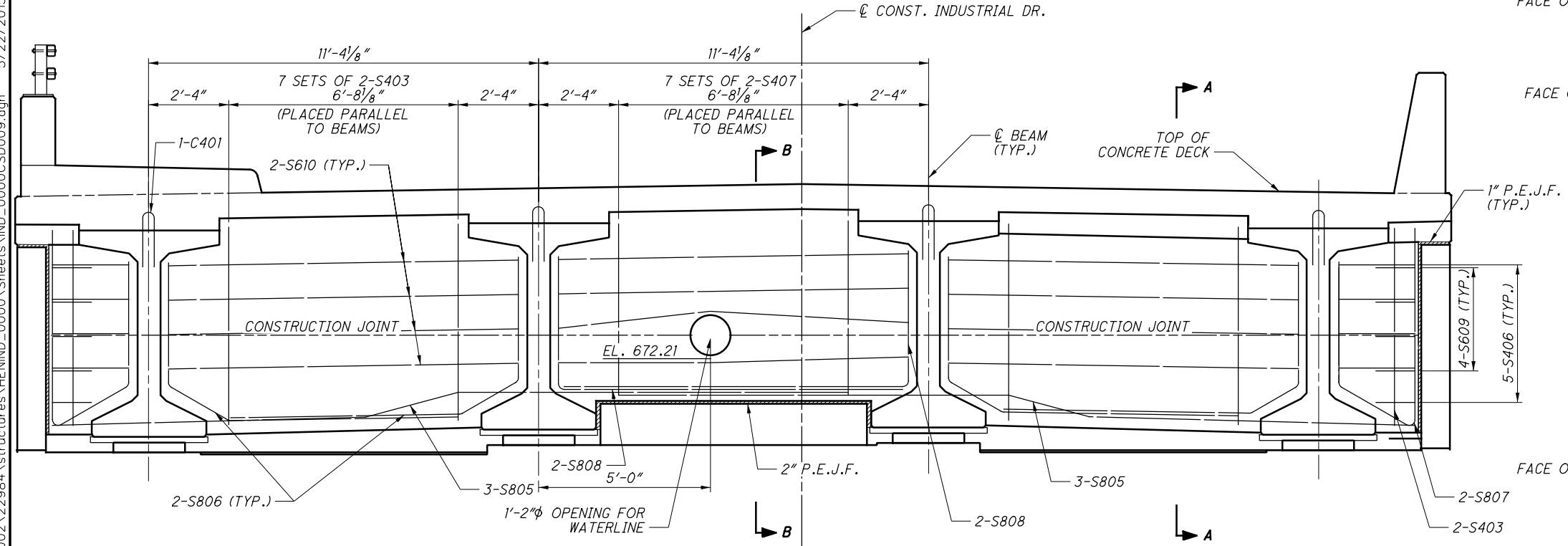
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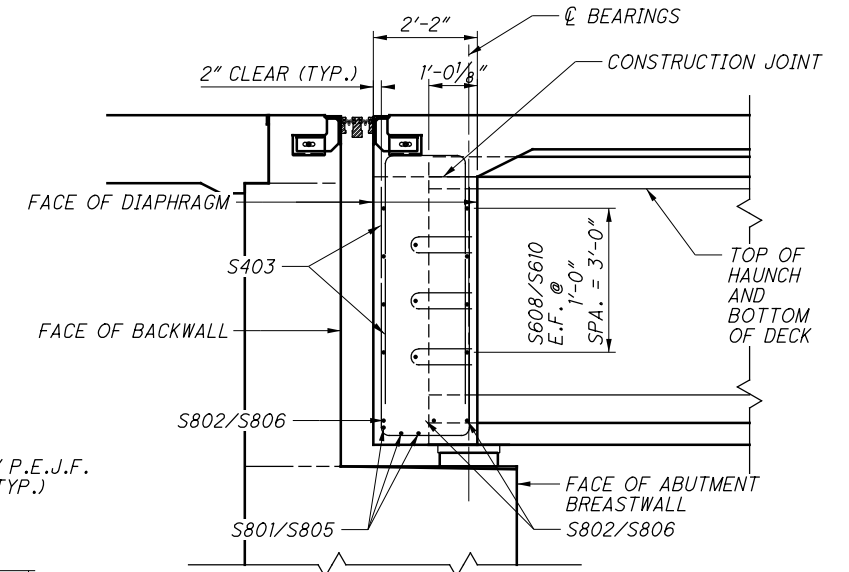
REAR ABUTMENT END DIAPHRAGM ELEVATION
(LOOKING DOWNSTATION)



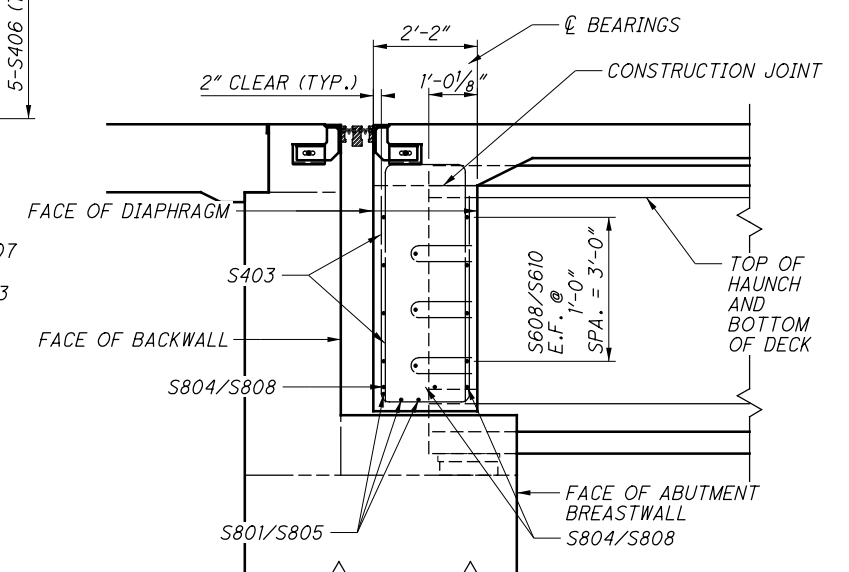
END DIAPHRAGM PARTIAL PLAN
(REAR ABUTMENT SHOWN, FORWARD ABUTMENT SIMILAR)
(MODULAR EXPANSION JOINT NOT SHOWN FOR CLARITY)



FORWARD ABUTMENT END DIAPHRAGM ELEVATION
(LOOKING UPSTATION)



SECTION A-A



SECTION B-B

- NOTES:**
1. FOR ADDITIONAL DETAILS AND NOTES NOT SHOWN, SEE ODOT STANDARD DRAWING PSID-1-13.
 2. FIELD BEND #6 BARS AROUND SUPPER AND WATERLINE OPENINGS.
 3. FOR BEARING DETAILS, SEE SHEET [33/64].
 4. PLACE VERTICAL BARS PARALLEL TO BEAMS.
 5. ABUTMENT DIAPHRAGM, PRESTRESSED I-BEAM SUPERSTRUCTURE: PLACE THE CONCRETE ENCASEING THE PRESTRESSED I-BEAM STRUCTURAL MEMBERS AS PART OF THE DECK POUR.

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

DESIGNED	DATE
CHKD	05/2015
DRAWN	FILE NUMBER
REVISED	TBD

END DIAPHRAGM DETAILS
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

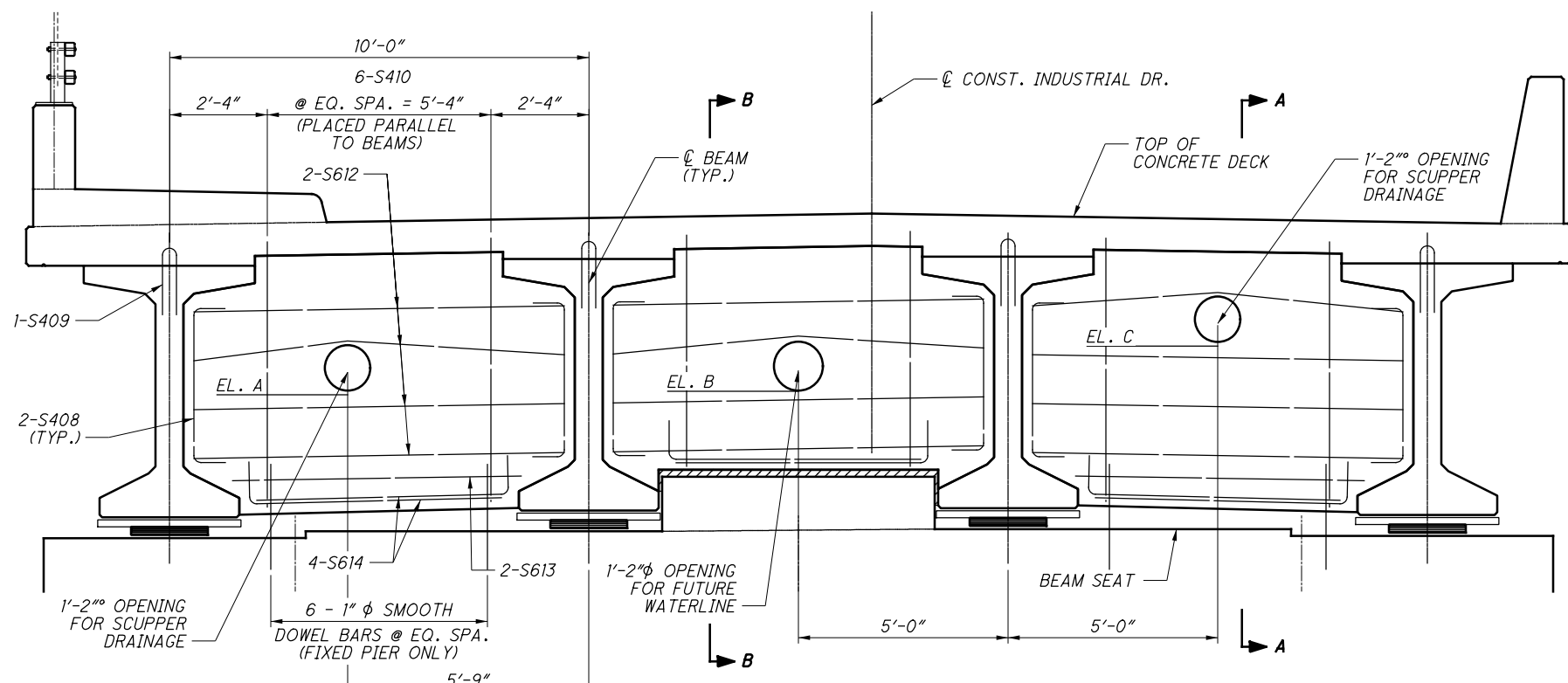
35 / 64

119

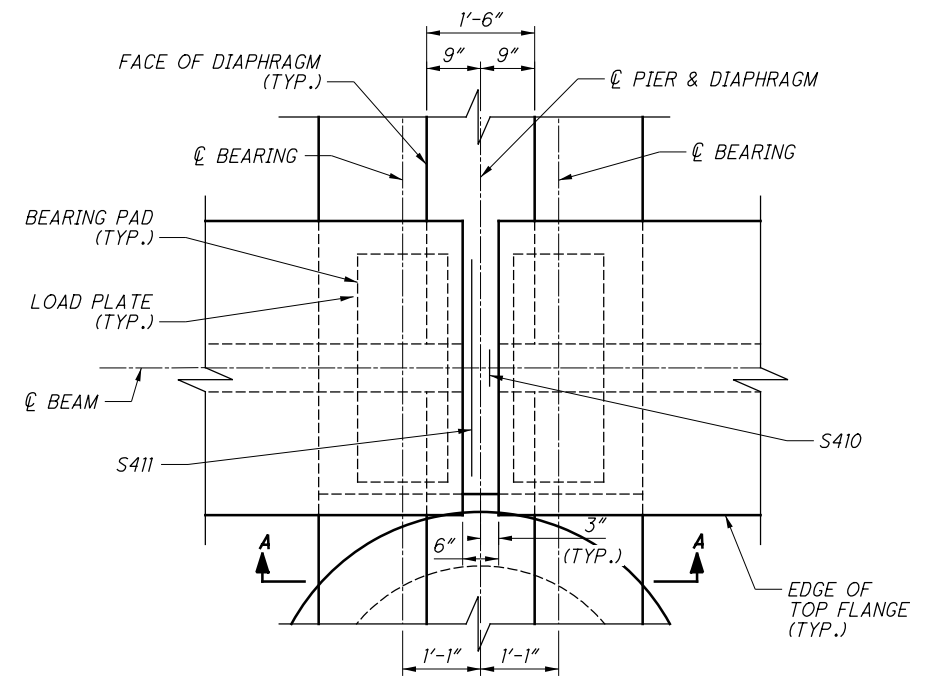
180

SCUPPER OPENING LOCATION		
PIER	EL. A	EL. C
1	667.43	668.59
2	668.60	-
3	669.76	-
4	670.96	-
5	672.14	-

WATERLINE OPENING LOCATION	
PIER	EL. B
1	666.62
2	667.42
3	668.22
4	669.02
5	669.82
6	670.62
7	671.42

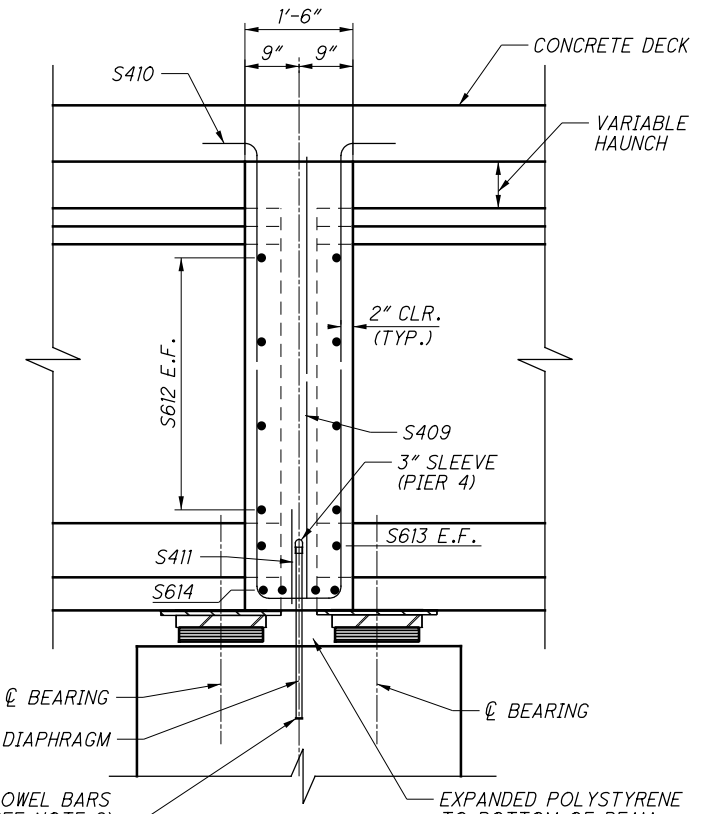


PIER DIAPHRAGM PART ELEVATION
(6 BAYS, LOOKING UPSTATION)

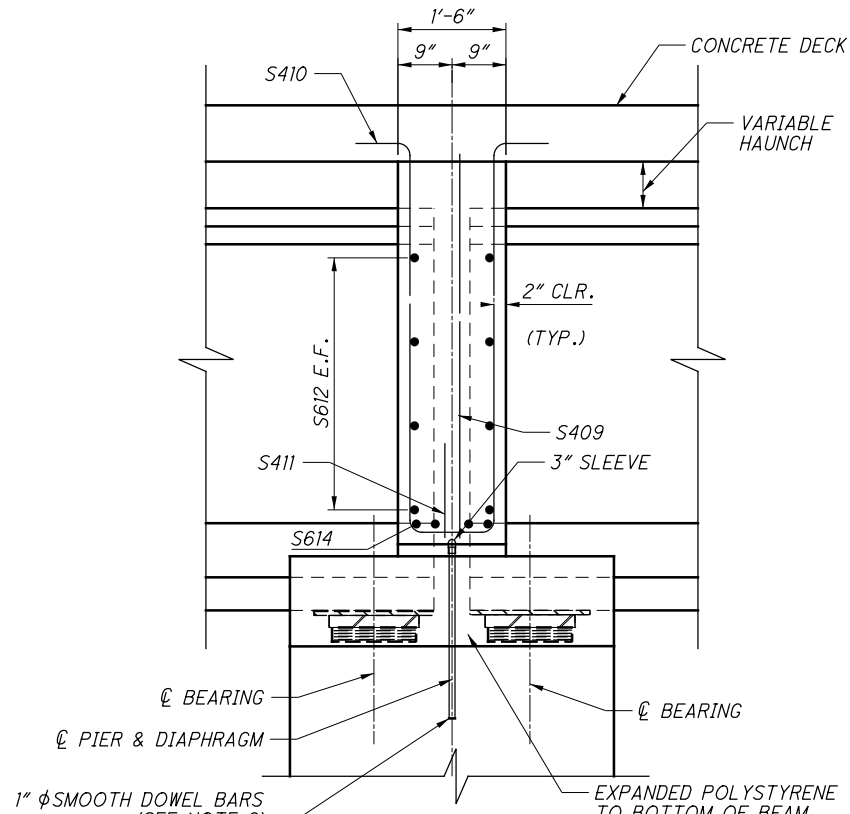


PIER PARTIAL PLAN

- NOTES:**
- FOR ADDITIONAL DETAILS AND NOTES NOT SHOWN, SEE ODOT STANDARD DRAWING PSID-1-13.
 - 1" ϕ SMOOTH DOWEL BARS ARE ASTM A311 CLASS A, GRADE 1018, WITH SLEEVE. (INSTALL DOWEL ACCORDING TO ITEM 510 DOWEL HOLES WITH NONSHRINK, NON-METALLIC GROUT, 705.20.) INCLUDED WITH ITEM 515, DRAPED STRAND PRESTRESSED CONCRETE BRIDGE I-BEAM MEMBERS, LEVEL 3, TYPE 4 WF 72-49 FOR PAYMENT.
 - FIELD BEND #6 BARS TO AVOID WATERLINE AND SUPPER OPENINGS.
 - BRIDGE SEAT REINFORCING, SETTING ANCHORS: ACCURATLY PLACE REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT TO AVOID INTERFERENCE WITH THE DRILLING OF ANCHOR HOLES OR THE PRESETTING OF ANCHORS.



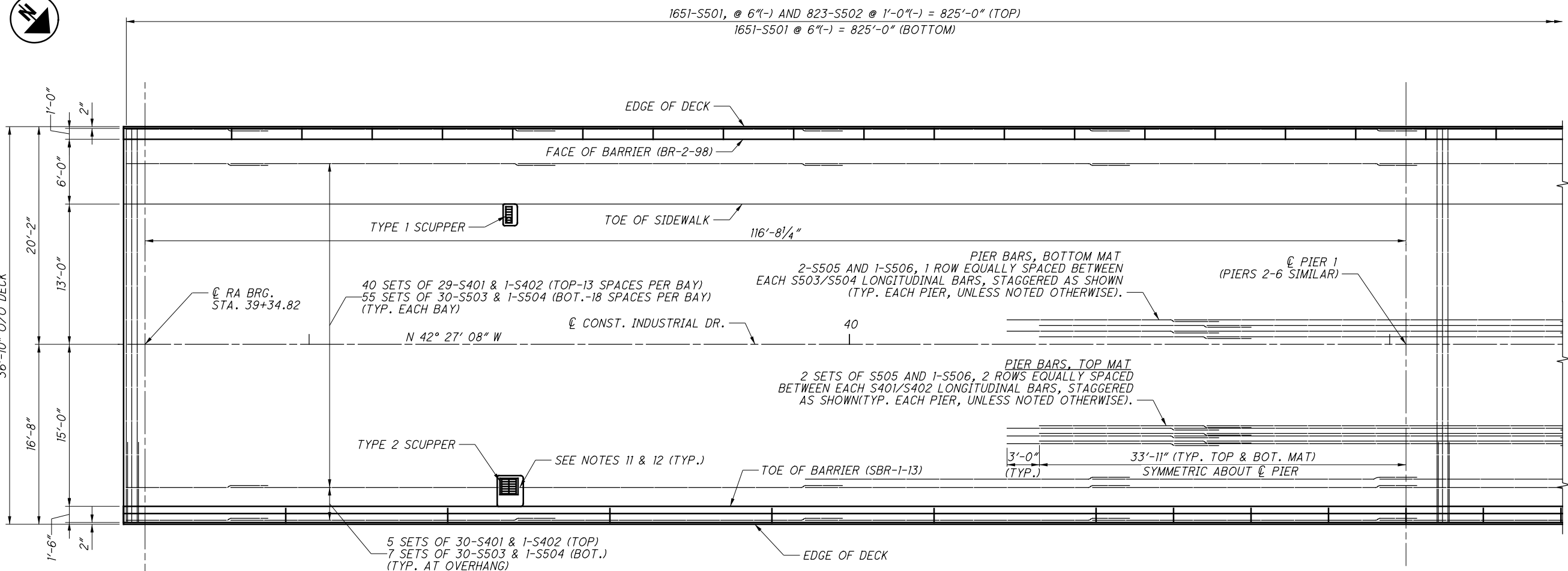
SECTION A-A



SECTION B-B

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DECK REINFORCING REQUIRED LAP LENGTHS	
NO. 4 BARS	2'-7"
NO. 5 BARS	3'-2"
NO. 6 BARS	3'-10"

PARTIAL DECK PLAN

NOTES:

- SPANS 2 - 7 NOT SHOWN.
- FOR SPAN 8, SEE SHEET 38/64.
- FOR RAILING DETAILS, SEE SHEETS 49-52/64.
- FOR TRANSVERSE SECTION, SEE SHEETS 39-40/64.
- FOR APPROACH SLAB DETAILS, SEE SHEETS 57-58/64.
- FOR SIDEWALK DETAILS, SEE SHEET 53/64.
- FOR SCREED AND TOP OF HAUNCH ELEVATIONS, SEE SHEETS 42-44/64.
- FOR FINAL DECK SURFACE ELEVATIONS, SEE SHEETS 45-46/64.
- FOR EXPANSION JOINT DETAILS, SEE SHEET 54-56/64.
- DECK POUR SEQUENCE SHALL BE AS PER STD. DWG. PSID-1-13.
- CUT REINFORCING BARS AROUND SCUPPER LOCATIONS (ALLOW 3" COVER) AND REPAIR BAR ENDS PER CMS 509.09.
- FOR SCUPPER DETAILS, SEE SHEET 59/64.
- DECK SLAB CONCRETE QUANTITY: THE ESTIMATED QUANTITY OF DECK SLAB CONCRETE IS BASED ON THE CONSTANT DECK SLAB THICKNESS THAT FORMS EACH BEAM/GIRDER HAUNCH. THE ESTIMATE ASSUMES A VARIABLE HAUNCH THICKNESS OF MINIMUM 2". DEVIATE FROM THIS HAUNCH THICKNESS AS NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED GRADE.

THE HAUNCH THICKNESS WAS MEASURED AT THE CENTERLINE OF THE BEAM/GIRDER, FROM THE SURFACE OF THE DECK TO THE TOP OF THE TOP FLANGE MINUS THE DECK SLAB THICKNESS.

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

Mannik Smith GROUP

DATE: 05/2015
REVIEWED: TLR
STRUCTURE FILE NUMBER: TBD
DRAWN: ANK
CHECKED: SCT
DESIGNED: KRH

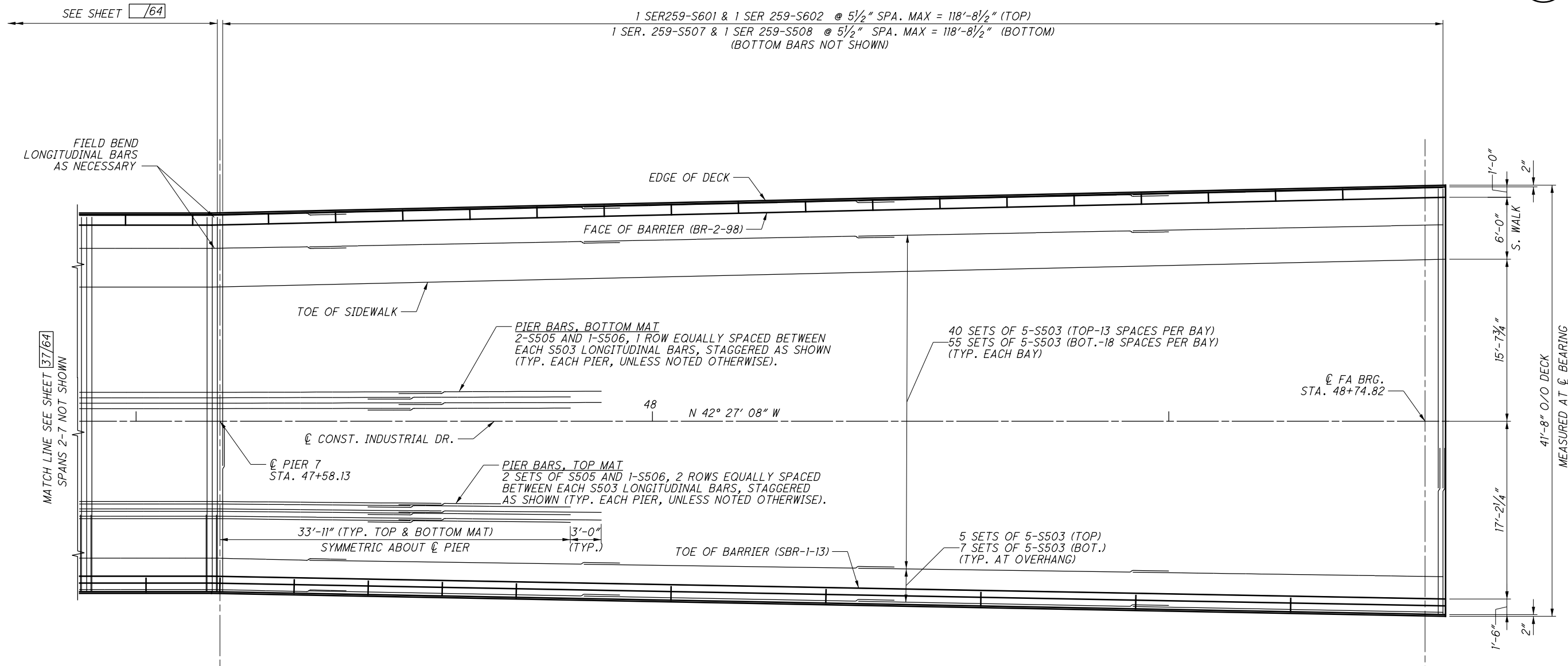
DECK PLAN (1 OF 2)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

37/64

121
180

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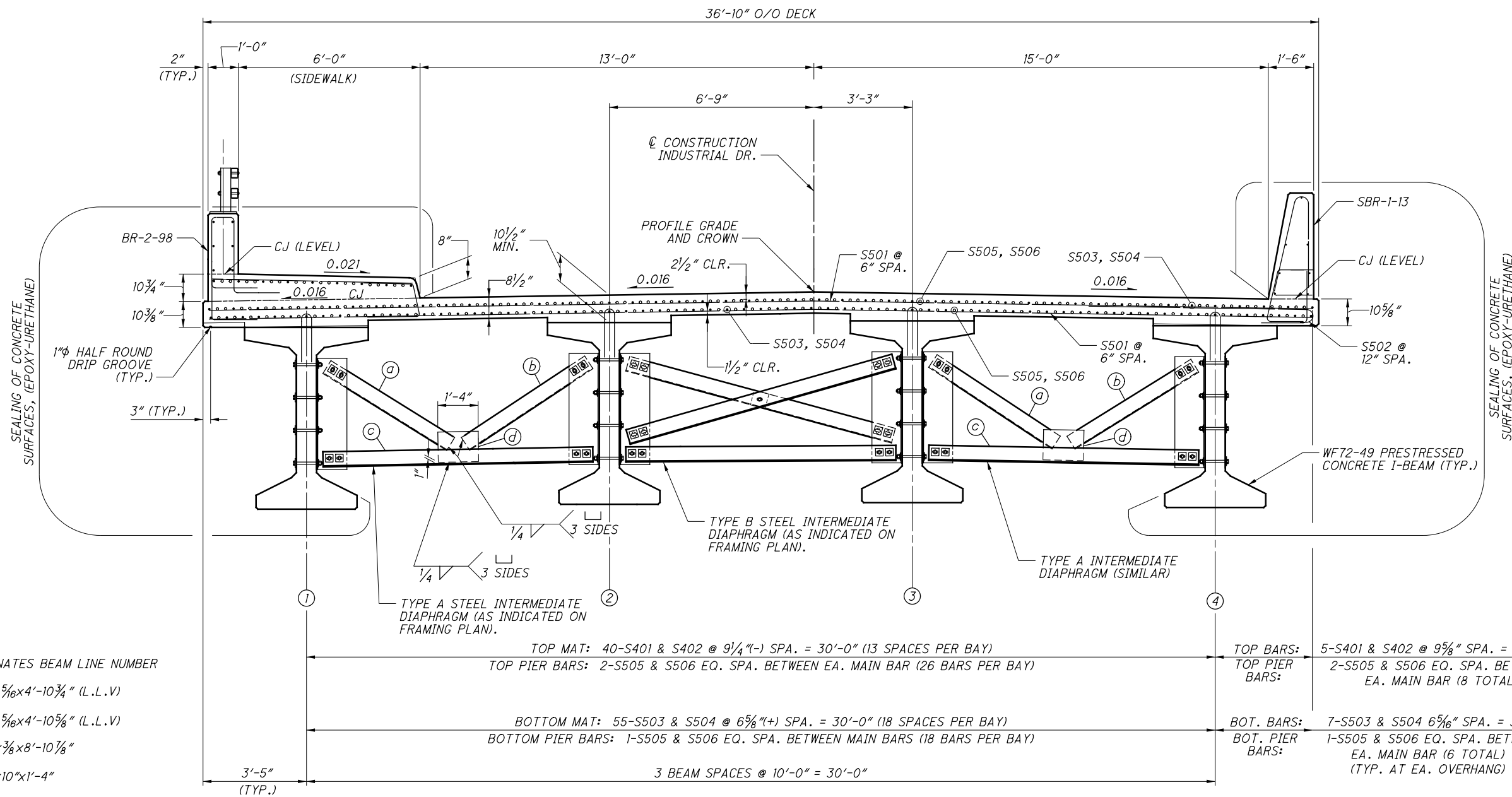
PARTIAL DECK PLAN

DECK REINFORCING REQUIRED LAP LENGTHS	
NO. 4 BARS	2'-7" MIN.
NO. 5 BARS	3'-2" MIN.
NO. 6 BARS	3'-10" MIN.

NOTES:
 1. FOR NOTES, SEE SHEET 37/64.



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LEGEND:

- ⊕ - DESIGNATES BEAM LINE NUMBER
- Ⓐ - L6x4x5/16x4'-10 3/4" (L.L.V)
- Ⓑ - L6x4x5/16x4'-10 5/8" (L.L.V)
- Ⓒ - L6x6x3/8x8'-10 7/8"
- Ⓓ - ∅ 5/8"x10"x1'-4"

DECK REINFORCING REQUIRED LAP LENGTHS	
NO. 4 BARS	2'-7" MIN.
NO. 5 BARS	3'-2" MIN.
NO. 6 BARS	3'-10" MIN.

**TRANSVERSE SECTION
(TYPICAL OF SPANS 1 - 7)**

NOTES:

1. FOR TRANSVERSE SECTIONS ALONG SPAN 8, SEE SHEET 40/64.
2. DECK SLAB THICKNESS FOR CONCRETE QUANTITY: THE TOPPING THICKNESSES SHOWN FROM THE TOP OF DECK SLAB TO THE TOP OF THE TOP FLANGE ALONG THE CENTERLINE OF THE I-BEAM ARE THEORETICAL DIMENSIONS. THE HAUNCH DEPTH IS THE TOPPING THICKNESS MINUS THE DESIGN SLAB THICKNESS. THE DEPARTMENT WILL PAY FOR SUPERSTRUCTURE CONCRETE BASED ON THE DESIGN SLAB THICKNESS AND THE AVERAGE OF THE THEORETICAL HAUNCH DEPTHS AT MID-SPAN AND AT EACH BEAM BEARING EVEN THOUGH THE DEVIATION FROM THE DIMENSIONS SHOWN MAY BE NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED GRADE. ONCE ALL BEAMS ARE SET IN THEIR FINAL POSITION, THE ACTUAL CAMBER FOR EACH MEMBER WILL BE THE TOP OF THE BEAM ELEVATION AT THE MID-SPAN MINUS THE AVERAGE TOP OF BEAM ELEVATION AT EACH BEARING. THE ACTUAL TOPPING THICKNESS AT MID-SPAN WILL BE THE THEORETICAL DIMENSION PLUS OR MINUS THE DIFFERENCE BETWEEN THE ACTUAL AND ANTICIPATED CAMBER.
3. FOR DECK PLAN, SEE SHEETS 37,38/64.
4. FOR FRAMING PLAN, SEE SHEETS 28-30/64.
5. FOR RAILING DETAILS, SEE SHEETS 49-52/64.
6. FOR SIDEWALK DETAILS, SEE SHEET 53/64.
7. FOR PRESTRESSED I-BEAM DETAILS, SEE SHEET 31,32/64.
8. FOR "TYPE B" STEEL INTERMEDIATE DIAPHRAGM DETAILS, SEE STD. DWG. PSID-1-13.
9. FOR "TYPE A" STEEL INTERMEDIATE DIAPHRAGM DETAILS, USE STD. DWG. PSID-1-13 EXCEPT AS DETAILED HEREIN.

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1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

DESIGNED: KRH
CHECKED: SCT

DRAWN: ANK
REVISED:

REVIEWED: TLR
STRUCTURE FILE NUMBER: TBD

DATE: 05/2015

TRANSVERSE SECTION (1 OF 2)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

39/64

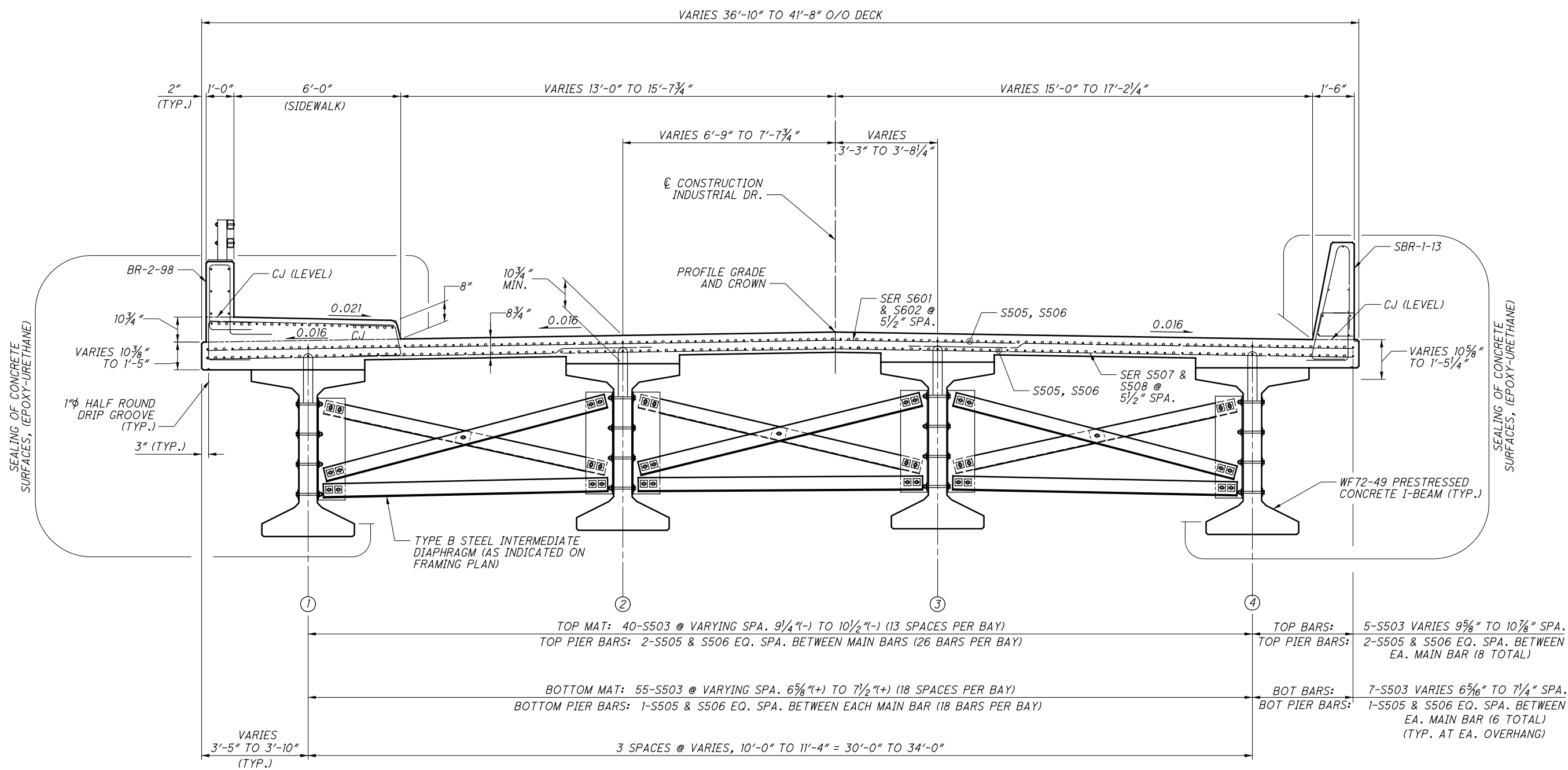
123
180

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DESIGNED	KRH	CHECKED	SCT
DRAWN	ANK	REVISED	
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	05/2015		

TRANSVERSE SECTION (2 OF 2)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984



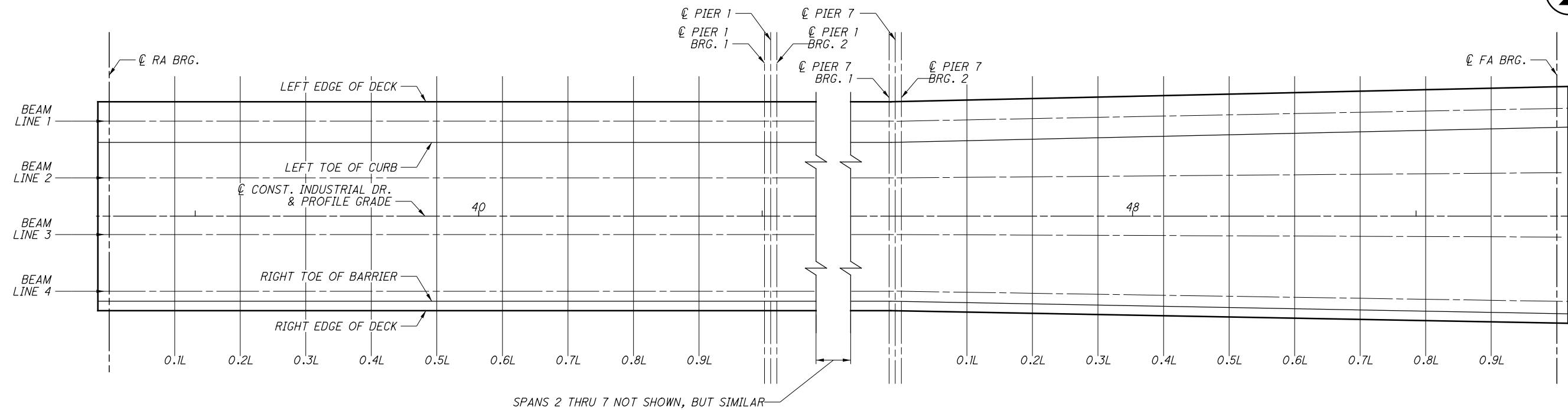
**TRANSVERSE SECTION
SPAN 8**

DECK REINFORCING REQUIRED LAP LENGTHS	
NO. 4 BARS	2'-7" MIN.
NO. 5 BARS	3'-2" MIN.
NO. 6 BARS	3'-10" MIN.

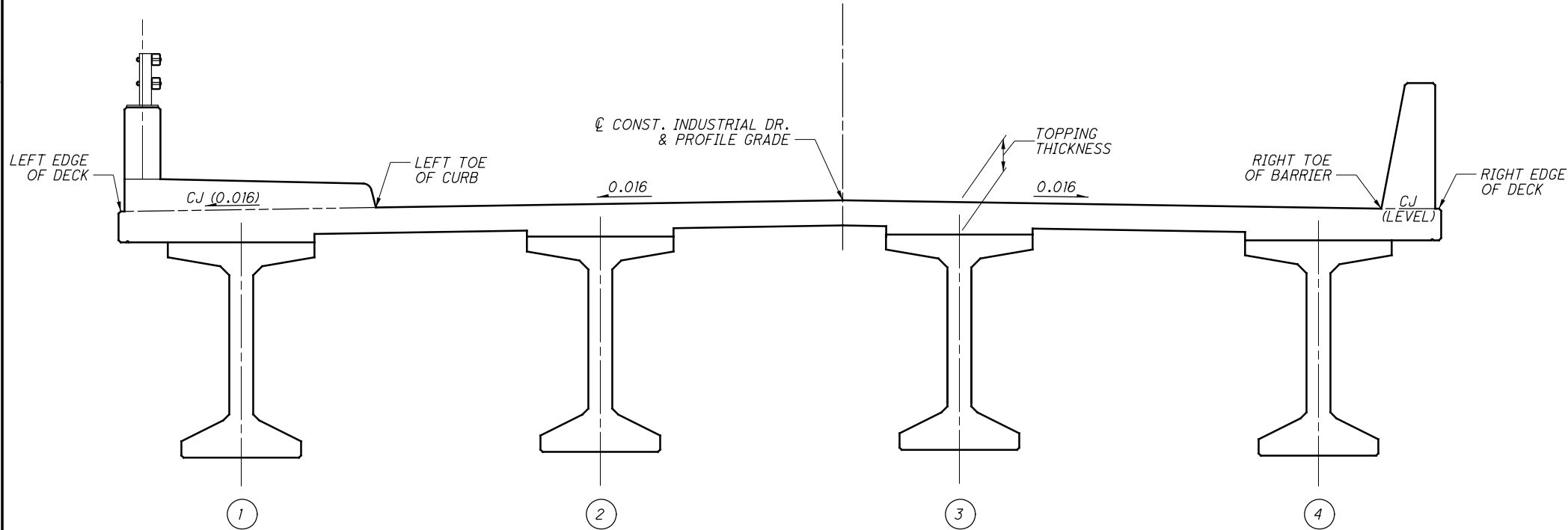
LEGEND:
⊕ - DESIGNATES BEAM LINE NUMBER

NOTES:
1. FOR TRANSVERSE SECTION ALONG SPANS 1 - 7, AND TRANSVERSE SECTION NOTES, SEE SHEET 39/64.

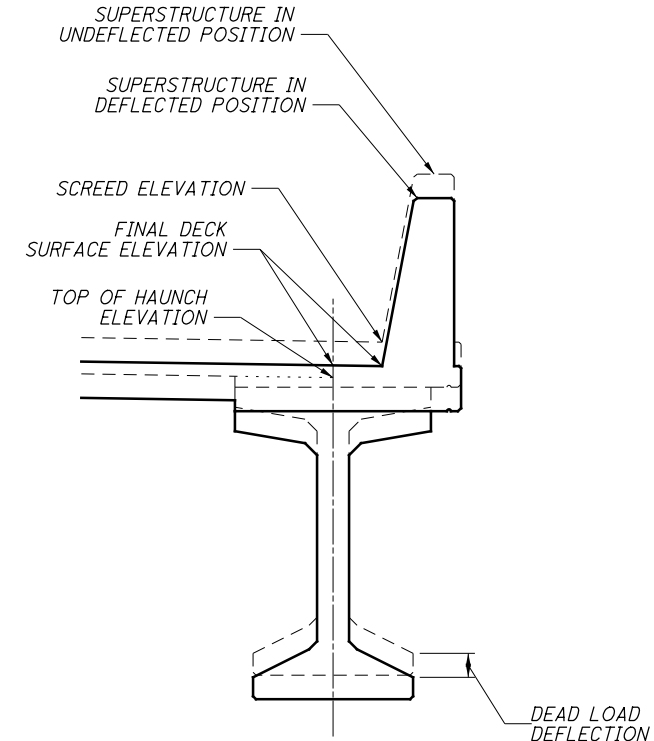
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KEY PLAN



TYPICAL CROSS SECTION



SCREED AND HAUNCH LOCATION

NOTES:

- DECK SLAB THICKNESS FOR CONCRETE QUANTITY: THE TOPPING THICKNESSES SHOWN FROM THE TOP OF THE DECK SLAB TO THE TOP OF THE TOP FLANGE ALONG THE CENTERLINE OF THE I-BEAM ARE THEORETICAL DIMENSIONS. THE HAUNCH DEPTH CONCRETE BASED ON THE DESIGN SLAB THICKNESS AND THE AVERAGE OF THE THEORETICAL HAUNCH DEPTHS AT MID-SPAN AND AT EACH BEAM BEARING EVEN THROUGH DEVIATION FROM THE DIMENSIONS SHOWN MAY BE NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED GRADE. ONCE ALL BEAM S ARE SET IN THEIR FINAL POSITION, THE ACTUAL CAMBER FOR EACH MEMBER WILL BE THE TOP OF BEAM ELEVATION AT MID-SPAN MINUS THE AVERAGE TOP OF BEAM ELEVATION AT EACH BEARING. THE ACTUAL TOPPING THICKNESS AT MID-SPAN WILL BE THE THEORETICAL DIMENSION PLUS OR MINUS THE DIFFERENCE BETWEEN THE ACTUAL AND ANTICIPATED CAMBER.

LEGEND:

⊙ - BEAM LINE NUMBER

BEAM LINE	BEAMS
1	B1, B5, B9, B13, B17, B21, B25, AND B29
2	B2, B6, B10, B14, B18, B22, B26, AND B30
3	B3, B7, B11, B15, B19, B23, B27, AND B31
4	B4, B8, B12, B16, B20, B24, B28, AND B32



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TOP OF HAUNCH ELEVATIONS													
LOCATION	BEAM LINE 1			BEAM LINE 2			BEAM LINE 3			BEAM LINE 4			
	STATION	ELEV.	OFFSET(FT.)	STATION	ELEV.	OFFSET(FT.)	STATION	ELEV.	OFFSET(FT.)	STATION	ELEV.	OFFSET(FT.)	
SPAN 5	P4 BRG 2	44+05.90	673.12	16.75	LT.	44+05.90	673.28	6.75	LT.	44+05.90	673.33	3.25	RT.
	0.1L	44+16.60	673.22	16.75	LT.	44+16.60	673.39	6.75	LT.	44+16.60	673.44	3.25	RT.
	0.2L	44+28.37	673.33	16.75	LT.	44+28.37	673.50	6.75	LT.	44+28.37	673.56	3.25	RT.
	0.3L	44+40.15	673.44	16.75	LT.	44+40.15	673.61	6.75	LT.	44+40.15	673.67	3.25	RT.
	0.4L	44+51.93	673.53	16.75	LT.	44+51.93	673.71	6.75	LT.	44+51.93	673.77	3.25	RT.
	0.5L	44+63.71	673.62	16.75	LT.	44+63.71	673.80	6.75	LT.	44+63.71	673.85	3.25	RT.
	0.6L	44+75.48	673.69	16.75	LT.	44+75.48	673.87	6.75	LT.	44+75.48	673.93	3.25	RT.
	0.7L	44+87.26	673.76	16.75	LT.	44+87.26	673.93	6.75	LT.	44+87.26	673.99	3.25	RT.
	0.8L	44+99.04	673.81	16.75	LT.	44+99.04	673.98	6.75	LT.	44+99.04	674.04	3.25	RT.
P5 BRG 1	45+10.81	673.86	16.75	LT.	45+10.81	674.03	6.75	LT.	45+10.81	674.08	3.25	RT.	
SPAN 6	P5 BRG 2	45+23.67	673.92	16.75	LT.	45+23.67	674.08	6.75	LT.	45+23.67	674.13	3.25	RT.
	0.1L	45+34.37	674.02	16.75	LT.	45+34.37	674.19	6.75	LT.	45+34.37	674.24	3.25	RT.
	0.2L	45+46.14	674.13	16.75	LT.	45+46.14	674.30	6.75	LT.	45+46.14	674.36	3.25	RT.
	0.3L	45+57.92	674.24	16.75	LT.	45+57.92	674.41	6.75	LT.	45+57.92	674.47	3.25	RT.
	0.4L	45+69.70	674.33	16.75	LT.	45+69.70	674.51	6.75	LT.	45+69.70	674.57	3.25	RT.
	0.5L	45+81.48	674.42	16.75	LT.	45+81.48	674.60	6.75	LT.	45+81.48	674.65	3.25	RT.
	0.6L	45+93.25	674.50	16.75	LT.	45+93.25	674.67	6.75	LT.	45+93.25	674.73	3.25	RT.
	0.7L	46+05.03	674.56	16.75	LT.	46+05.03	674.73	6.75	LT.	46+05.03	674.79	3.25	RT.
	0.8L	46+16.81	674.62	16.75	LT.	46+16.81	674.79	6.75	LT.	46+16.81	674.84	3.25	RT.
P6 BRG 1	46+28.58	674.66	16.75	LT.	46+28.58	674.83	6.75	LT.	46+28.58	674.89	3.25	RT.	
SPAN 7	P6 BRG 2	46+39.28	674.70	16.75	LT.	46+39.28	674.86	6.75	LT.	46+39.28	674.92	3.25	RT.
	0.1L	46+41.44	674.72	16.75	LT.	46+41.44	674.88	6.75	LT.	46+41.44	674.93	3.25	RT.
	0.2L	46+52.14	674.82	16.75	LT.	46+52.14	674.99	6.75	LT.	46+52.14	675.04	3.25	RT.
	0.3L	46+63.92	674.94	16.75	LT.	46+63.92	675.10	6.75	LT.	46+63.92	675.16	3.25	RT.
	0.4L	46+75.69	675.04	16.75	LT.	46+75.69	675.21	6.75	LT.	46+75.69	675.27	3.25	RT.
	0.5L	46+87.47	675.14	16.75	LT.	46+87.47	675.31	6.75	LT.	46+87.47	675.37	3.25	RT.
	0.6L	46+99.25	675.22	16.75	LT.	46+99.25	675.40	6.75	LT.	46+99.25	675.45	3.25	RT.
	0.7L	47+11.02	675.30	16.75	LT.	47+11.02	675.47	6.75	LT.	47+11.02	675.53	3.25	RT.
	0.8L	47+22.80	675.36	16.75	LT.	47+22.80	675.53	6.75	LT.	47+22.80	675.59	3.25	RT.
P7 BRG 1	47+34.58	675.42	16.75	LT.	47+34.58	675.59	6.75	LT.	47+34.58	675.64	3.25	RT.	
SPAN 8	P7 BRG 2	47+46.36	675.47	16.75	LT.	47+46.36	675.63	6.75	LT.	47+46.36	675.69	3.25	RT.
	0.1L	47+57.05	675.50	16.75	LT.	47+57.05	675.66	6.75	LT.	47+57.05	675.72	3.25	RT.
	0.2L	47+59.22	675.50	16.75	LT.	47+59.22	675.66	6.75	LT.	47+59.22	675.71	3.25	RT.
	0.3L	47+69.80	675.60	16.96	LT.	47+69.80	675.77	6.84	LT.	47+69.80	675.82	3.29	RT.
	0.4L	47+81.47	675.71	17.18	LT.	47+81.47	675.89	6.93	LT.	47+81.47	675.94	3.33	RT.
	0.5L	47+93.14	675.81	17.41	LT.	47+93.14	676.00	7.02	LT.	47+93.14	676.06	3.37	RT.
	0.6L	48+04.81	675.91	17.63	LT.	48+04.81	676.10	7.11	LT.	48+04.81	676.16	3.42	RT.
	0.7L	48+16.48	675.99	17.86	LT.	48+16.48	676.19	7.20	LT.	48+16.48	676.25	3.46	RT.
	0.8L	48+28.14	676.06	18.08	LT.	48+28.14	676.26	7.29	LT.	48+28.14	676.32	3.51	RT.
CL FA BRG	48+39.81	676.12	18.31	LT.	48+39.81	676.32	7.38	LT.	48+39.81	676.38	3.55	RT.	
	48+51.48	676.17	18.53	LT.	48+51.48	676.37	7.47	LT.	48+51.48	676.43	3.59	RT.	
	48+63.15	676.21	18.76	LT.	48+63.15	676.40	7.56	LT.	48+63.15	676.46	3.64	RT.	
	48+74.82	676.25	18.98	LT.	48+74.82	676.43	7.65	LT.	48+74.82	676.49	3.68	RT.	

TOP OF HAUNCH ELEVATIONS

TOP OF HAUNCH ELEVATIONS SHOWN REPRESENT THE THEORETICAL LOCATION OF THE BOTTOM OF THE DECK ABOVE THE BEAM/GIRDER HAUNCH PRIOR TO DEFLECTIONS CAUSED BY DECK PLACEMENT AND OTHER ANTICIPATED DEAD LOADS.

NOTES:

1. FOR DECK KEY PLAN, SEE SHEET 41/64
2. FOR SCREED ELEVATIONS, SEE SHEET 42/64
3. FOR FINAL DECK ELEVATIONS, SEE SHEETS 45-46/64
4. FOR HAUNCH THICKNESS, SEE SHEETS 47-48/64



DATE: 05/2015
REVIEWED TLR
STRUCTURE FILE NUMBER: TBD

DRAWN: KRH
CHECKED: KRH
REVISED: ...

DESIGNED: KRH
CHECKED: SCT

TOP OF HAUNCH ELEVATION (2 OF 2)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

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FINAL DECK ELEVATIONS

Table with columns: LOCATION, LEFT EDGE OF DECK, BEAM LINE 1, LEFT TOE OF CURB, BEAM LINE 2, CL CONST. IND DR & PG, BEAM LINE 3, BEAM LINE 4, RIGHT TOE OF BARRIER, RIGHT EDGE OF DECK. Rows include SPAN 1, SPAN 2, SPAN 3, SPAN 4 and various BRG and PIER locations.

FINAL DECK SURFACE ELEVATIONS

FINAL DECK SURFACE ELEVATIONS SHOWN REPRESENT THE DECK SURFACE LOCATION AFTER ALL ANTICIPATED DEAD LOAD DEFLECTIONS HAVE OCCURRED.

NOTES:

- 1. FOR DECK KEY PLAN, SEE SHEET 41/64
2. FOR SCREED ELEVATIONS, SEE SHEET 42/64
3. FOR TOP OF HAUNCH ELEVATIONS, SEE SHEETS 43-44/64
4. FOR HAUNCH THICKNESS, SEE SHEETS 47-48/64

Vertical sidebar containing logos (Mannik Smith GROUP), project title (HEN-IND-00.00), PID No. 22984, and drawing details (DESIGNED, CHECKED, DRAWN, REVIEWED).

FINAL DECK ELEVATIONS

Table with columns: LOCATION, LEFT EDGE OF DECK, BEAM LINE 1, LEFT TOE OF CURB, BEAM LINE 2, CL CONST. IND DR & PG, BEAM LINE 3, BEAM LINE 4, RIGHT TOE OF BARRIER, RIGHT EDGE OF DECK. Rows include PIER 4, SPAN 5 (P4 BRG 2, 0.1L-0.9L), PIER 5, SPAN 6 (P5 BRG 2, 0.1L-0.9L), PIER 6, SPAN 7 (P6 BRG 2, 0.1L-0.9L), PIER 7, SPAN 8 (P7 BRG 2, 0.1L-0.9L), and FA BRG.

NOTES:

- 1. FOR DECK KEY PLAN, SEE SHEET 41/64
2. FOR SCREED ELEVATIONS, SEE SHEET 42/64
3. FOR TOP OF HAUNCH ELEVATIONS, SEE SHEETS 43-44/64
4. FOR HAUNCH THICKNESS, SEE SHEETS 47-48/64

FINAL DECK SURFACE ELEVATIONS

FINAL DECK SURFACE ELEVATIONS SHOWN REPRESENT THE DECK SURFACE LOCATION AFTER ALL ANTICIPATED DEAD LOAD DEFLECTIONS HAVE OCCURRED.



DATE: 05/2015
REVIEWED: TLR
STRUCTURE FILE NUMBER: TBD

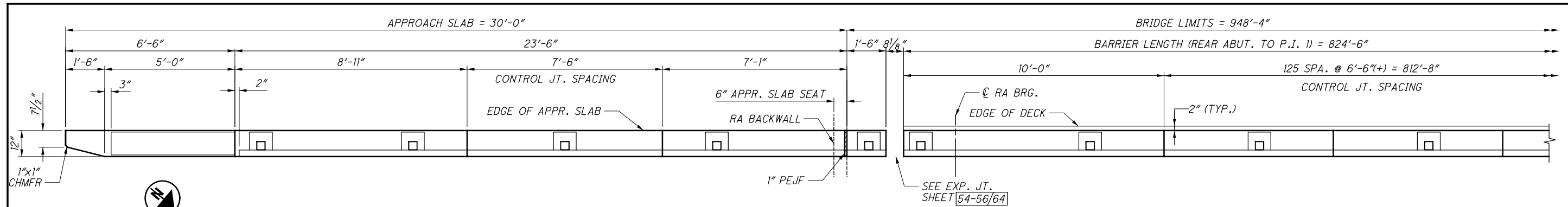
DRAWN: KRH
CHECKED: SCT
DESIGNED: KRH

FINAL DECK ELEVATION (2 OF 2)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

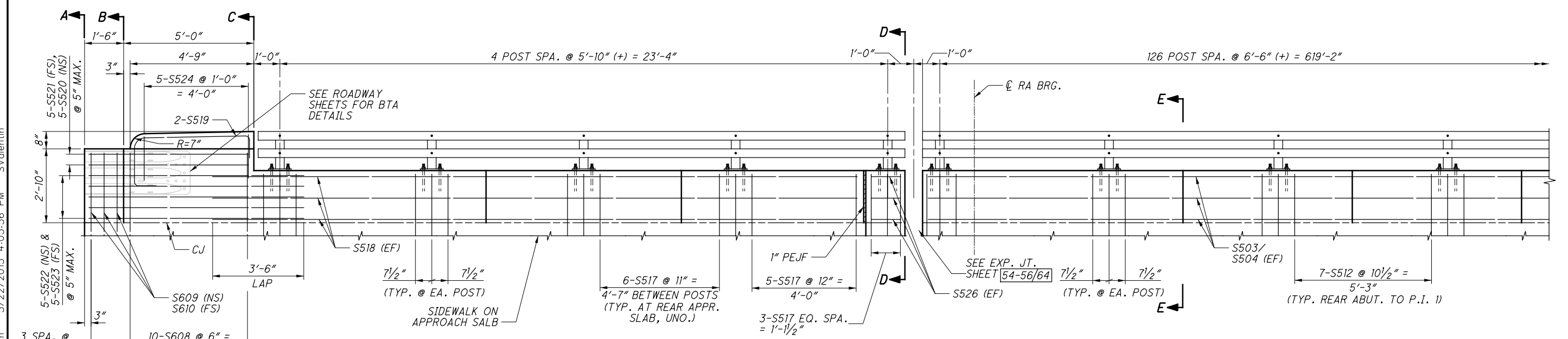
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PID No. 22984

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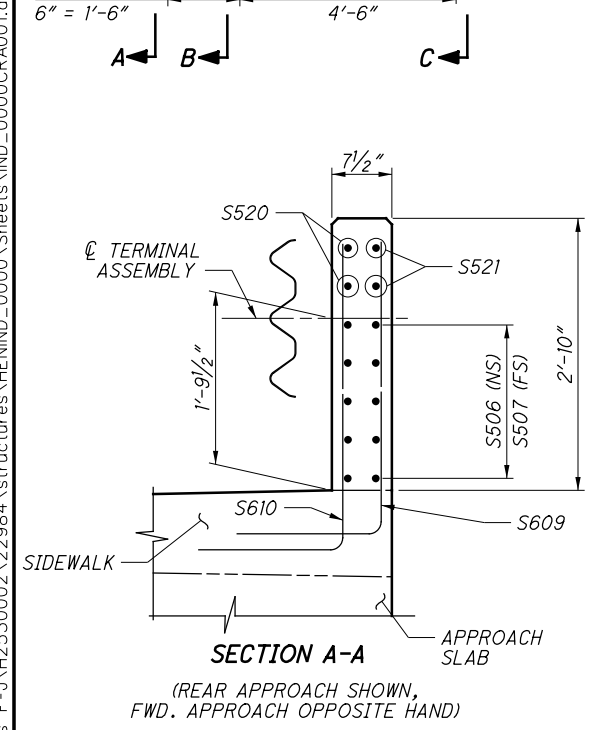
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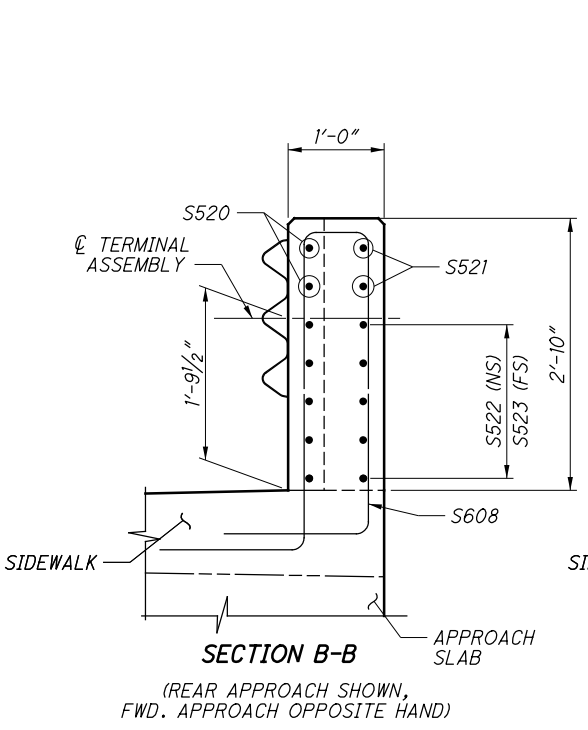
SIDEWALK BARRIER PLAN
(LEFT SIDE - TYPE BR-2-98)



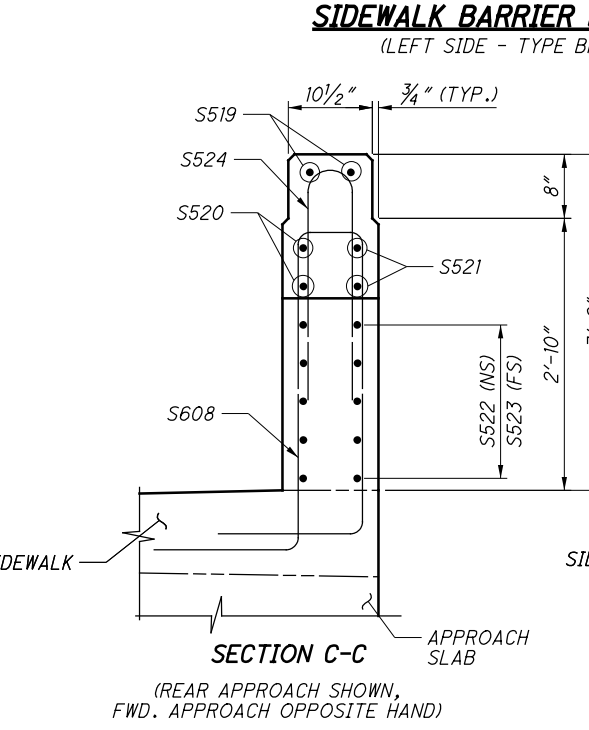
SIDEWALK BARRIER ELEVATION
(LEFT SIDE - TYPE BR-2-98)



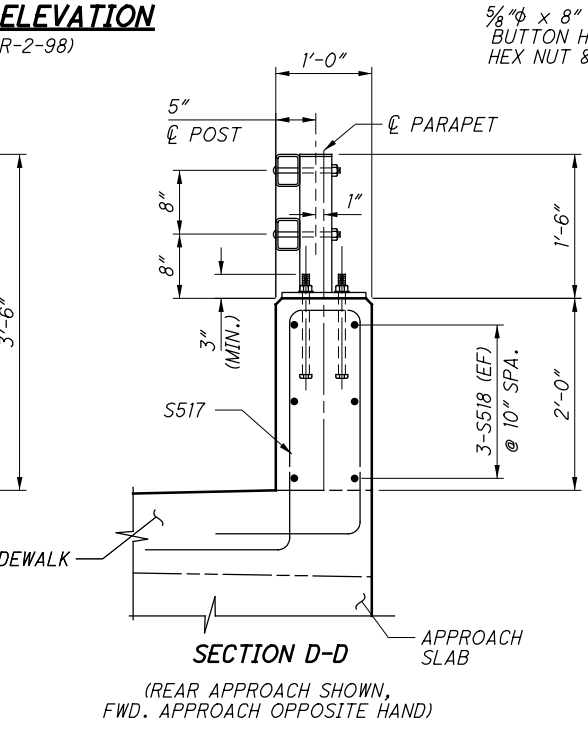
SECTION A-A
(REAR APPROACH SHOWN,
FWD. APPROACH OPPOSITE HAND)



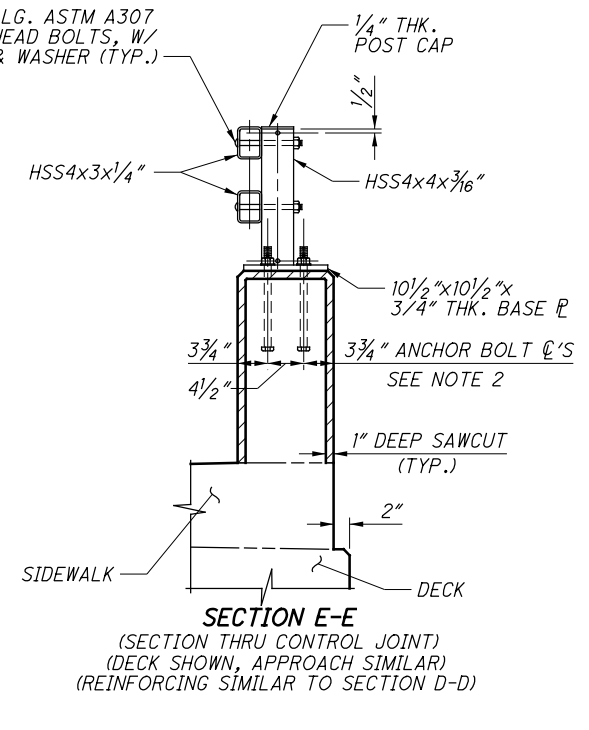
SECTION B-B
(REAR APPROACH SHOWN,
FWD. APPROACH OPPOSITE HAND)



SECTION C-C
(REAR APPROACH SHOWN,
FWD. APPROACH OPPOSITE HAND)



SECTION D-D
(REAR APPROACH SHOWN,
FWD. APPROACH OPPOSITE HAND)

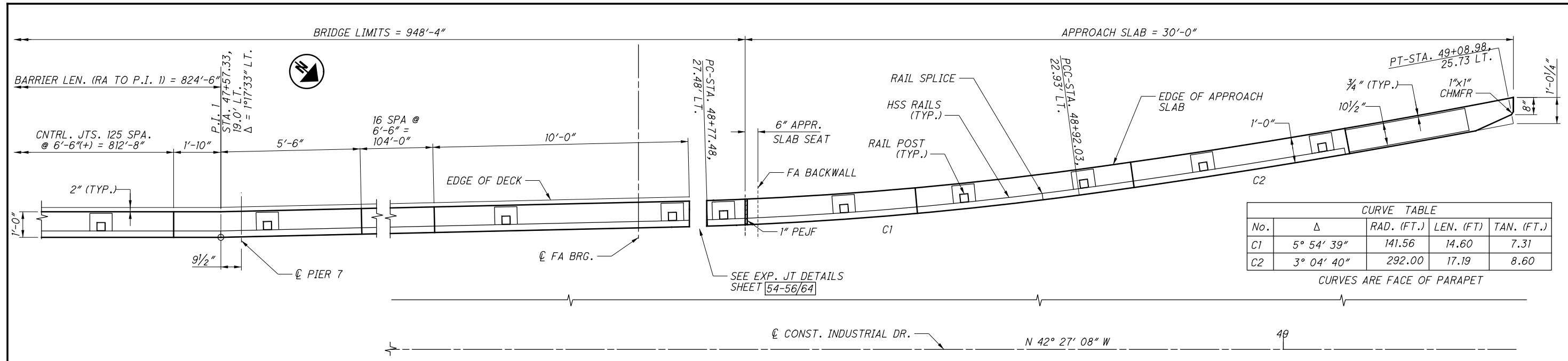


SECTION E-E
(SECTION THRU CONTROL JOINT)
(DECK SHOWN, APPROACH SIMILAR)
(REINFORCING SIMILAR TO SECTION D-D)

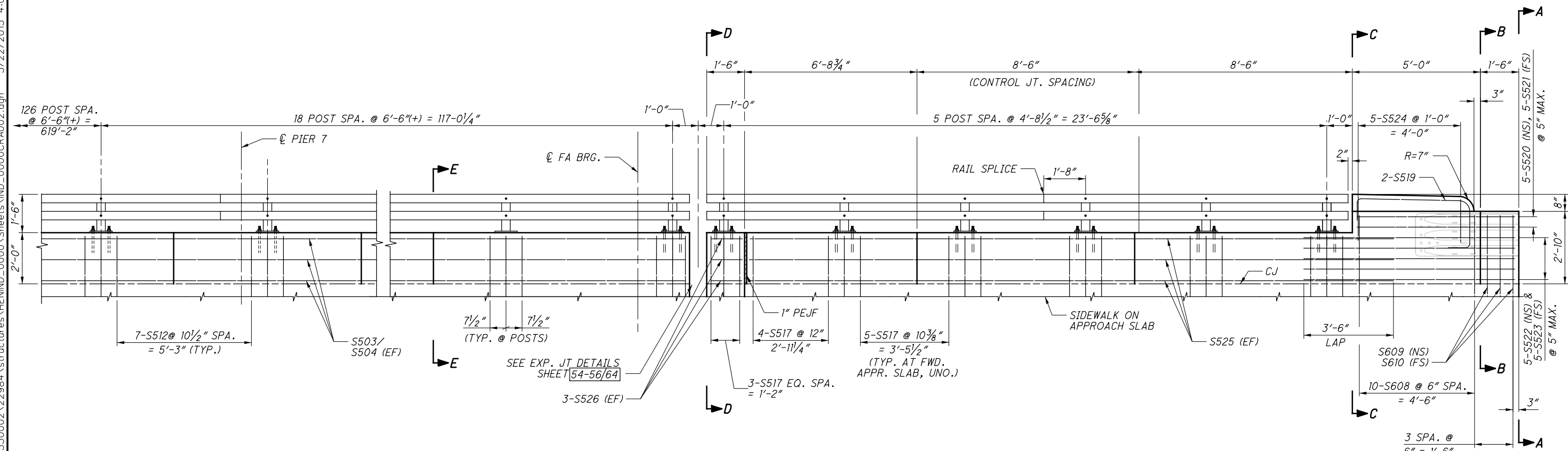
- NOTES:**
- CONTRACTOR HAS THE OPTION TO ADJUST RAILING SPLICE LOCATIONS PROVIDED ALL CRITERIA ON ODOT STD. DWG. BR-2-98 WILL BE MET.
 - HORIZONTAL RAILING ELEMENTS SHALL BE PLACED FLUSH WITH THE FACE OF THE PARAPET WALL.
 - ANCHOR BOLTS SHALL BE ASTM A449 7/8" x 1'-1" LG. WITH HEX NUT AND WASHER, AND MIN. 10" EMBEDMENT IN THE CONCRETE PARAPET.
 - FOR ADDITIONAL INFORMATION, SEE ODOT STD. DWG. BR-2-98.

LEFT RAILING DETAILS (1 OF 2)
 HEN-IND-00.00
 PID No. 22984
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER
 MAUMEE, OHIO 43537
 MANNIK SMITH GROUP
 DATE: 4/15
 REVIEWED: TLR
 DRAWN: ANK
 DESIGNED: AMK
 CHECKED: SCT
 STRUCTURE FILE NUMBER: TBD
 49/64
 133
 180

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SIDEWALK BARRIER PLAN
(LEFT SIDE - TYPE BR-2-98)



SIDEWALK BARRIER ELEVATION
(LEFT SIDE - TYPE BR-2-98)

- NOTES:**
- FOR SECTIONS A-A THRU E-E, SEE SHEET 49/64.
 - FOR ADDITIONAL NOTES, SEE SHEET 49/64.

1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

Mannik Smith GROUP

DATE: 4/15
REVIEWED: TLR
DRAWN: AMK
DESIGNED: AMK
CHECKED: SCT

STRUCTURE FILE NUMBER: TBD

LEFT RAILING DETAILS (2 OF 2)

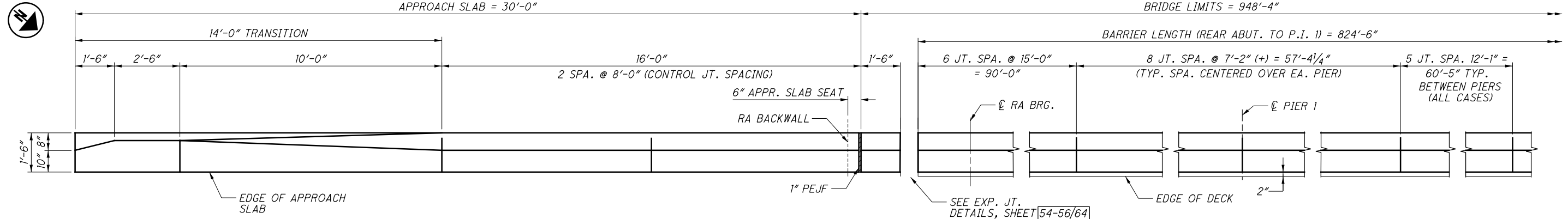
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00

PID No. 22984

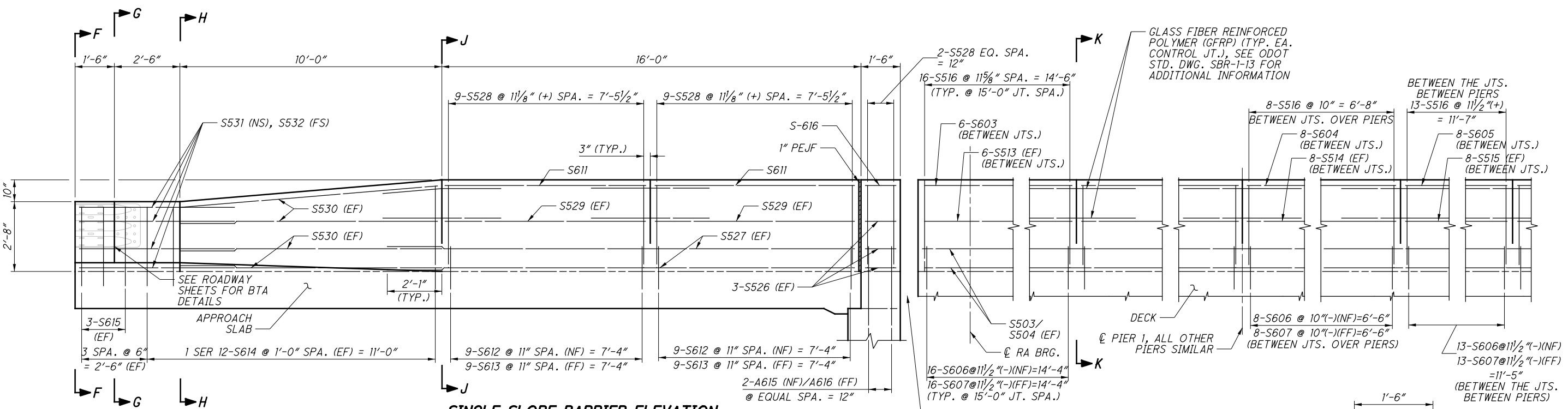
50/64

134
180

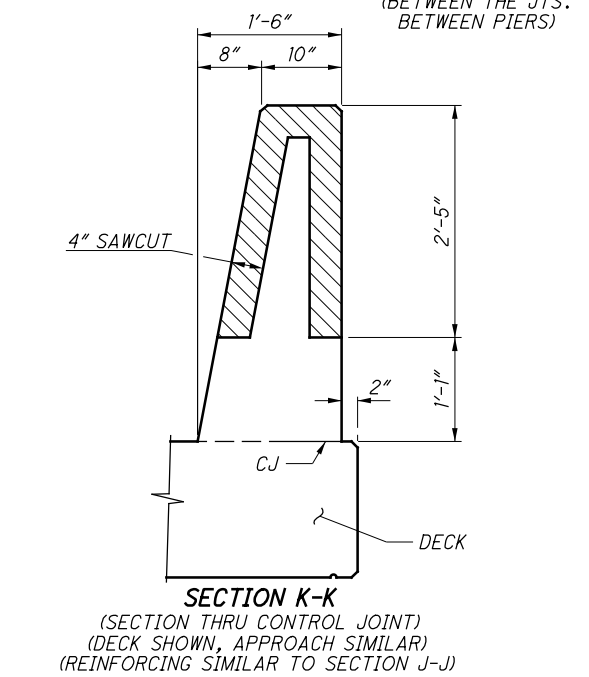
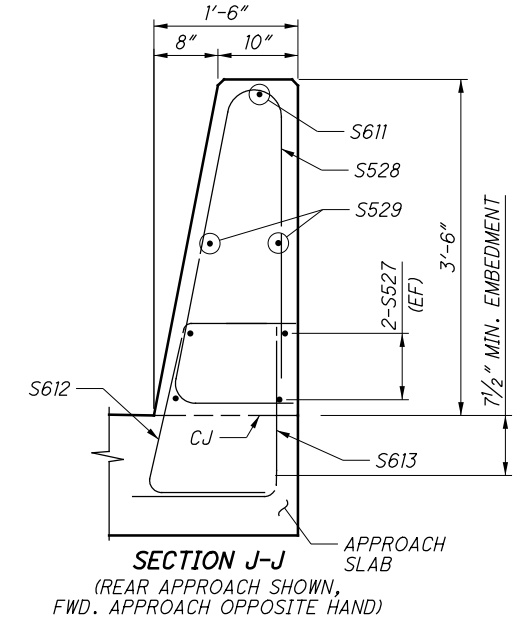
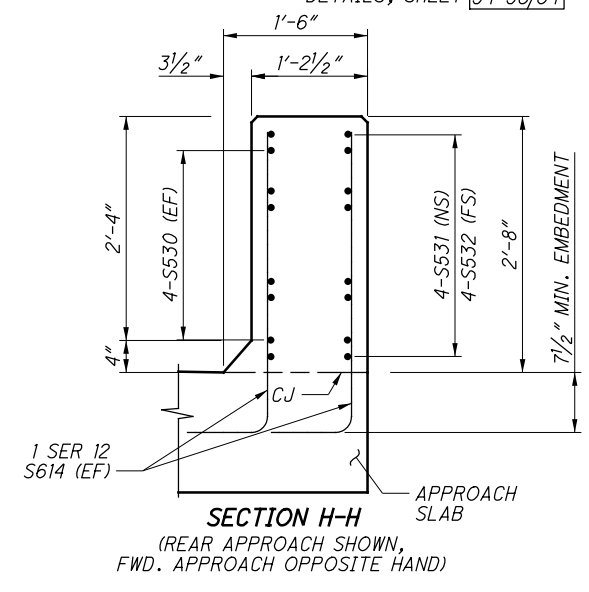
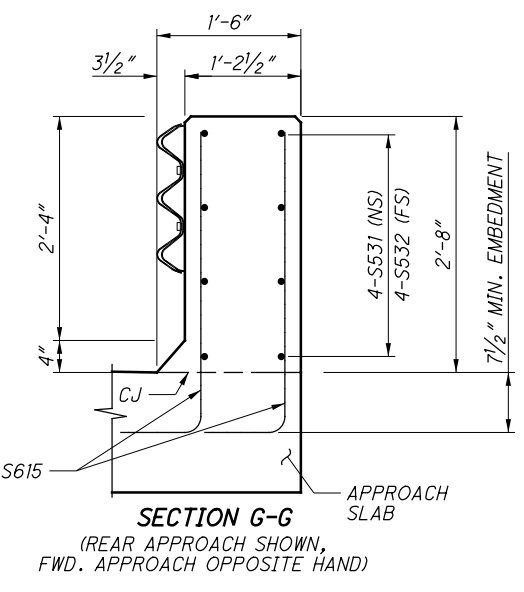
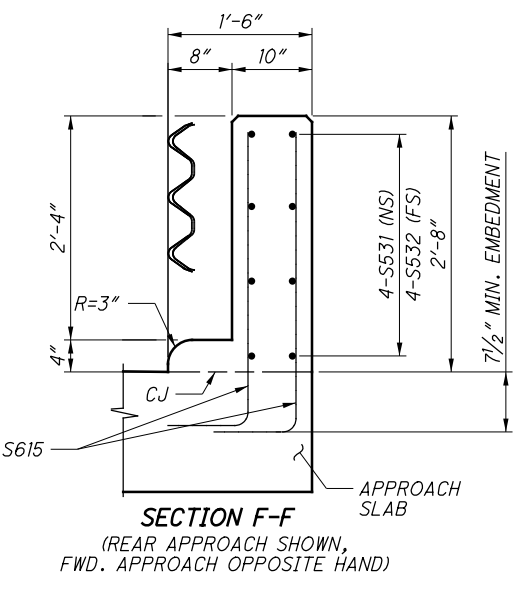


NOTES:
 1. FOR ADDITIONAL INFORMATION, SEE ODOT STD. DWG. SBR-1-13.
 2. FOR RAILING DETAIL PLAN NOTES, SEE SHEET 52/64.

SINGLE SLOPE BARRIER PLAN
 (RIGHT SIDE - TYPE SBR-1-13)



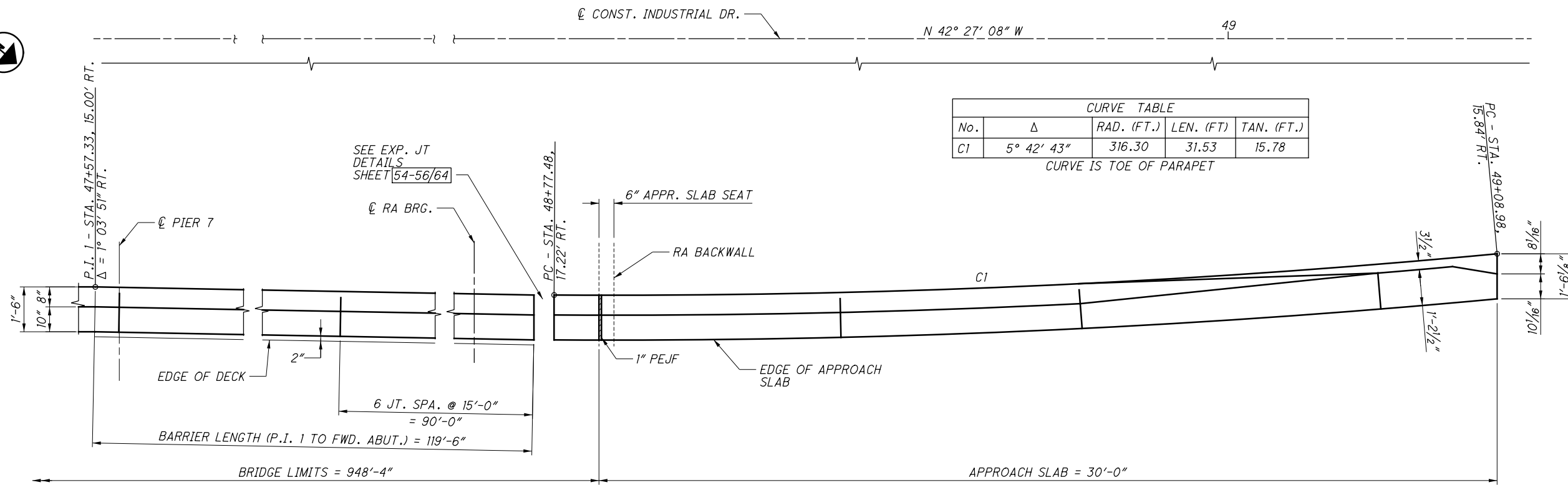
SINGLE SLOPE BARRIER ELEVATION
 (RIGHT SIDE - TYPE SBR-1-13)



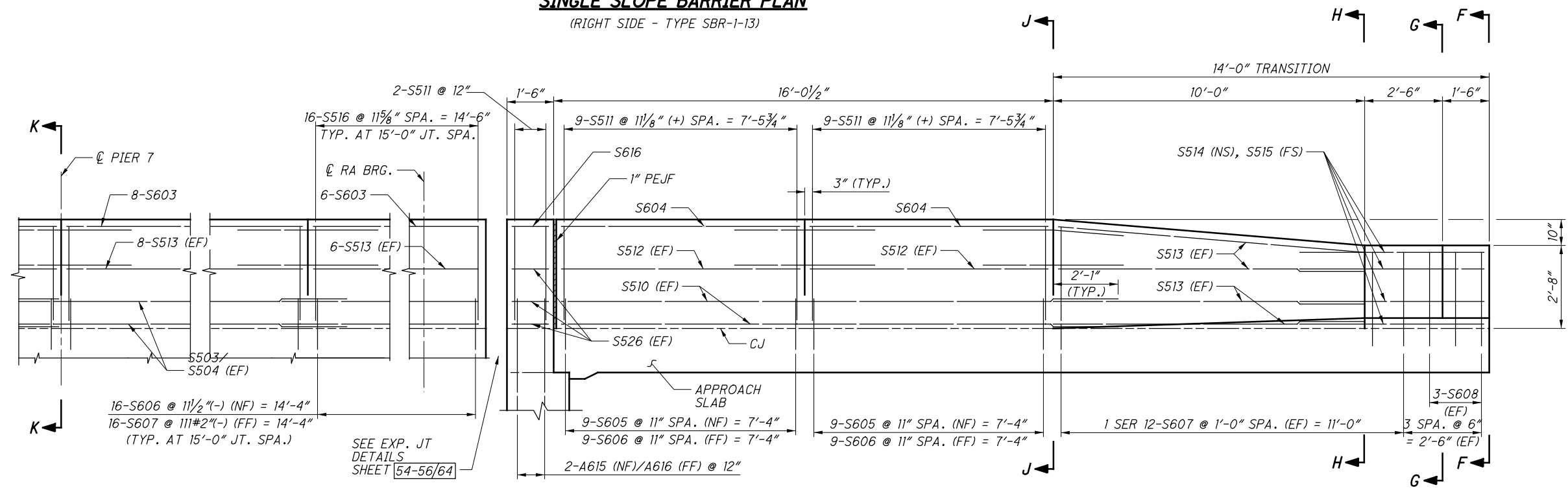
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 1800 INDIAN WOOD CIRCLE
 MAUMEE, OHIO 43537
 DATE: 4/15
 REVIEWED: TLR
 DRAWN: AMK
 DESIGNED: AMK
 CHECKED: SCT
 STRUCTURE FILE NUMBER: TBD
RIGHT RAILING DETAILS (1 OF 2)
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER
HEN-IND-00.00
 PID No. 22984
 51/64
 135
 180

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SINGLE SLOPE BARRIER PLAN
(RIGHT SIDE - TYPE SBR-1-13)



SINGLE SLOPE BARRIER ELEVATION
(RIGHT SIDE - TYPE SBR-1-13)

- NOTES:**
1. FOR ADDITIONAL APPROACH SLAB INFORMATION, SEE ODOT STD. DWGS. AS-1-15 & AS -2-15.
 2. FOR ADDITIONAL SINGLE SLOPE RAILING INFORMATION, SEE ODOT STD. DWGS. SBR-1-13.
 3. FOR DECK PLAN, SEE SHEETS 37-38/64.
 4. FOR ABUTMENT DETAILS, SEE SHEETS 11-20/64.
 5. FOR APPROACH SLAB DETAILS, SEE SHEETS 57-58/64.
 6. FOR SECTIONS F-F THRU K-K, SEE SHEET 51/64.



DESIGNED	AMK	CHECKED	SCT
DRAWN	AMK	REVISED	
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD
DATE	4/15		

RIGHT RAILING DETAILS (2 OF 2)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

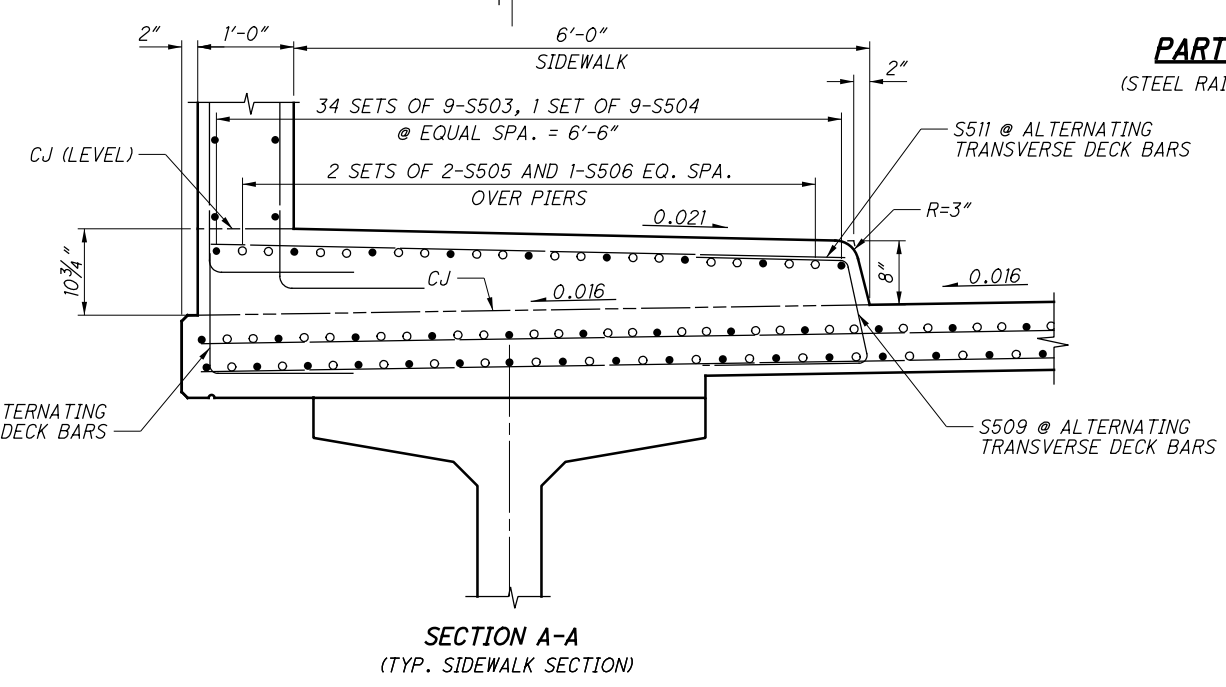
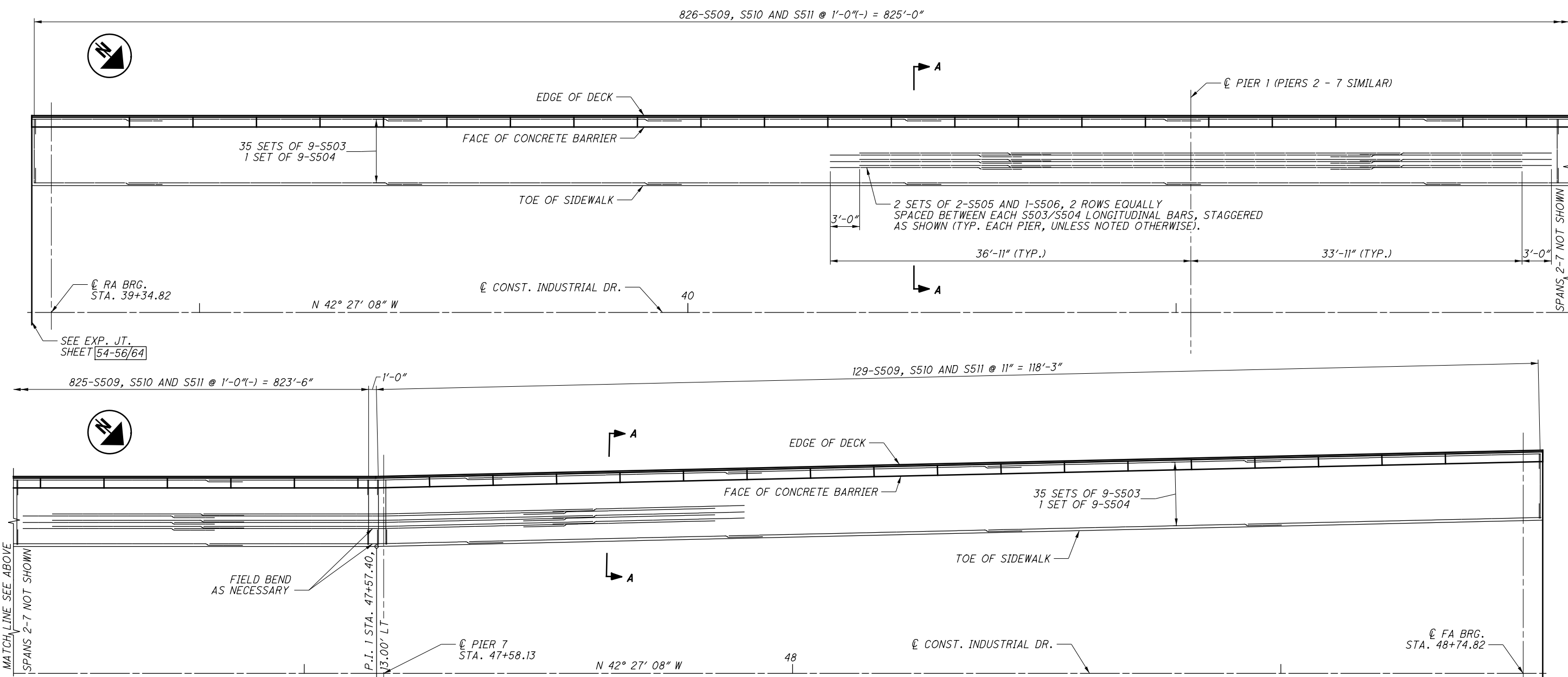
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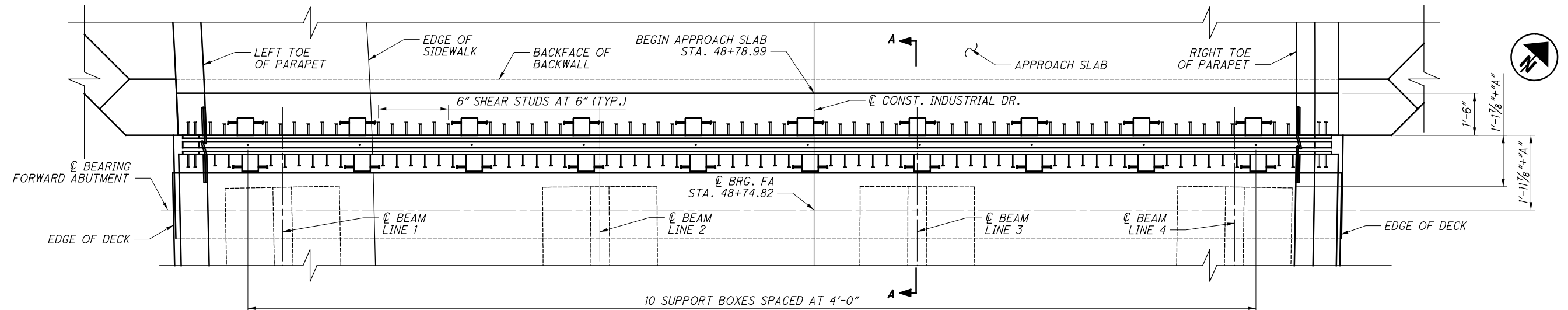
DESIGNED	DATE
KRH	05/2015
SCT	TLR
SCT	STRUCTURE FILE NUMBER
	TBD

SIDEWALK PLAN AND SECTION
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

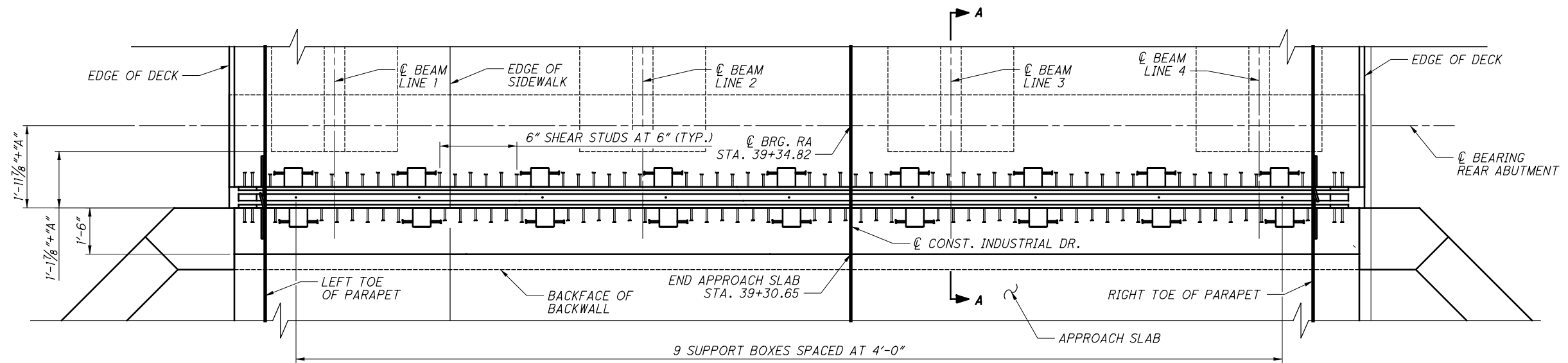
HEN-IND-00.00
 PID No. 22984
 53/64
 137
 180



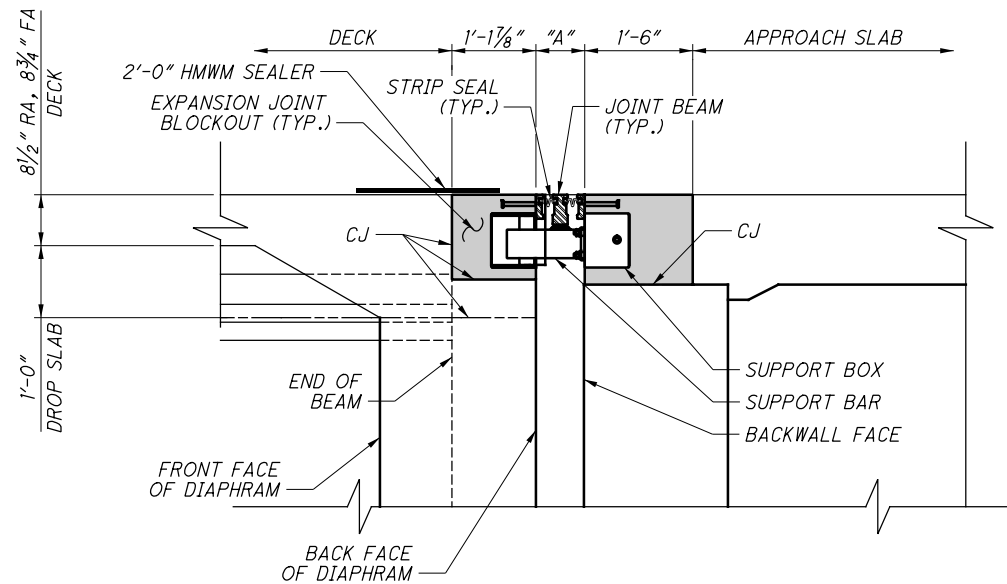
- NOTES:**
1. SPANS 2 - 7 NOT SHOWN.
 2. FOR SIDEWALK RAILING DETAILS, SEE SHEETS 49-50/64.
 3. FOR TRANSVERSE SECTION INCLUDING CONCRETE SEALING INFORMATION, SEE SHEETS 39-40/64.
 4. FOR DECK PLAN, SEE SHEETS 37-38/64.
 5. FOR EXPANSION JOINT DETAILS, SEE SHEETS 54-56/64.
 6. FOR APPROACH SLAB DETAILS, SEE SHEETS 57-58/64.



FORWARD ABUTMENT EXPANSION JOINT DETAIL



REAR ABUTMENT EXPANSION JOINT DETAIL



SECTION A-A

TEMP	DIMENSION "A" (INCHES)	
	REAR ABUT	FWD ABUT
15°F	9 13/16	9 13/16
20°F	9 5/8	9 5/8
30°F	9 1/4	9 1/4
40°F	8 7/8	8 7/8
50°F	8 1/2	8 1/2
60°F	8 1/8	8 1/8
70°F	7 3/4	7 3/4
80°F	7 3/8	7 3/8
90°F	7	7
95°F	6 13/16	6 13/16

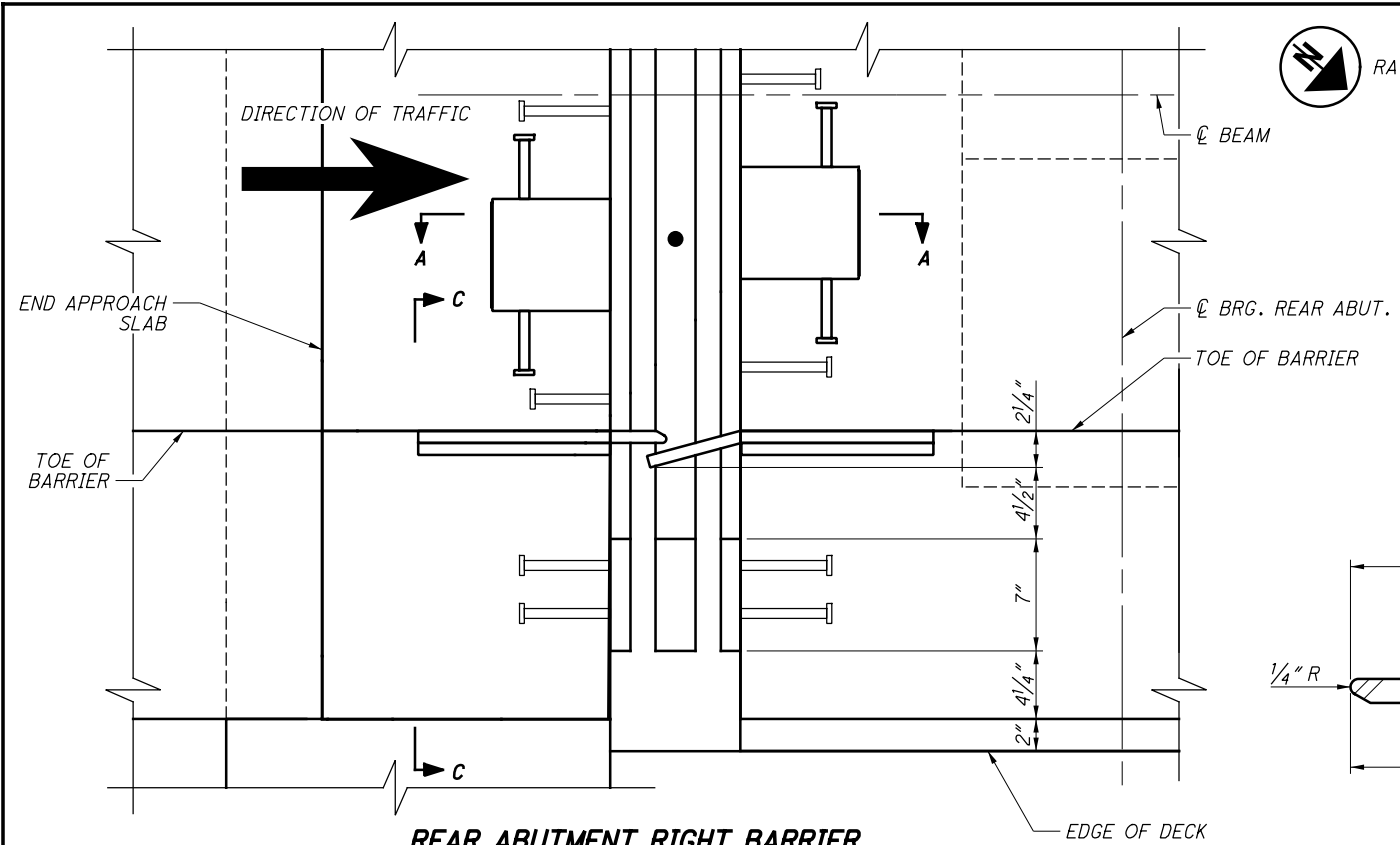
NOTES:

- SEAL TRANSVERSE DECK CONSTRUCTION JOINT WITH 2'-0" HMWM, CENTERED ABOUT JOINT.
- THE MODULAR JOINT SPECIFIED SHALL BE A D.S. BROWN "D160-PV-S" STEELFLEX MODULAR EXPANSION JOINT SYSTEM OR EQUIVALENT. IF AN ALTERNATE JOINT IS USED, THE PLANS SHALL BE MODIFIED TO ACCOMMODATE THE NEW JOINT SYSTEM.
- FOR DECK PLAN, SEE SHEETS 37-38/64
- FOR END DIAPHRAM DETAILS, SEE SHEET 35/64
- DROP SLAB SHALL EXTEND THE ENTIRE WIDTH OF DECK.
- CONCRETE PARAPETS TO BE INSTALLED AFTER INSTALLATION OF MODULAR EXPANSION JOINT.

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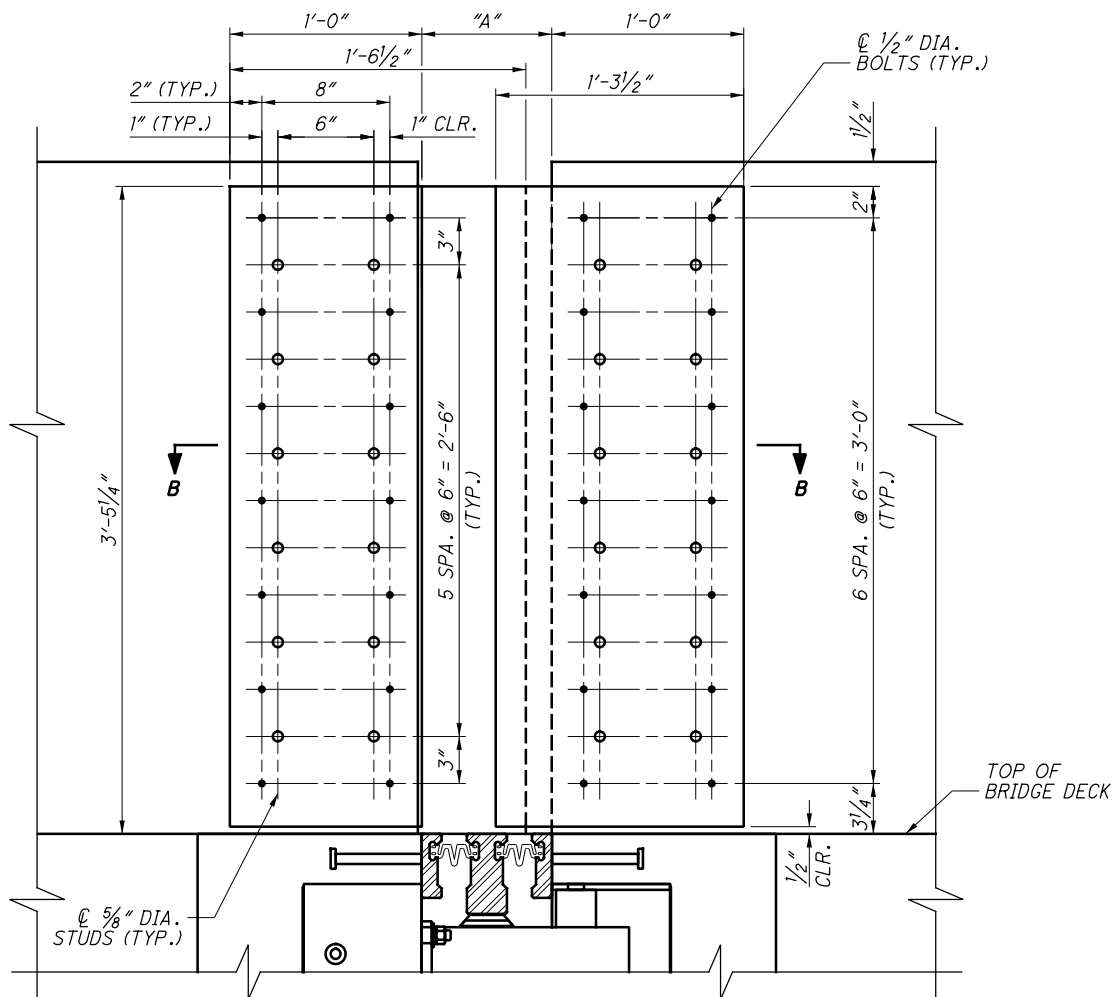

 1800 INDIAN WOOD CIRCLE
 MAUMEE, OHIO 43537
 DATE: 05/2015
 TLR: TLR
 STRUCTURE FILE NUMBER: TBD
 DRAWN: JEC
 CHECKED: SCT
 DESIGNED: CWE
 MODULAR EXPANSION JOINT DETAILS (1 OF 3)
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER
 HEN-IND-00.00
 PID No. 22984
 54/64
 138
 180

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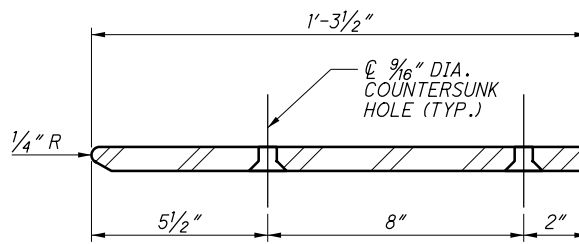
**REAR ABUTMENT RIGHT BARRIER
PART PLAN**

(FORWARD ABUTMENT OPPOSITE HAND)

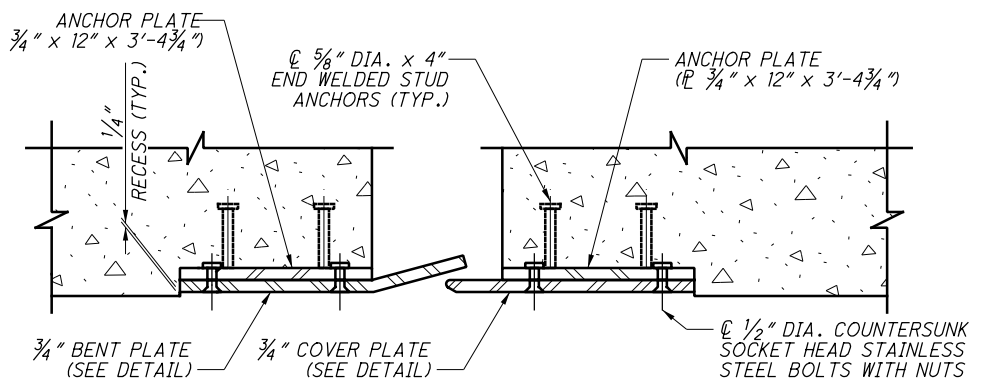


SECTION A-A

(DIMENSIONS MEASURED ARE ALONG THE FACE OF THE PARAPET)

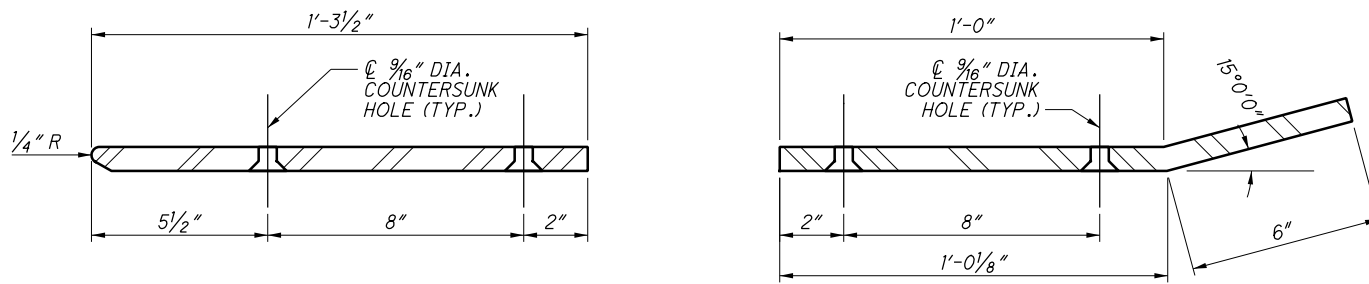


3/4" COVER PLATE

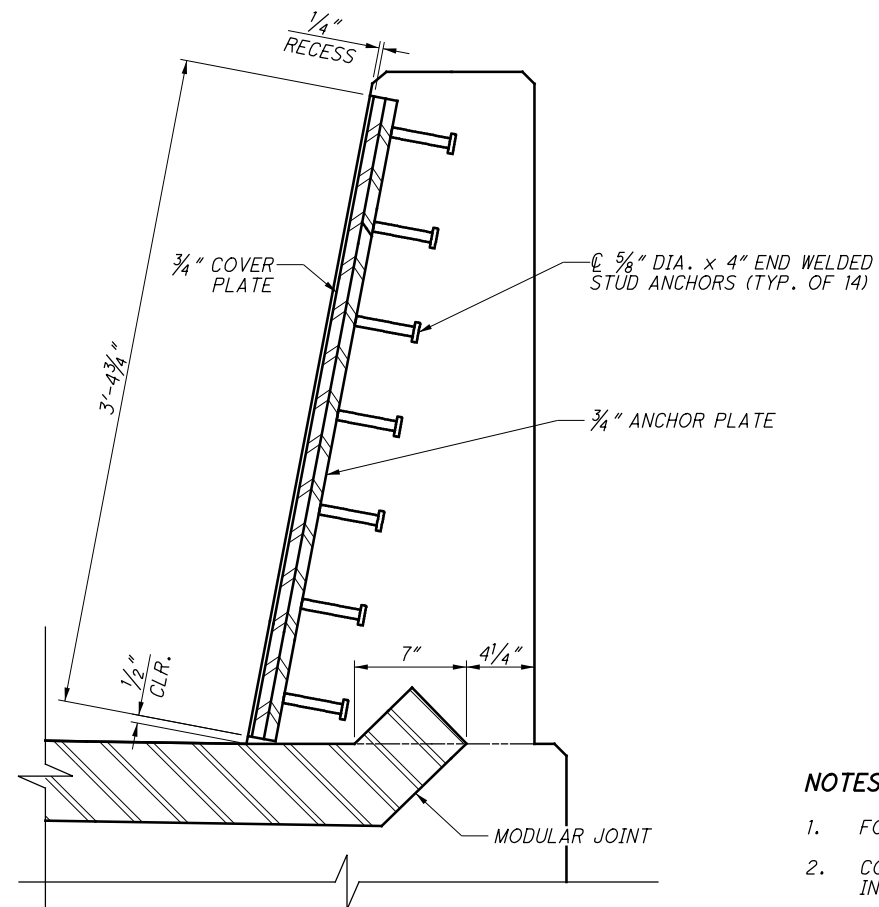


SECTION B-B

(REAR ABUTMENT RIGHT BARRIER SHOWN)
(ALL OTHER PLATES SIMILAR)



3/4" BENT PLATE

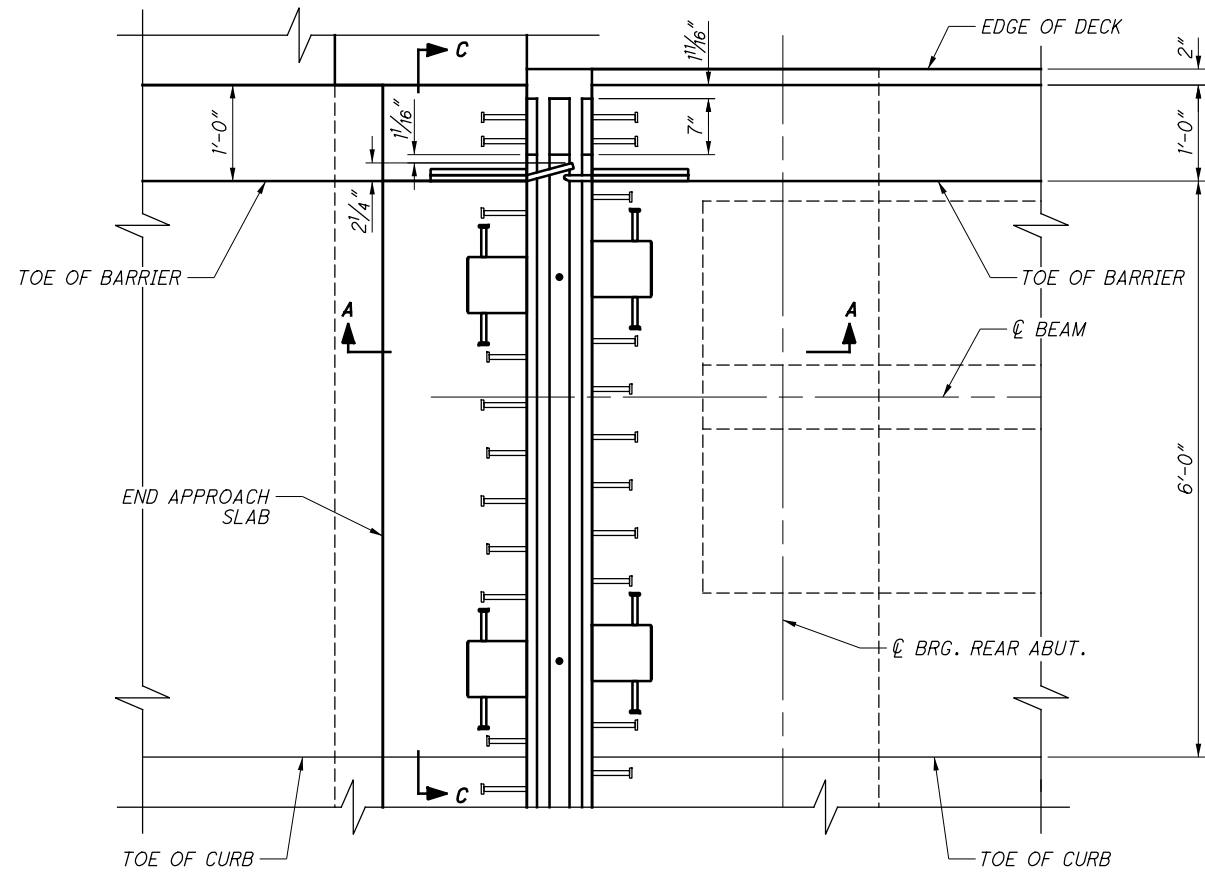


SECTION C-C

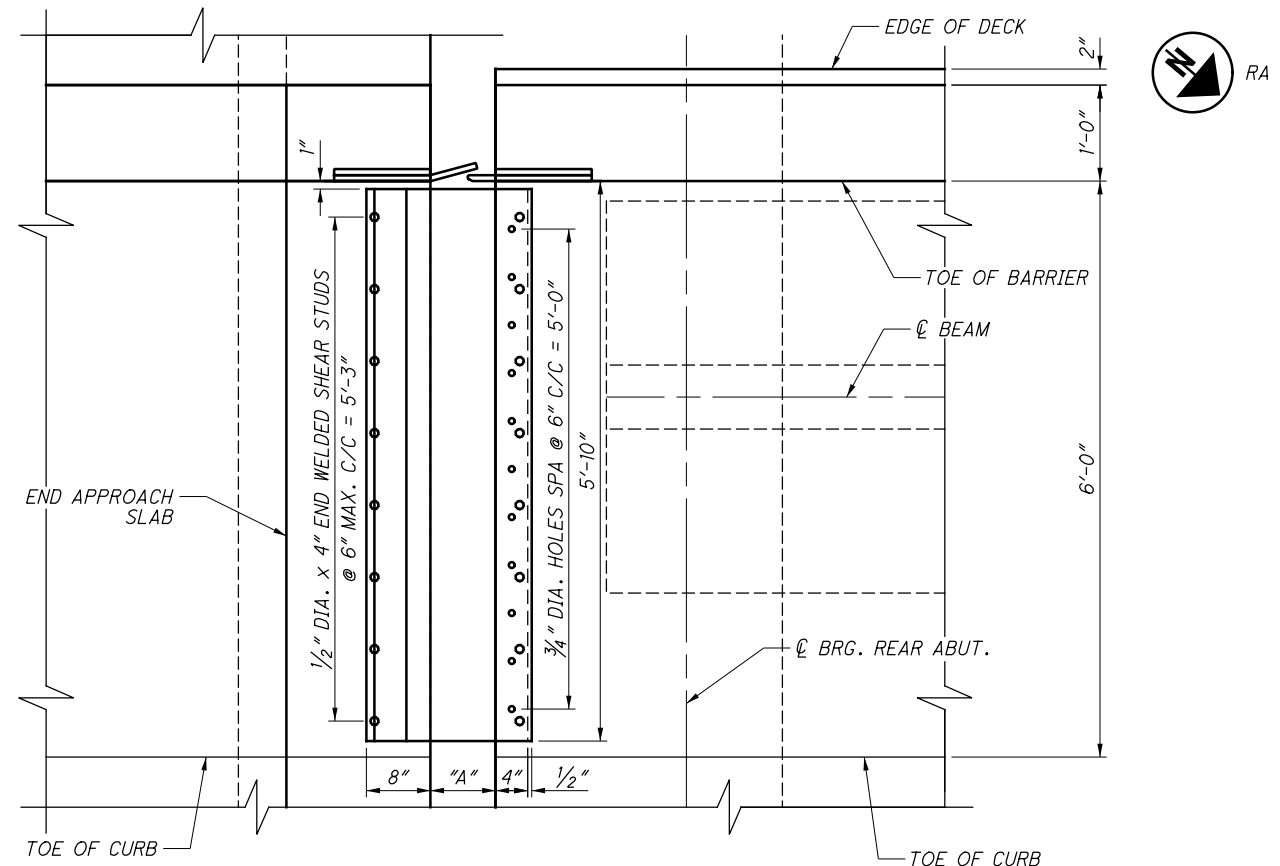
NOTES:

1. FOR DIMENSIONS "A", SEE SHEET 54/64
2. CONCRETE PARAPETS TO BE INSTALLED AFTER INSTALLATION OF MODULAR EXPANSION JOINTS.
3. FOR ADDITIONAL NOTES SEE SHEET 54/64

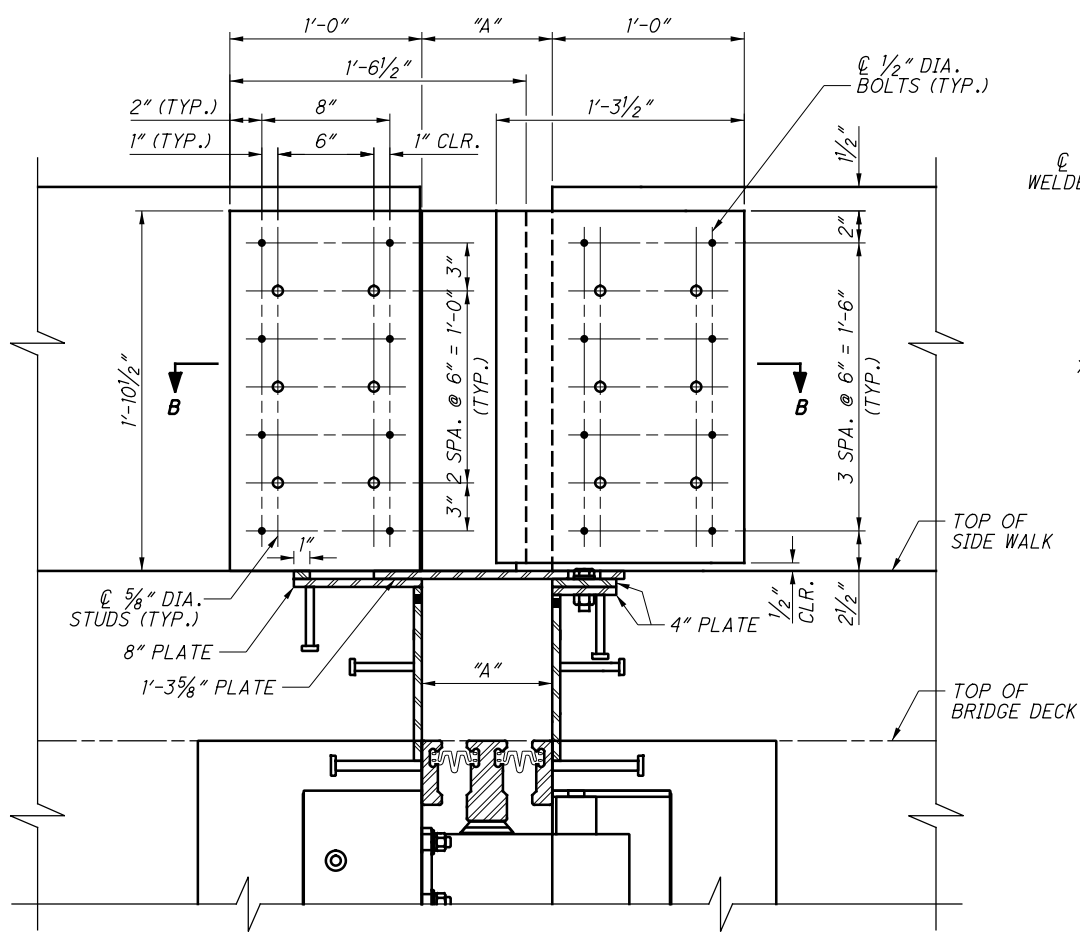
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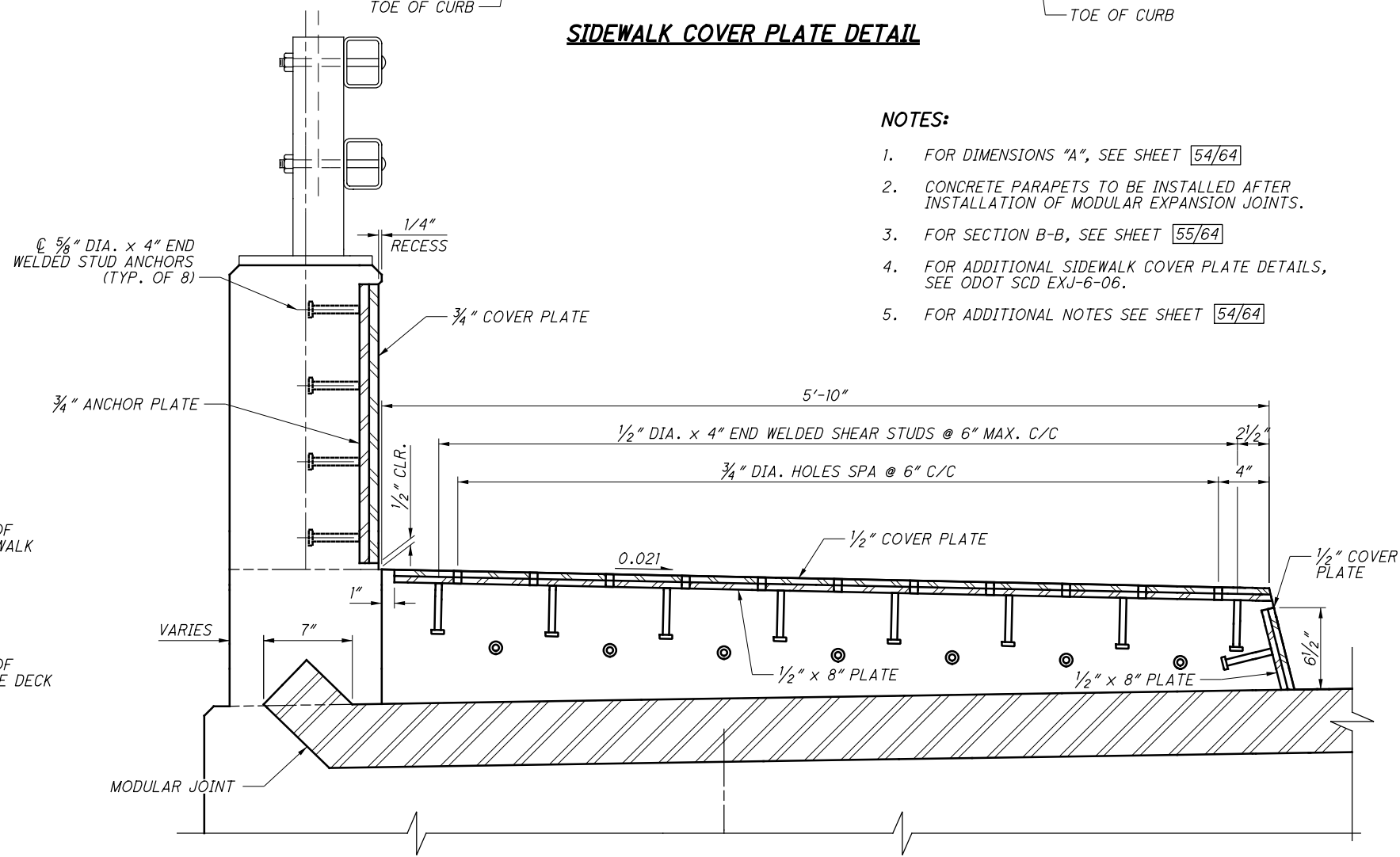
**REAR ABUTMENT LEFT BARRIER
PART PLAN**
(FORWARD ABUTMENT OPPOSITE HAND)



SIDEWALK COVER PLATE DETAIL



SECTION A-A



SECTION C-C

NOTES:

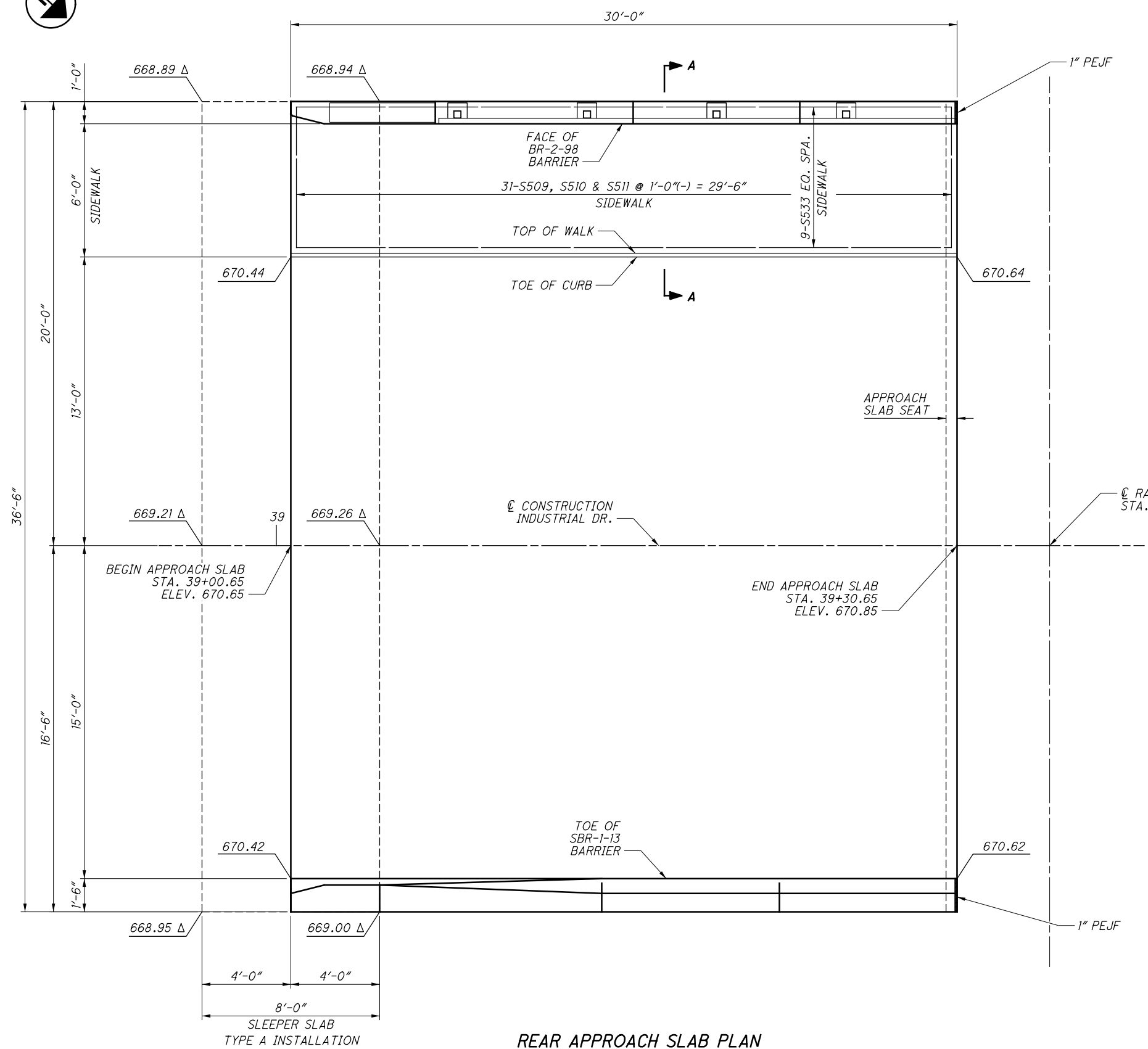
1. FOR DIMENSIONS "A", SEE SHEET 54/64
2. CONCRETE PARAPETS TO BE INSTALLED AFTER INSTALLATION OF MODULAR EXPANSION JOINTS.
3. FOR SECTION B-B, SEE SHEET 55/64
4. FOR ADDITIONAL SIDEWALK COVER PLATE DETAILS, SEE ODOT SCD EXJ-6-06.
5. FOR ADDITIONAL NOTES SEE SHEET 54/64

MODULAR EXPANSION JOINT DETAILS (3 OF 3)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

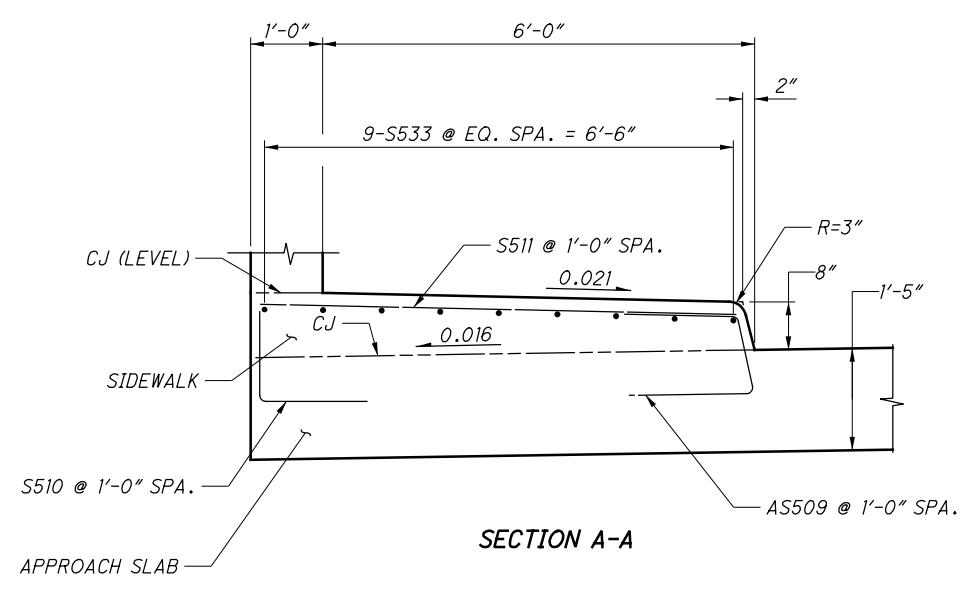
HEN-IND-00.00
PID No. 22984

56/64

140
180



REAR APPROACH SLAB PLAN



SECTION A-A

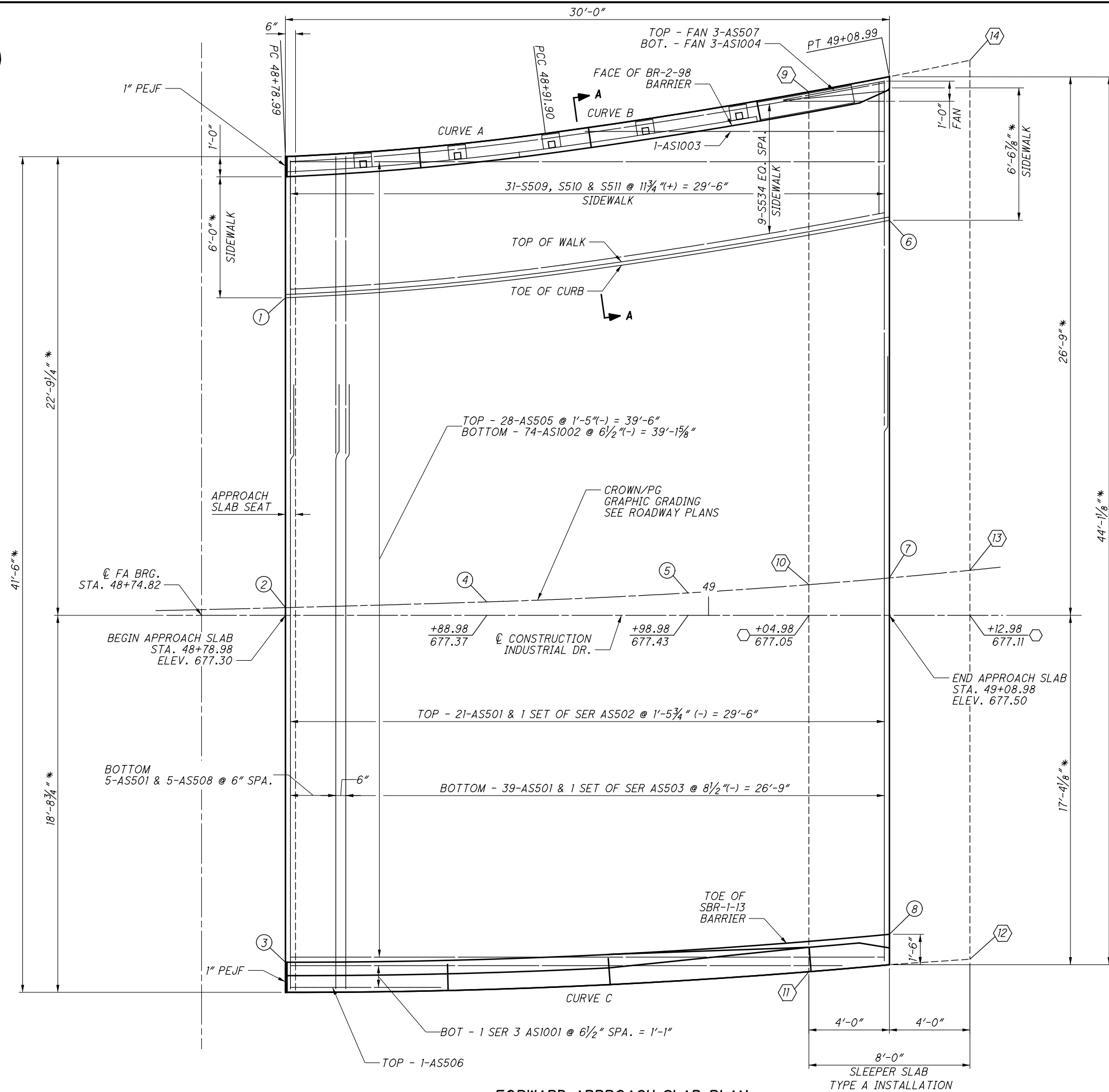
LEGEND:
 Δ - SLEEPER SLAB ELEVATIONS

- NOTES:**
1. HMWM SEALER INCLUDED WITH ITEM 512 SEALING OF CONCRETE BRIDGE DECKS WITH HMWM RESIN AS PER PLAN FOR PAYMENT.
 2. SEE STD. DWG. AS-1-15 AND AS-2-15 FOR ADDITIONAL DETAILS INCLUDING APPROACH SLAB REINFORCING STEEL.
 3. FOR SIDEWALK RAILING DETAILS, SEE SHEETS 49-50/64.
 4. FOR FORWARD APPROACH SLAB DETAILS, SEE SHEET 58/64.
 5. THE CONCRETE AND REINFORCING STEEL FOR THE SIDEWALK PARAPET IS INCLUDED WITH ITEM 517, RAILING MISC.: CONCRETE PARAPET WITH STEEL RAILING.

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REAR APPROACH SLAB DETAILS HEN-INDUSTRIAL DRIVE-0000 INDUSTRIAL DRIVE OVER MAUMEE RIVER	DATE: 05/2015 REVIEWED: TLR DRAWN: AMK DESIGNED: AMK CHECKED: SCT STRUCTURE FILE NUMBER: TBD REVISIONS:
HEN-IND-00.00 PID No. 22984	57/64

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APPROACH SLAB SURFACE			
No.	STA.	OFFSET	ELEV.
1	48+78.98	15.77 LT	677.07
2	48+78.98	0.38 LT	677.31
3	48+78.98	17.22 RT	677.03
4	48+88.98	0.67 LT	677.38
5	48+98.98	1.11 LT	677.45
6	49+08.98	19.62 LT	677.27
7	49+08.98	1.85 LT	677.53
8	49+08.98	15.84 RT	677.25

SLEEPER SLAB SURFACE			
No.	STA.	OFFSET	ELEV.
9	49+04.99	25.99 LT	675.70
10	49+04.99	1.52' LT	676.08
11	49+04.99	17.70 RT	675.77
12	49+12.99	17.08 RT	675.85
13	49+12.99	2.24 LT	676.15
14	49+12.99	27.56 LT	675.76

APPROACH SLAB CURVE DATA

CURVE A
 $\Delta = 5^\circ 17' 11''$
 $D = 40.76'$
 $R = 140.56'$
 $T = 6.49'$
 $L = 12.97'$
 $CH. = 12.96'$
 $CB = N 84^\circ 55' 05'' E$

CURVE B
 $\Delta = 3^\circ 24' 38''$
 $D = 19.69'$
 $R = 291.00'$
 $T = 8.66'$
 $L = 17.32'(+)$
 $CH. = 17.32'(-)$
 $CB = N 80^\circ 36' 53'' E$

CURVE C
 $\Delta = 5^\circ 24' 59''$
 $D = 18.03'$
 $R = 317.80'$
 $T = 15.03'$
 $L = 30.04'$
 $CH. = 30.03'$
 $CB = N 87^\circ 22' 25'' E$

APPROACH SLAB REINFORCING REQUIRED LAP LENGTHS	
NO. 5 BARS	3'-5" MIN.

LEGEND:
 * MEASURED PERPENDICULAR TO ϕ CONSTRUCTION INDUSTRIAL DR.
 ◻ - SLEEPER SLAB ELEVATIONS

- NOTES:**
- FOR APPROACH SLAB DETAIL NOTES, SEE SHEET [57/64].
 - FOR SECTION A-A, SEE SHEET [57/64].
 - FORWARD APPROACH SLAB TO BE INCLUDED WITH ITEM 526, REINFORCED CONCRETE APPROACH SLAB WITH QC/QA (T=17") AS PER PLAN.
 - CURVE INFORMATION DEFINES OUTSIDE EDGES OF APPROACH/SLEEPER SLAB.

1800 INDIAN WOOD CIRCLE
 MAUMEE, OHIO 43537

DATE: 05/2015
 REVIEWED: TLR
 DRAWN: ANK
 DESIGNED: KRH
 CHECKED: SCT

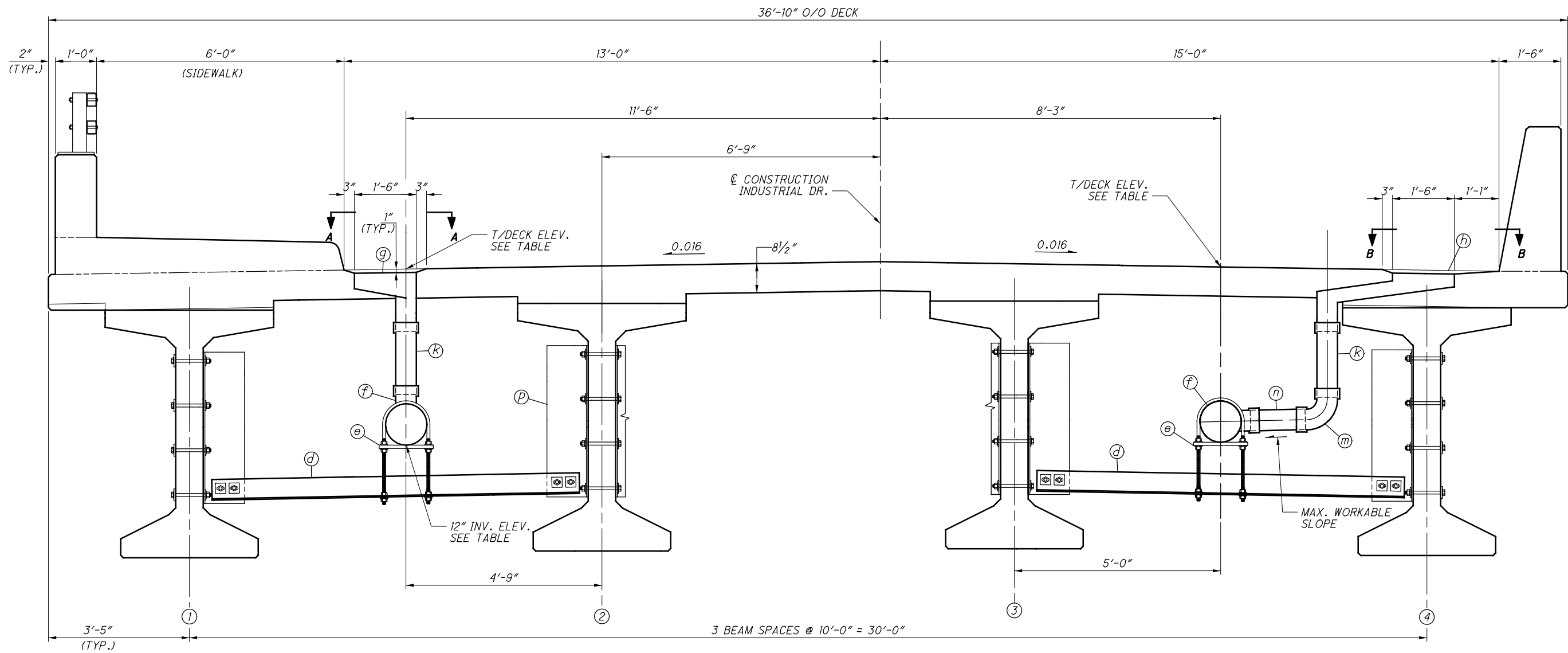
FORWARD APPROACH SLAB DETAILS
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
 PID No. 22984

58/64

142
 180

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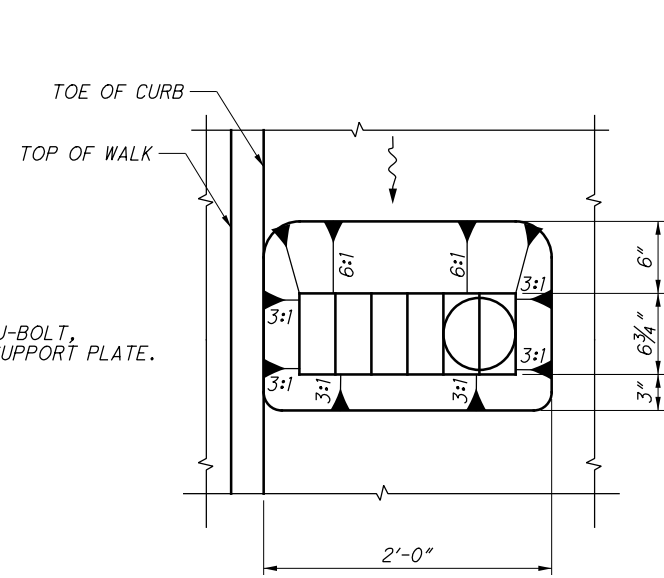
SCUPPER DETAIL

INTERMEDIATE DIAPHRAGM CROSSFRAMING NOT SHOWN

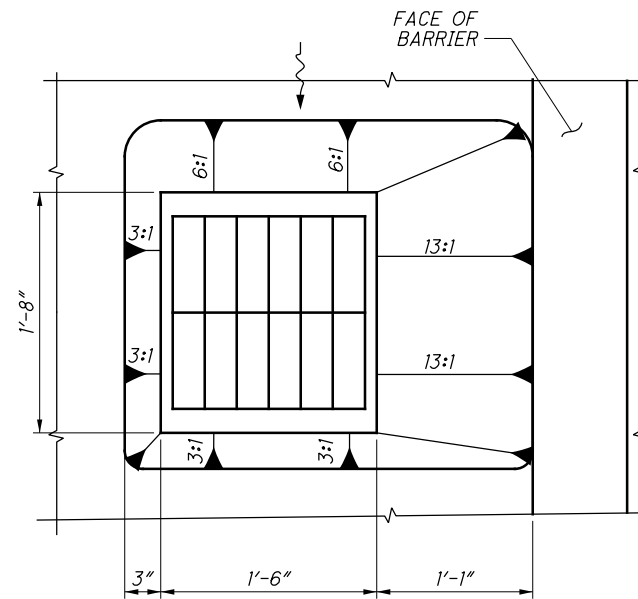
SCUPPER LOCATION TABLE			
STATION	SIDE	T/DECK	INVERT
39+68.50	LEFT	670.94	666.67
39+68.50	RIGHT	670.99	667.00
40+90.00	LEFT	671.76	667.89
40+90.00	RIGHT	671.81	669.43
42+07.00	LEFT	672.56	669.06
43+25.00	LEFT	673.37	670.24
44+43.00	LEFT	674.16	671.42
45+60.00	LEFT	674.96	672.59

LEGEND:

- Ⓝ - DESIGNATES BEAM LINE NUMBER
- Ⓣ - L6x6x3/8 SEE TYPE A CROSSFRAME, SEE SHEET 39/64.
- ⓔ - 12"φ SUPPORT CLAMP, COMPLETE WITH 1/8"φ VARIABLE LENGTH U-BOLT, NUTS, FLAT WASHERS, BEVELED WASHERS, LOCK WASHERS AND SUPPORT PLATE.
- ⓕ - 12"x12"x6" TEE (PVC)
- ⓖ - TYPE 1 SCUPPER - PER ODOT SD-1-69
- ⓗ - TYPE 2 SCUPPER - CUSTOMIZED NEENAH TYPE R-3935
- Ⓚ - 6"φ STRAIGHT PIPE (PVC), VARIABLE LENGTH, HUB ON ONE END
- Ⓛ - 6"x6" STD. RADIUS 90° BEND (PVC)
- Ⓜ - 6"φ STRAIGHT "SPOOL PIECE", NO HUB (PVC)
- Ⓨ - STEEL SUPPORT, SEE STD. DWG. PSID-1-13



SECTION A-A



SECTION B-B

NOTES:

1. FOR TRANSVERSE SECTIONS ALONG SPANS 1 - 7, SEE SHEET 39/64.
2. FOR DECK PLAN, SEE SHEETS 37-38/64.
3. FOR FRAMING PLAN, SEE SHEETS 28-50/64.
4. FOR RAILING DETAILS, SEE SHEETS 49-52/64.
5. FOR SIDEWALK DETAILS, SEE SHEET 53/64.
6. FOR PRESTRESSED I-BEAM DETAILS, SEE SHEET 31-32/64
7. FOR STEEL INTERMEDIATE DIAPHRAGM DETAILS, SEE STD. DWG. PSID-1-13.
8. PROVIDE 12"φ PIPE EXPANSION FITTING IN SPAN 1 NEAR REAR ABUTMENT DIAPHRAGM. TYPE SUBJECT TO ENGINEER APPROVAL.
9. ALL PVC PIPE SHALL CONFORM TO ITEM 707.45.
10. INCLUDE Ⓣ AND Ⓨ WITH ITEM 513 FOR PAYMENT.
11. INCLUDE ⓔ, ⓖ AND ⓗ WITH ITEM 518 - SCUPPERS, INCLUDING SUPPORTS, AS PER PLAN FOR PAYMENT.
12. INCLUDE Ⓚ, Ⓛ, Ⓜ AND ⓕ WITH ITEM 518 - 6" PIPE DOWNSPOUT, INCLUDING SPECIALS FOR PAYMENT.

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MARK	NUMBER			LENGTH	WEIGHT	TYPE	DIMENSIONS						
	REAR	FWD.	TOTAL				A	B	C	D	E	R	INC
ABUTMENT													
D801													
A401													
A501													
A502													
A503													
A504													
A505													
A506													
A507													
A508													
A509													
A510													
A511													
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A527													
A528													
A529													

MARK	NUMBER			LENGTH	WEIGHT	TYPE	DIMENSIONS						
	REAR	FWD.	TOTAL				A	B	C	D	E	R	INC
ABUTMENT													
A601													
A602													
A603													
A604													
A605													
A606													
A607													
A608													
A609													
A610													
A611													
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A808													
A809													
A810													
A811													
A812													
A813													
A814													
A815													
A816													
A817													
A901													
A902													
A903													
				<i>SUB-TOTAL</i>									

- NOTES**
- ALL REINFORCING STEEL SHALL BE EPOXY COATED.
 - FOR BAR BEND DIAGRAMS, SEE SHEET 64/64.



DESIGNED: KRH
 CHECKED: SCT
 DRAWN: KRH
 REVISED:
 REVIEWED: TLR
 DATE: 05/2015
 STRUCTURE FILE NUMBER: TBD

REINFORCING STEEL LIST (1 OF 4)
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-0000
 PID No. 22984

60/64

144
180

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MARK	TEST SHAFT	NUMBER							TOTAL	LENGTH	WEIGHT	TYPE	DIMENSIONS						
		PIER 1	PIER 2	PIER 3	PIER 4	PIER 5	PIER 6	PIER 7					A	B	C	D	E	R	INC
DRILLED SHAFT*																			
SP401																			
SP402																			
SP501																			
SP502																			
SP503																			
SP504																			
SP505																			
SP506																			
DS1101																			
DS1102																			
DS1103																			
DS1104																			
DS1105																			
DS1106																			
DS1107																			
SUB-TOTAL																			

MARK	NUMBER							TOTAL	LENGTH	WEIGHT	TYPE	DIMENSIONS							
	PIER 1	PIER 2	PIER 3	PIER 4	PIER 5	PIER 6	PIER 7					A	B	C	D	E	R	INC	
PIER																			
P501																			
P502																			
P503																			
P504																			
P505																			
P506																			
P507																			
P601																			
P602																			
P603																			
P604																			
P605																			
P606																			
P607																			
P901																			
P1101																			
P1102																			
P1103																			
P1104																			
P1105																			
P1106																			
P1107																			
P1108																			
P1109																			
P1110																			
SP403																			
SP404																			
SP405																			
SP406																			
SP407																			
SP408																			
SP409																			
SUB-TOTAL																			

NOTES

- ALL REINFORCING STEEL SHALL BE EPOXY COATED.
- FOR BAR BEND DIAGRAMS, SEE SHEET [64/64].
- * - INDICATES ITEM IS INCLUDED IN COST OF ITEM 524 DRILLED SHAFTS.



REVIEWED DATE 05/2015
TLR
STRUCTURE FILE NUMBER TBD

DRAWN RJS
RJS
REVISSED

DESIGNED RJS
RJS
CHECKED SCT
SCT

REINFORCING STEEL LIST (2 OF 4)
HEN-INDUSTRIAL DRIVE-0000
INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
PID No. 22984

61/64

145
180

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MARK	NUMBER		LENGTH	WEIGHT	TYPE	DIMENSIONS					
	TOTAL					A	B	C	D	E	R
SUPERSTRUCTURE											
S401											
S402											
S501											
S502											
S503											
S504											
S505											
S506											
S507											
S508											
S601											
S602											
<i>SUB-TOTAL</i>											
SUPERSTRUCTURE - SIDEWALK											
S503											
S504											
S505											
S506											
S509											
S510											
S511											
<i>SUB-TOTAL</i>											
SUPERSTRUCTURE - BR-2-98											
S503*											
S504*											
S512*											
<i>SUB-TOTAL</i>											
SUPERSTRUCTURE - SBR-1-13											
S503											
S504											
S513											
S514											
S515											
S516											
S603											
S604											
S605											
S606											
S607											
<i>SUB-TOTAL</i>											

NOTES

- ALL REINFORCING STEEL SHALL BE EPOXY COATED.
- FOR BAR BEND DIAGRAMS, SEE SHEET [64/64].
- * - INDICATES ITEM IS INCLUDED IN COST OF ITEM 517 RAILING (CONCRETE PARAPET WITH TWIN STEEL TUBE RAILING).
- ** - INDICATES ITEM IS INCLUDED IN COST OF ITEM 526 REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=17").

MARK	NUMBER			LENGTH	WEIGHT	TYPE	DIMENSIONS					
	REAR	FWD.	TOTAL				A	B	C	D	E	R
APPROACH SLAB												
AS501												
AS502												
AS503												
AS505												
AS506												
AS507												
AS508												
AS1001												
AS1002												
AS1003												
AS1004												
<i>SUB-TOTAL</i>												
APPROACH SLAB - SIDEWALK												
S509												
S5010												
S511												
S533												
S534												
<i>SUB-TOTAL</i>												
APPROACH SLAB - BR-2-98												
S517*												
S518*												
S519*												
S520*												
S521*												
S522*												
S523*												
S524*												
S525*												
S526*												
S608*												
S609*												
S610*												
<i>SUB-TOTAL</i>												
APPROACH SLAB - SBR-1-13												
S526												
S527												
S528												
S529												
S530												
S531												
S532												
S611												
S612												
S613												
S614												
S615												
S616												
<i>SUB-TOTAL</i>												



DESIGNED: RJS
 CHECKED: SCT
 DRAWN: RJS
 REVISED:
 REVIEWED: TLR
 DATE: 05/2015
 STRUCTURE FILE NUMBER: TBD

REINFORCING STEEL LIST (3 OF 4)
 HEN-INDUSTRIAL DRIVE-0000
 INDUSTRIAL DRIVE OVER MAUMEE RIVER

HEN-IND-00.00
 PID No. 22984

W:\Projects\Projects F - J\H2530002\22984\structures\HENIND_0000\Sheets\IND_0000CRL003.dgn 5/22/2015 4:06:12 PM SValentin

MARK	NUMBER							TOTAL	LENGTH	WEIGHT	TYPE	DIMENSIONS					
	PIER 1	PIER 2	PIER 3	PIER 4	PIER 5	PIER 6	PIER 7					A	B	C	D	E	R
PIER																	
S408																	
S409																	
S410																	
S411																	
S612																	
S613																	
S614																	
SUB-TOTAL																	

MARK	NUMBER			LENGTH	WEIGHT	TYPE	DIMENSIONS					
	REAR	FWD.	TOTAL				A	B	C	D	E	R
ABUTMENT DIAPHRAGMS												
S403												
S404												
S405												
S406												
S407												
S608												
S609												
S610												
S611												
S801												
S802												
S803												
S804												
S805												
S806												
S807												
S808												
SUB-TOTAL												

NOTES

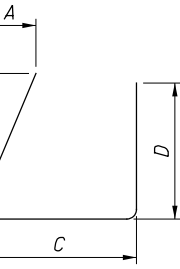
- ALL REINFORCING STEEL SHALL BE EPOXY COATED.
- FOR BAR BEND DIAGRAMS, SEE SHEET 64/64.

REINFORCING STEEL LIST (4 OF 4)	HEN-IND-00.00 HEN-INDUSTRIAL DRIVE-0000 INDUSTRIAL DRIVE OVER MAUMEE RIVER	PID No. 22984	147 180	63/64
DESIGNED	RJS	CHECKED	SCT	
DRAWN	RJS	REVISED		
REVIEWED	TLR	STRUCTURE FILE NUMBER	TBD	
DATE	05/2015			

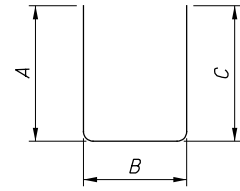
1800 INDIAN WOOD CIRCLE
MAUMEE, OHIO 43537

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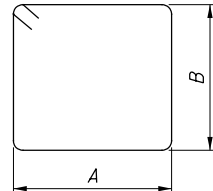
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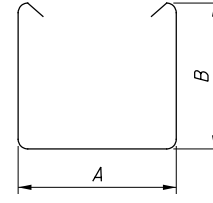
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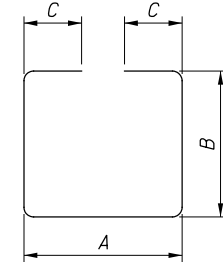
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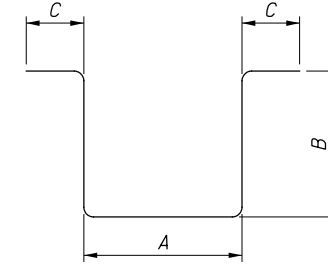
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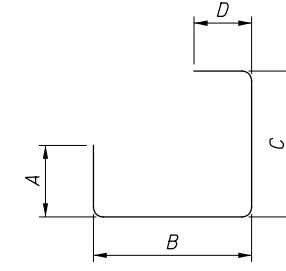
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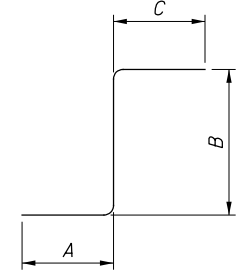
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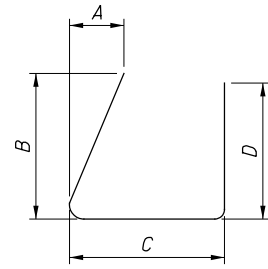
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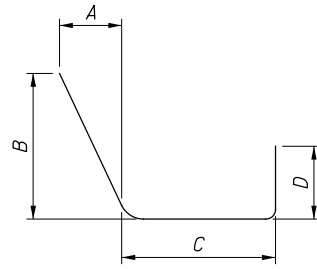
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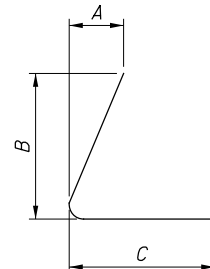
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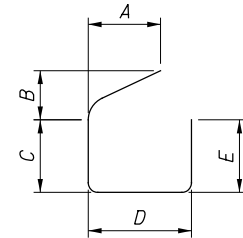
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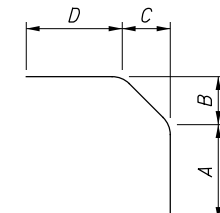
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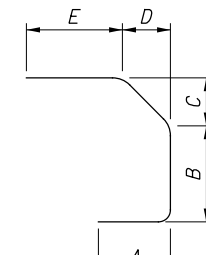
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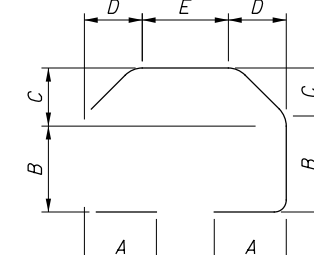
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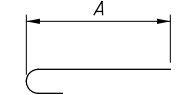
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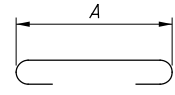
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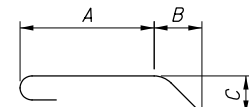
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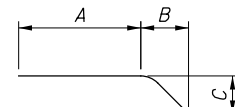
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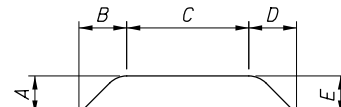
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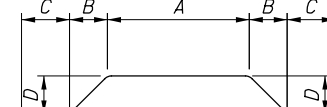
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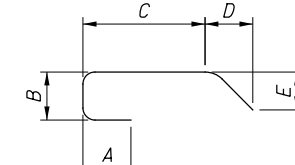
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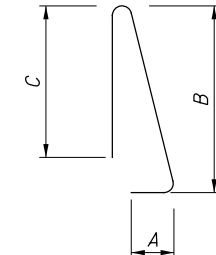
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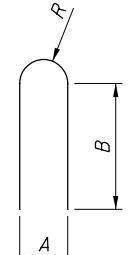
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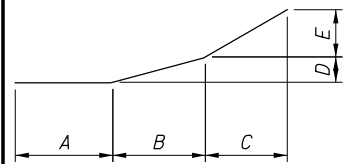
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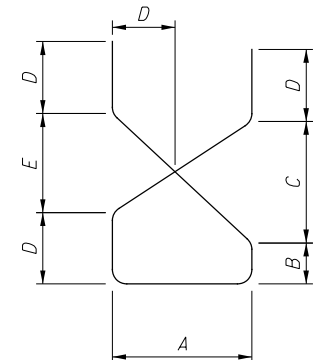
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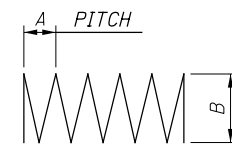
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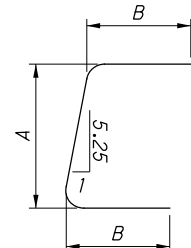
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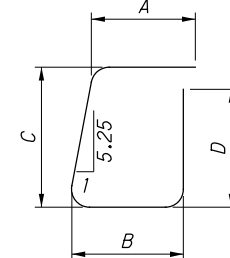
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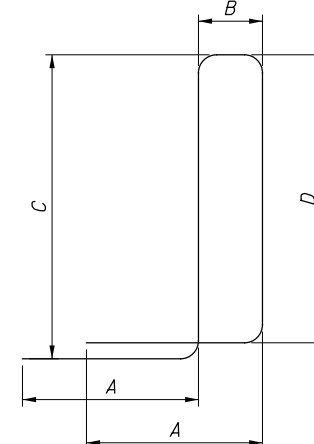
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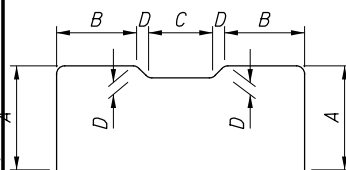
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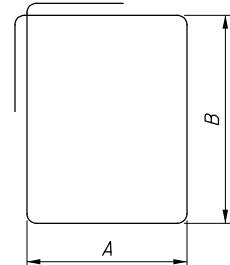
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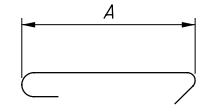
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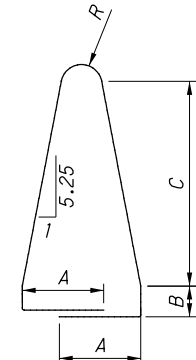
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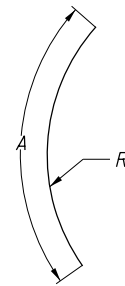
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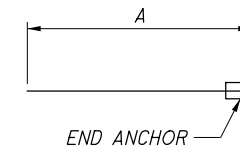
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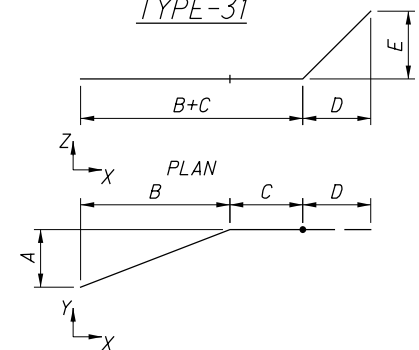
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TYPE-51



TYPE-31



TYPE-53

PROJECT DESCRIPTION

THE PROJECT, HEN-IND-0000, WILL INCLUDE CONSTRUCTING A NEW BRIDGE OVER THE MAUMEE RIVER NEAR THE CITY OF NAPOLEON, OHIO TO PROVIDE A CONNECTION BETWEEN SR 110 ON THE SOUTH SIDE OF THE RIVER TO RIVERVIEW AVENUE ON THE NORTH SIDE OF THE RIVER.

HISTORIC RECORDS

NO HISTORIC BORINGS WERE AVAILABLE FOR THIS PROJECT.

GEOLOGY

THE PROJECT SITE IS LOCATED WITHIN THE MAUMEE LAKE PLAINS PHYSIOGRAPHIC REGION OF OHIO WHICH GENERALLY CONSISTS OF PLEISTOCENE-AGE SILT AND CLAY LACUSTRINE DEPOSITS AND/OR DELTAIC SEDIMENTS OVERLYING GLACIAL TILL AND DEVONIAN-AGE LIMESTONE AND SHALE BEDROCK. THE CLOSE PROXIMITY OF THE SITE TO THE MAUMEE RIVER INDICATES THE SITE MAY CONTAIN RIVER ALLUVIUM WITH OVERBANK DEPOSITS. FLOODPLAINS AND TERRACES FLANK THE MAUMEE RIVER WITH OVERBURDEN SOILS GENERALLY CONSISTING OF SILTY AND CLAYEY FLOODPLAIN DEPOSITS OR SANDY AND LOAMY SOILS IN THE TERRACES. NEARBY WATER WELL LOGS INDICATE THE OVERBURDEN SOILS CONSIST OF CLAY AND GRAVEL OVERLYING SHALE THAT WAS ENCOUNTERED AT DEPTHS RANGING FROM 30 TO 40 FEET BELOW GROUND SURFACE (BGS). DRIFT THICKNESS MAPPING INDICATES THE OVERBURDEN IS APPROXIMATELY 15 TO 45 FEET THICK WITH BEDROCK POTENTIALLY AS SHALLOW AS THE GROUND SURFACE WITHIN THE RIVERBED.

RECONNAISSANCE

PERSONNEL FROM THE MANNIK & SMITH GROUP, INC. (MSG) CONDUCTED A SITE WALKTHROUGH ON JANUARY 20, 2014 TO OBSERVE AND DOCUMENT THE SITE CONDITIONS AND NOTE ANY GEOTECHNICAL RELATED ISSUES. A LARGE UNCONTROLLED FILL AREA IS LOCATED BETWEEN EAST RIVERVIEW AVENUE AND THE MAUMEE RIVER. THE CURRENT PROPERTY OWNER STATED TO MSG PERSONNEL THAT HE HAS BEEN DUMPING CONCRETE DEBRIS AND FILL IN THE AREA FOR SEVERAL YEARS IN ORDER TO ESTABLISH A LEVEL FINISH GRADE. THE PAVEMENT AREAS WITHIN THE PROJECT LIMITS APPEARED TO BE IN GOOD TO FAIR CONDITION WITH MINOR TO MODERATE TRANSVERSE AND LONGITUDINAL CRACKING NOTED.

SUBSURFACE EXPLORATION

TWENTY-EIGHT (28) SOIL BORINGS, B-001-0-13 THROUGH B-026-0-13 AND OFFSET BORINGS B-008-1-13 AND B-013-1-13 (HEREAFTER REFERRED TO AS B-001 THROUGH B-026, B-008-1 AND B-013-1) WERE COMPLETED AS PART OF THIS SUBSURFACE EXPLORATION BETWEEN APRIL 22, 2014 AND JUNE 12, 2014. BORINGS B-001 THROUGH B-005 AND B-013 THROUGH B-026 (EXCEPT B-016) WERE DRILLED DURING THE FIRST MOBILIZATION FROM APRIL 22, 2014 TO APRIL 29, 2014. BORINGS B-006 THROUGH B-012 (BARGE DRILLING) AND B-016 WERE DRILLED DURING THE SECOND MOBILIZATION FROM JUNE 3, 2014 TO JUNE 12, 2014. A TRACK-MOUNTED GEOPROBE 7822DT DRILL RIG WAS USED TO ADVANCE THE BORINGS BY MECHANICALLY TURNING 4-1/4-INCH INNER DIAMETER HOLLOW-STEM AUGERS INTO THE SOIL MATERIAL. DISTURBED SOIL SAMPLING WAS CONDUCTED USING THE STANDARD PENETRATION TEST (SPT) IN GENERAL ACCORDANCE WITH ASTM D1586. DISTURBED SAMPLING INTERVALS WERE VARIED DEPENDING ON THE BORING TYPE AND THE PROPOSED TOP OF SUBGRADE RELATIVE TO THE EXISTING TOP OF SUBGRADE. UNDISTURBED SAMPLING (I.E. SHELBY TUBE) WAS ALSO PERFORMED AT BORINGS B-002, B-004, B-005 AND B-013-1. THE AUTOMATIC HAMMER ON THE DRILL RIG WAS CALIBRATED ON MAY 10, 2013 AND HAS A DRILL ROD ENERGY RATIO OF 89.3%. AT THE TWO ABUTMENT LOCATIONS (B-005 AND B-013-1) 10 FEET OF BEDROCK WAS CORED AND AT THE BORINGS FOR THE BRIDGE PIER LOCATIONS (B-006 TO B-012) PERFORMED ON A BARGE IN THE RIVER, 5 TO 20 FEET OF BEDROCK WAS CORED. THE SAMPLING OF THE BEDROCK WAS PERFORMED WITH A TYPE NW SERIES CORE BARREL.

EXPLORATION FINDINGS

SR 110

BORINGS B-017 TO B-021 WERE PERFORMED ALONG SR 110. AT THE GROUND SURFACE OF ALL THE BORINGS EXCEPT B-020, 10 INCHES OF ASPHALT PAVEMENT UNDERLAIN BY 4 INCHES OF AGGREGATE BASE WAS ENCOUNTERED. AT B-020 PERFORMED JUST OUTSIDE THE PAVEMENT, THE SURFICIAL MATERIAL CONSISTED OF 3 INCHES OF TOPSOIL. THE SOIL OVERBURDEN CONSISTS PRIMARILY OF DEPOSITS OF CLAY WITH LAYERS OF SAND AND GRAVEL UNDER THE AGGREGATE BASE IN B-017 AND B-019. SOIL UNIT 1 CONSISTS OF LOOSE TO MEDIUM DENSE, GRAY COARSE AND FINE SAND AND GRAVEL WITH SAND AND SILT (A-3a AND A-2-4) ENCOUNTERED BENEATH THE AGGREGATE BASE TO DEPTHS OF 2.7 FEET AND 4.2 FEET, RESPECTIVELY, IN B-019 AND B-017. THE N_{60} VALUES RANGED FROM 10 TO 22 BLOWS PER FOOT (BPF) WITH AN AVERAGE OF 18 BPF. THE MOISTURE CONTENTS RANGED BETWEEN 6 AND 21 PERCENT WITH AN AVERAGE OF 13 PERCENT. SOIL UNIT 2 CONSISTS PREDOMINANTLY OF STIFF TO HARD, BROWN AND/OR GRAY SILT AND CLAY, SILTY CLAY AND CLAY (A-6a, A-6b AND A-7-6). THIS SOIL UNIT WAS ENCOUNTERED BENEATH SOIL UNIT 1 AT B-017 AND B-019 AND BELOW THE PAVEMENT SECTION IN THE OTHER BORING LOCATIONS AND CONTINUED TO DEPTHS OF 6.2 TO 7.2 FEET (BORING TERMINATION DEPTHS). THE N_{60} VALUES RANGED FROM 10 TO 60 BPF WITH AN AVERAGE OF 32 BPF. THE POCKET PENETROMETER TESTS RESULTS WERE BETWEEN 2.25 AND 4.5+ TONS PER SQUARE FOOT (TSF). THE MOISTURE CONTENTS RANGED BETWEEN 13 AND 27 PERCENT WITH AN AVERAGE OF 19 PERCENT.

INDUSTRIAL DRIVE

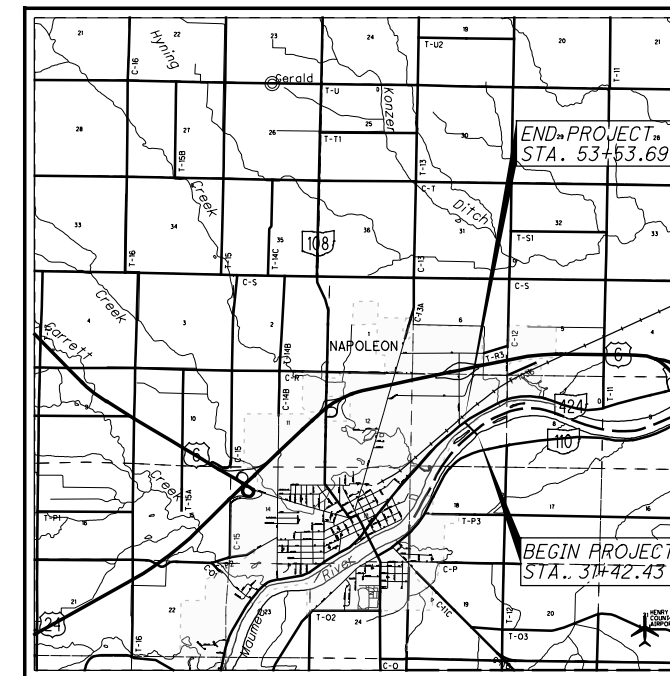
BORINGS B-001 TO B-016 WERE PERFORMED ALONG THE EXISTING AND PROPOSED ALIGNMENT OF INDUSTRIAL DRIVE. AT THE GROUND SURFACE OF BORINGS B-001 TO B-005, B-013-1 AND B-015, 4 TO 12 INCHES OF TOPSOIL WAS ENCOUNTERED. AT BORING B-014, THE SURFICIAL MATERIAL CONSISTED OF 12 INCHES OF GRAVEL BACKFILL. AT BORING B-016 PERFORMED WITHIN THE EXISTING INDUSTRIAL DRIVE PAVEMENT, 8 INCHES OF ASPHALT OVERLYING 12 INCHES OF AGGREGATE BASE WAS ENCOUNTERED. THE SOIL OVERBURDEN CONSISTS PRIMARILY OF DEPOSITS OF SILTY CLAY. THE BEDROCK WAS IDENTIFIED TO BE SHALE. AT B-013, VERY LOOSE BROWN SANDY SILT FILL WAS UNDERLAIN BY CONCRETE RUBBLE. REFUSAL IN THE CONCRETE RUBBLE WAS ENCOUNTERED AT 4 FEET BGS. SOIL UNIT 1 CONSISTS PREDOMINANTLY OF LOOSE TO MEDIUM DENSE, BROWN OR GRAY NON-COHESIVE SANDY SILT (A-4a) ENCOUNTERED BELOW THE SURFICIAL MATERIAL IN BORINGS B-003, B-004 AND B-013-1 TO DEPTHS OF 2.5 TO 6 FEET BGS. THE N_{60} VALUES RANGED FROM 6 TO 12 BPF WITH AN AVERAGE OF 9 BPF. HOWEVER, AT B-004, VERY LOOSE (N_{60} OF 3 BPF) SANDY SILT WAS ENCOUNTERED BETWEEN 3.5 AND 6 FEET BGS. THE MOISTURE CONTENTS GENERALLY RANGED BETWEEN 14 AND 21 PERCENT WITH AN AVERAGE OF 19 PERCENT. THE SOIL UNIT AT B-013-1 HAD TRACE ORGANIC CONTENT AND WATER CONTENT WAS DETERMINED TO BE 47 PERCENT. SOIL UNIT 2 CONSISTS OF SOFT TO MEDIUM STIFF, BROWN AND/OR GRAY COHESIVE SANDY SILT AND SILT (A-4a AND A-4b) AS WELL AS SILT AND CLAY AND SILTY CLAY (A-6a AND A-6b). THE N_{60} VALUES RANGED FROM 3 TO 7 BPF WITH AN AVERAGE OF 5 BPF. HOWEVER, A LAYER OF VERY SOFT (N_{60} VALUE

CONTINUED ON SHEET 2

LEGEND

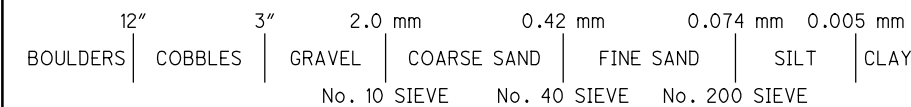
DESCRIPTION	ODOT CLASS	CLASSIFIED MECH./VISUAL
GRAVEL AND/OR STONE FRAGMENTS	A-1-a	1 0
GRAVEL AND/OR STONE FRAGMENTS WITH SAND, AND SILT	A-2-4	1 1
FINE SAND	A-3	1 0
COARSE AND FINE SAND	A-3a	0 2
SANDY SILT	A-4a	6 14
SILT	A-4b	1 1
SILT AND CLAY	A-6a	12 16
SILTY CLAY	A-6b	7 16
CLAY	A-7-6	10 9
	TOTAL	39 59
CONCRETE	VISUAL	
PAVEMENT OR BASE = X = APPROXIMATE THICKNESS	VISUAL	
SOD AND TOPSOIL	VISUAL	
SHALE BEDROCK		
EXPLORATION LOCATION - PLAN VIEW		
DRIVE SAMPLE AND/OR ROCK CORE BORING PLOTTED TO VERTICAL SCALE ONLY. HORIZONTAL BAR INDICATES A CHANGE IN STRATIGRAPHY.		
WC	INDICATES WATER CONTENT IN PERCENT.	
N₆₀	INDICATES STANDARD PENETRATION RESISTANCE NORMALIZED TO 60% DRILL ROD ENERGY RATIO.	
	INDICATES A PLASTIC MATERIAL WITH A MOISTURE CONTENT EQUAL TO OR GREATER THAN THE LIQUID LIMIT MINUS 3.	
	INDICATES A NON-PLASTIC MATERIAL WITH A MOISTURE CONTENT GREATER THAN 25% OR GREATER THAN 19% WITH A WET APPEARANCE.	
X/Y/D"	NUMBER OF BLOWS FOR STANDARD PENETRATION TEST (SPT): X = NUMBER OF BLOWS FOR 6 INCHES (UNCORRECTED) Y/D" = NUMBER OF BLOWS (UNCORRECTED) FOR D" OF PENETRATION AT REFUSAL	
	INDICATES STATIC WATER ELEVATION.	
	INDICATES FREE WATER ELEVATION.	
	INDICATES A SAMPLE TAKEN WITHIN 3 FT OF PROPOSED GRADE.	
SS	INDICATES A SPLIT SPOON SAMPLE.	
ST	INDICATES A SHELBY TUBE SAMPLE.	
NP	INDICATES A NON-PLASTIC SAMPLE.	
TR	INDICATES TOP OF ROCK.	

RECON. - LV 01/20/2014
 DRILLING - RJS 04/22 - 06/12/2014
 DRAWN - SJV 05/05/2015
 REVIEWED - JLS 05/19/2015



LOCATION MAP
SCALE IN MILES

PARTICLE SIZE DEFINITIONS



INDEX OF SHEETS					
LOCATION FROM STA. TO STA.	PLAN VIEW SHEET	PROFILE SHEET	CUT MAX.	FILL EMB. MAX.	
INDUSTRIAL DRIVE					
31+42.43 36+50.00	8	8	1 FT	10	FT
36+50.00 41+00.00	9	10	- FT	16	FT
41+00.00 45+50.00	11	12	- FT	-	FT
45+50.00 50+50.00	13	14	- FT	27	FT
50+50.00 56+00.00	15	15	1 FT	2	FT
RIVERVIEW AVENUE					
586+50.00 591+50.00	16	16	- FT	-	FT
591+50.00 596+50.00	17	17	- FT	2	FT
596+50.00 601+50.00	18	18	- FT	-	FT
STATE ROUTE 110					
95+00.00 100+00.00	19	19	- FT	-	FT
100+00.00 105+00.00	20	20	- FT	3	FT
105+00.00 110+00.00	21	21	- FT	5	FT

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EXPLORATION FINDINGS CONTINUED

INDUSTRIAL DRIVE CONTINUED

OF 1 BPF) SILTY CLAY WAS FOUND AT B-013-1 BETWEEN DEPTHS OF 6 AND 7.5 FEET BGS. THE POCKET PENETROMETER TESTS RESULTS WERE BETWEEN 0 AND 3.0 TSF. THE MOISTURE CONTENTS RANGED BETWEEN 17 AND 26 PERCENT WITH AN AVERAGE OF 22 PERCENT. SOIL UNIT 3 CONSISTS OF STIFF TO HARD, BROWN AND/OR GRAY COHESIVE SANDY SILT AND SILT (A-4a AND A-4b) AS WELL AS SILT AND CLAY, SILTY CLAY AND CLAY (A-6a, A-6b AND A-7-6). THIS UNIT WAS ENCOUNTERED BELOW SOIL UNITS 1 OR 2 AND WAS IDENTIFIED UP TO DEPTHS OF 8 TO 20 FEET WHERE THE BORINGS WERE TERMINATED OR BEDROCK WAS ENCOUNTERED. THE N_{60} VALUES RANGED FROM 9 TO 55 BPF WITH AN AVERAGE OF 25 BPF. THE POCKET PENETROMETER TESTS RESULTS WERE BETWEEN 2.0 AND 4.5+ TSF. THE MOISTURE CONTENTS RANGED BETWEEN 9 AND 26 PERCENT WITH AN AVERAGE OF 16 PERCENT. ROCK UNIT 1 CONSISTS OF WEAK TO STRONG, BROWN OR BLACK SHALE. THESE FORMATIONS WERE FOUND AT DEPTHS OF 22 FEET BGS AND 19.2 FEET BGS AT BORINGS B-005 AND B-013-1, RESPECTIVELY, AND AT THE SURFACE FOR THE RIVER BORINGS B-006 TO B-012. THE ROD OF THE ROCK CORES WERE BETWEEN 0 AND 75 PERCENT WITH AN AVERAGE VALUE OF 29 PERCENT. NINE (9) UNCONFINED COMPRESSIVE STRENGTH TESTS PERFORMED ON THE ROCK CORES INDICATED THE UNCONFINED COMPRESSIVE STRENGTH WAS BETWEEN 1,423 AND 7,676 POUNDS PER SQUARE INCH (PSI) WITH AN AVERAGE OF 4,633 PSI. THE ROCK FORMATIONS WERE FOUND TO BE SLIGHTLY TO SEVERELY WEATHERED.

EAST RIVERVIEW AVENUE

BORINGS B-022 TO B-026 WERE PERFORMED ALONG EAST RIVERVIEW AVENUE. AT THE GROUND SURFACE OF ALL THE BORINGS, 10 INCHES OF ASPHALT PAVEMENT UNDERLAIN BY 4 INCHES OF AGGREGATE BASE WAS ENCOUNTERED. THE SOIL OVERBURDEN CONSISTS PRIMARILY OF DEPOSITS OF SILTY CLAY. SOIL UNIT 1 CONSISTS OF LOOSE TO MEDIUM DENSE, BROWN OR GRAY GRAVEL OR GRAVEL WITH SAND AND SILT (A-1-a AND A-2-4) FOUND AT BORINGS B-023 AND B-025. THE DEPOSIT IS ENCOUNTERED BELOW THE PAVEMENT SECTION AND CONTINUES TO A DEPTH OF 2.7 FEET. THE N_{60} VALUES WERE BETWEEN 22 TO 31 BPF WITH AN AVERAGE OF 27 BPF. THE MOISTURE CONTENTS RANGED BETWEEN 3 AND 16 PERCENT WITH AN AVERAGE OF 10 PERCENT. SOIL UNIT 2 CONSISTS OF MEDIUM DENSE TO DENSE, BROWN AND/OR GRAY FINE SAND AND SANDY SILT (A-3 AND A-4a). THESE DEPOSITS WERE FOUND AT ALL THE BORING LOCATIONS EXCEPT B-025 BELOW THE SURFICIAL MATERIAL OR SOIL UNIT 1 WITH A THICKNESS OF ABOUT 1.5 FEET. THE N_{60} VALUES RANGED FROM 16 TO 34 BPF WITH AN AVERAGE OF 22 BPF. THE MOISTURE CONTENTS RANGED BETWEEN 2 AND 21 PERCENT WITH AN AVERAGE OF 10 PERCENT. SOIL UNIT 3 CONSISTS PREDOMINANTLY OF STIFF TO HARD, BROWN AND/OR GRAY SILTY CLAY. THESE DEPOSITS WERE ENCOUNTERED BELOW SOIL UNITS 1 AND 2 AND EXTENDED TO THE BORING TERMINATION DEPTHS AT 7.2 FEET BGS. THE SOIL UNIT WAS IDENTIFIED AS A-4a, A-6a, A-6b AND/OR A-7-6 IN ALL THE SOIL BORINGS. THE N_{60} VALUES RANGED FROM 18 TO 63 BPF WITH AN AVERAGE OF 38 BPF. THE POCKET PENETROMETER TESTS RESULTS WERE BETWEEN 3.0 AND 4.5 TSF. THE MOISTURE CONTENTS RANGED BETWEEN 14 AND 25 PERCENT WITH AN AVERAGE OF 21 PERCENT.

GROUNDWATER

GROUNDWATER WAS ENCOUNTERED IN FOUR (4) OF THE BORINGS DRUING DRILLING ACTIVITIES INCLUDING B-002, B-003, B-005 AND B-031-1 WITH WATER LEVELS MEASURED AT THE COMPLETION OF DRILLING AT DEPTHS RANGING FROM APPROXIMATELY 2.5 TO 16 FEET BGS. THE WATER LEVELS WERE MEASURED IN THE RIVER BORINGS AND INDICATED THAT THE RIVER ELEVATIONS VARIED BETWEEN ELEVATION 638 AND 641 AT THE TIME OF DRILLING.

SPECIFICATIONS

THIS GEOTECHNICAL EXPLORATION WAS PERFORMED IN ACCORDANCE WITH THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, OFFICE OF GEOTECHNICAL ENGINEERING, SPECIFICATIONS FOR GEOTECHNICAL EXPLORATIONS, DATED JULY 2013.

AVAILABLE INFORMATION

ALL AVAILABLE SOIL AND BEDROCK INFORMATION THAT CAN BE CONVENIENTLY SHOWN ON THE GEOTECHNICAL EXPLORATION SHEETS HAS BEEN SO REPORTED. ADDITIONAL EXPLORATIONS MAY HAVE BEEN MADE TO STUDY SOME SPECIAL ASPECT OF THE PROJECT. COPIES OF THIS DATA, IF ANY, MAY BE INSPECTED IN THE DISTRICT DEPUTY DIRECTOR'S OFFICE, THE OFFICE OF GEOTECHNICAL ENGINEERING AT 1600 WEST BROAD STREET OR THE OFFICE OF STRUCTURAL ENGINEERING AT 1980 WEST BROAD STREET.

BEDROCK TEST SUMMARY			
BORING NO.	SAMPLE ID	SAMPLE DEPTH	Qu (PSI)
B-006-0-13	RC-2	5'-10'	1423
B-006-0-13	RC-4	15'-20'	1641
B-007-0-13	RC-2	5'-10'	4446
B-008-1-13	RC-2	5'-10'	4862
B-009-0-13	RC-4	15'-20'	1451
B-010-0-13	RC-2	5'-10'	7676
B-011-0-13	RC-1	0'-5'	4644
B-012-0-13	RC-1	0'-5'	5539
B-012-0-13	RC-3	10'-15'	6652

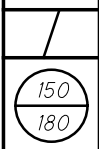
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DESIGN AGENCY
 PID NO.
22984

SOIL PROFILE

HEN - IND - 0000



SUMMARY OF SOIL TEST DATA
INDUSTRIAL DRIVE

EXPLORATION NO., STATION & OFFSET	DEPTH		SAMPLE ID	% REC	HP tsf	% GR	% CS	% FS	% SILT	% CLAY	LL	PL	PI	% WC	ODOT CLASS (GI)	SO4 ppm		
	FROM	TO																
B-001-0-13 STA. 32+47.39, 102.9 LT. NORTHING = 636432.356 EASTING = 1529466.854	1.0	2.5	SS-1	67	0.75	2	3	16	35	44	37	18	19	21	A-6b (12)			
	3.5	5.0	SS-2	50	2.0	BROWN MOTTLED WITH GRAY SILT AND CLAY, LITTLE SAND, TRACE GRAVEL										17	A-6a (VISUAL)	
	6.0	7.5	SS-3	0	-	SAME AS SS-2										-	A-6a (VISUAL)	
	8.5	10.0	SS-4	61	4.5	5	6	13	35	41	29	15	14	14	A-6a (10)			
	11.0	12.5	SS-5	94	4.5	SAME AS SS-4										14	A-6a (VISUAL)	
	13.5	15.0	SS-6	25	4.5	SAME AS SS-4										14	A-6a (VISUAL)	
B-002-0-13 STA. 34+29.06, 80.4 LT. NORTHING = 636554.632 EASTING = 1529405.397	1.0	2.5	SS-1	33	2.0	BROWN SILTY CLAY, TRACE SAND										21	A-6b (VISUAL)	
	3.5	5.5	ST-1	100	4.5+	3	6	15	35	41	29	16	13	22	A-6a (9)			
	6.0	7.5	SS-2	61	4.0	SAME AS ST-1										14	A-6a (VISUAL)	
	8.5	10.0	SS-3	72	4.5	SAME AS ST-1										13	A-6a (VISUAL)	
	11.0	12.5	SS-4	0	-	SAME AS ST-1										-	A-6a (VISUAL)	
	13.5	15.0	SS-5	83	4.5+	3	5	14	35	43	28	16	12	16	A-6a (9)			
B-003-0-13 STA. 36+41.71, 24.9 LT. NORTHING = 636705.174 EASTING = 1529253.905	1.0	2.5	SS-1	67	-	SAME AS SS-2										21	A-4a (VISUAL)	
	3.5	5.0	SS-2	61	-	1	4	50	34	11	NP	NP	NP	14	A-4a (2)			
	6.0	7.5	SS-3	78	4.5+	SAME AS SS-6										14	A-6a (VISUAL)	
	8.5	10.0	SS-4	78	4.5+	SAME AS SS-6										13	A-6a (VISUAL)	
	11.0	12.5	SS-5	78	4.5+	SAME AS SS-6										14	A-6a (VISUAL)	
	13.5	15.0	SS-6	44	4.5+	3	4	10	41	42	27	16	11	14	A-6a (8)			
B-004-0-13 STA. 38+25, 4' RT. NORTHING = 636837.733 EASTING = 1529144.268	1.0	2.5	SS-1	44	-	SAME AS SS-2										19	A-4a (VISUAL)	
	3.5	5.0	SS-2	83	-	0	2	29	46	23	NP	NP	NP	21	A-4a (7)			
	6.0	7.5	SS-3	56	-	SAME AS ST-1										16	A-4a (VISUAL)	
	6.0	7.5	ST-1	94	-	0	0	41	45	14	22	18	4	11	A-4a (5)			
	8.5	10.0	SS-4	83	-	GRAY SANDY SILT, AND CLAY										14	A-4a (VISUAL)	
	11.0	12.5	SS-5	72	4.5+	4	5	11	40	40	27	15	12	13	A-6a (9)			
B-014-0-13 STA. 49+50, CL NORTHING = 637665.517 EASTING = 1528381.896	13.5	15.0	SS-6	56	4.5+	SAME AS SS-5										15	A-6a (VISUAL)	
	1.0	2.5	SS-1	78	4.5+	5	7	13	32	43	32	17	15	15	A-6a (10)			
	3.5	5.0	SS-2	17	4.5	SAME AS SS-1										15	A-6a (VISUAL)	
	6.0	7.5	SS-3	50	4.0	SAME AS SS-1										17	A-6a (VISUAL)	
	8.5	10.0	SS-4	50	4.5+	2	5	12	34	47	37	16	21	20	A-6b (12)			
	11.0	12.5	SS-5	61	3.0	GRAY CLAY, SOME SILT, TRACE GRAVEL										20	A-7-6 (VISUAL)	
	13.5	15.0	SS-6	83	4.5	SAME AS SS-6										13	A-7-6 (VISUAL)	
	16.0	17.5	SS-7	78	4.5+	BROWN SILTY CLAY, LITTLE SAND, TRACE GRAVEL										17	A-6b (VISUAL)	
18.5	20.0	SS-8	89	4.5+	GRAY SILTY CLAY, LITTLE SAND, TRACE GRAVEL										9	A-6b (VISUAL)		
B-015-0-13 STA. 52+77.55, 26.7 RT. NORTHING = 637952.823 EASTING = 1528225.366	1.0	2.5	SS-1	67	2.5	BROWN SILT AND CLAY, LITTLE SAND										19	A-6a (VISUAL) *	1275
	3.5	5.0	SS-2	67	4.5	1	3	3	10	83	48	23	25	26	A-7-6 (16)			
	6.0	7.5	SS-3	83	4.5	4	6	11	30	49	32	17	15	17	A-6a (10)			
	8.5	10.0	SS-4	100	4.5	SAME AS SS-3										14	A-6a (VISUAL)	
B-016-0-13 STA. 55+57, CL NORTHING = 638227.110 EASTING = 1528197.360	1.7	3.2	SS-1	78	3.5	SAME AS SS-2										22	A-6b (VISUAL) *	
	3.2	4.7	SS-2	89	3.0	1	1	3	44	51	39	19	20	25	A-6b (12)			
	4.7	6.2	SS-3	44	2.5	1	1	4	37	57	44	20	24	28	A-7-6 (14)			
	6.2	7.7	SS-4	78	3.0	SAME AS SS-3										26	A-7-6 (VISUAL)	1140

NOTE:
Excludes B-005 through B-013

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SOIL PROFILE
SUMMARY OF SOIL TEST DATA - INDUSTRIAL DRIVE

HEN-IND-0000

SUMMARY OF SOIL TEST DATA
RIVERVIEW AVE.

EXPLORATION NO., STATION & OFFSET	DEPTH		SAMPLE ID	% REC	HP tsf	% GR	% CS	% FS	% SILT	% CLAY	LL	PL	PI	% WC	ODOT CLASS (GI)	SO4 ppm
	FROM	TO														
B-022-0-13	1.2	2.7	SS-1	56	-				DARK BROWN SANDY SILT, SOME GRAVEL					3	A-4a (MSUAL)*	160
STA. 587+45, 7' RT.	2.7	4.2	SS-2	56	4.0	6	4	5	35	50	44	19	25	23	A-7-6 (15)	
NORTHING = 637343.455	4.2	5.7	SS-3	67	4.5				SAME AS SS-2					14	A-7-6 (MSUAL)	
EASTING = 1527818.884	5.7	7.2	SS-4	61	4.5	15	10	17	23	35	32	17	15	17	A-6a (7)	
B-023-0-13	1.2	2.7	SS-1	61	-	66	18	7	7	2	NP	NP	NP	3	A-1-a (0)*	
STA. 591+43, 9' LT.	2.7	4.2	SS-2	61	3.25			GRAY MOTTLED WITH BROWN SANDY SILT, LITTLE CLAY					21	A-4a (MSUAL)	1300	
NORTHING = 637615.734	4.2	5.7	SS-3	89	4.5	0	4	5	19	72	40	20	20	22	A-6b (12)	
EASTING = 1528108.483	5.7	7.2	SS-4	83	4.5			SAME AS SS-3					24	A-6b (MSUAL)		
B-024-0-13	1.2	2.7	SS-1	72	-	0	5	85	9	1	NP	NP	NP	14	A-3 (0)	
STA. 594+66, 9' RT.	2.7	4.2	SS-2	67	3.0	1	0	1	41	57	49	22	27	25	A-7-6 (17)	
NORTHING = 637800.802	4.2	5.7	SS-3	100	4.5			BROWN SILTY CLAY, TRACE SAND					24	A-6b (MSUAL)	2880	
EASTING = 1528374.091	5.7	7.2	SS-4	50	4.5			SAME AS SS-3					23	A-6b (MSUAL)		
B-025-0-13	1.2	2.7	SS-1	83	-			LIGHT GRAY GRAVEL WITH SAND AND SILT, SOME CLAY					16	A-2-4 (MSUAL)*	200	
STA. 597+23, 8' LT.	2.7	4.2	SS-2	67	4.5	2	6	7	38	47	33	18	15	19	A-6a (10)	
NORTHING = 637966.627	4.2	5.7	SS-3	56	4.5			BROWN SILTY CLAY, TRACE SAND AND GRAVEL					19	A-6b (MSUAL)		
EASTING = 1528571.100	5.7	7.2	SS-4	78	4.5			SAME AS SS-3					22	A-6b (MSUAL)		
B-026-0-13	1.2	2.7	SS-1	50	-			DARK BROWN SANDY SILT, SOME GRAVEL					2	A-4a (MSUAL)*		
STA. 600+30, 5' LT.	2.7	4.2	SS-2	50	4.5			BROWN MOTTLED WITH GRAY SILTY CLAY, TRACE SAND					23	A-6b (MSUAL)	1140	
NORTHING = 638130.283	4.2	5.7	SS-3	78	4.5	3	5	8	22	62	41	20	21	17	A-7-6 (13)	
EASTING = 1528832.319	5.7	7.2	SS-4	89	4.5			SAME AS SS-3					15	A-7-6 (MSUAL)		

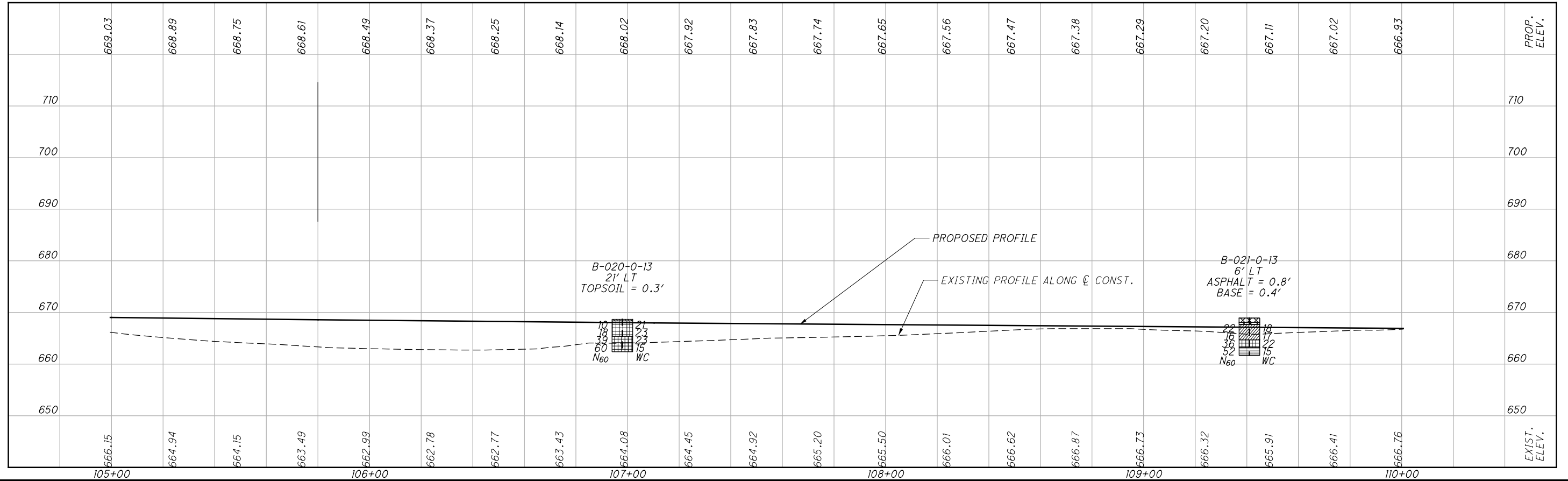
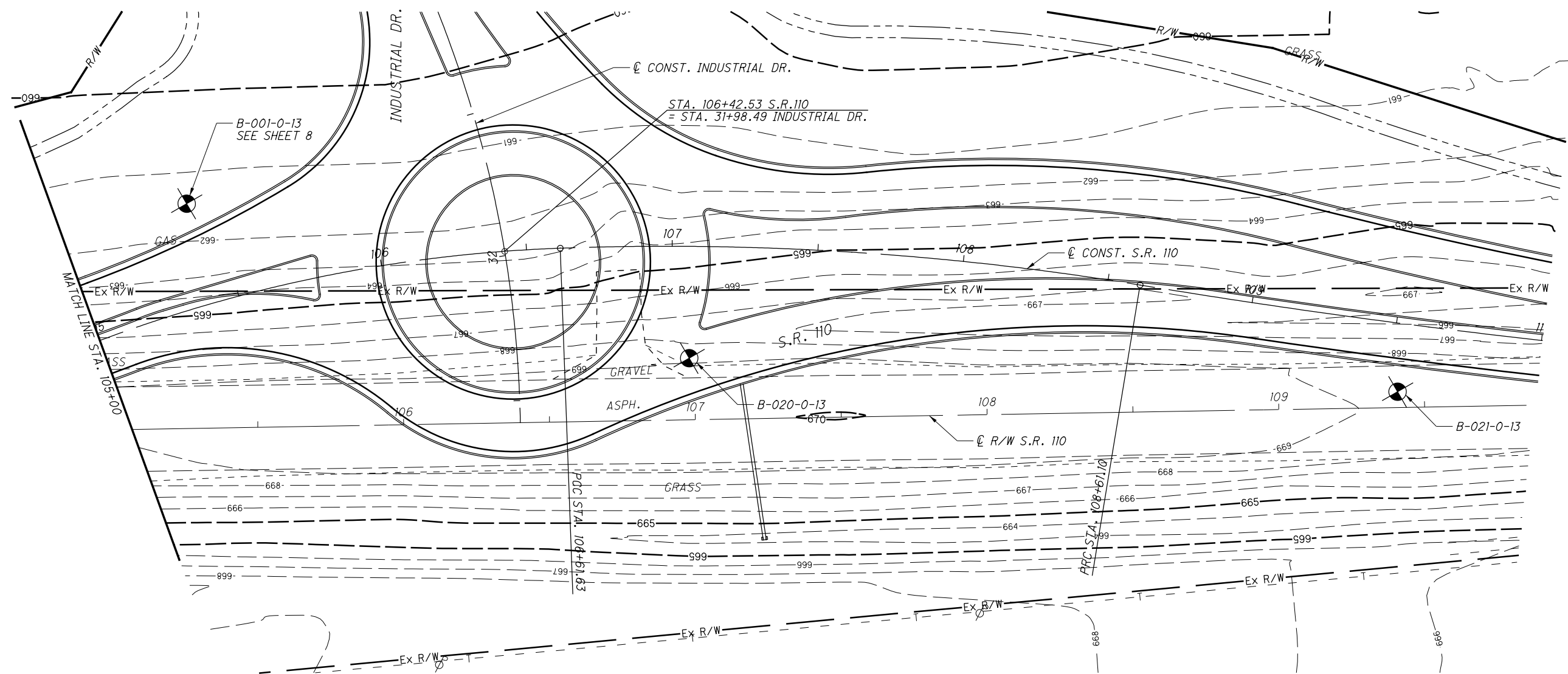
SUMMARY OF SOIL TEST DATA
S.R. 110

EXPLORATION NO., STATION & OFFSET	DEPTH		SAMPLE ID	% REC	HP tsf	% GR	% CS	% FS	% SILT	% CLAY	LL	PL	PI	% WC	ODOT CLASS (GI)	SO4 ppm
	FROM	TO														
B-017-0-13	1.2	2.7	SS-1	56	-			GRAY COARSE AND FINE SAND, LITTLE GRAVEL, TRACE SILT					6	A-3a (MSUAL)		
STA. 97+38, 5' RT.	2.7	4.2	SS-2	11	-			SAME AS SS-1					21	A-3a (MSUAL)	560	
NORTHING = 636139.886	4.2	5.7	SS-3	78	2.5	5	5	14	27	49	41	23	18	22	A-7-6 (11)	
EASTING = 1528743.168	5.7	7.2	SS-4	67	2.5	2	6	15	32	45	40	19	21	22	A-6b (12)	
B-018-0-13	1.2	2.7	SS-1	72	4.5+			DARK GRAY CLAY, SOME GRAVEL AND SILT, TRACE SAND					13	A-7-6 (MSUAL)	500	
STA. 99+90, 8' LT.	2.7	4.2	SS-2	67	4.5	2	10	35	20	33	26	12	14	16	A-6a (5)	
NORTHING = 636237.894	4.2	5.7	SS-3	67	2.25	1	2	10	16	71	49	20	29	27	A-7-6 (17)	
EASTING = 1528975.860	5.7	7.2	SS-4	78	3.5			SAME AS SS-3					20	A-7-6 (MSUAL)		
B-019-0-13	1.2	2.7	SS-1	61	-	42	18	10	14	16	23	14	9	16	A-2-4 (0)*	
STA. 103+15, CL	2.7	4.2	SS-2	56	4.5+	6	6	8	24	56	39	19	20	17	A-6b (12)	
NORTHING = 636312.038	4.2	5.7	SS-3	67	4.5+			SAME AS SS-2					16	A-6b (MSUAL)	760	
EASTING = 1529291.934	5.7	7.2	SS-4	78	4.5+			SAME AS SS-2					15	A-6b (MSUAL)		
B-020-0-13	0.3	1.8	SS-1	61	4.5+			SAME AS SS-2					21	A-7-6 (MSUAL)*	2160	
STA. 106+98, 21' LT.	1.8	3.3	SS-2	42	4.5+	0	2	11	18	69	49	21	28	23	A-7-6 (17)*	
NORTHING = 636420.916	3.3	4.8	SS-3	56	4.5+	1	2	3	16	78	48	24	24	23	A-7-6 (15)	
EASTING = 1529660.274	4.8	6.3	SS-4	78	4.5+			SAME AS SS-3					15	A-7-6 (MSUAL)		
B-021-0-13	1.2	2.7	SS-1	56	4.5+	13	9	30	23	25	28	15	13	18	A-6a (4)*	
STA. 109+41.6, 6' LT.	2.7	4.2	SS-2	61	3.0			SAME AS SS-1					17	A-6a (MSUAL)*	660	
NORTHING = 636461.405	4.2	5.7	SS-3	67	4.5+	3	4	5	16	72	47	25	22	22	A-7-6 (14)	
EASTING = 1529900.005	5.7	7.2	SS-4	78	4.5+			BROWN MOTTLED WITH GRAY SILTY CLAY, TRACE SAND AND GRAVEL					15	A-6b (MSUAL)		

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SUMMARY OF SOIL TEST DATA - RIVERVIEW AVE, & S.R. 110	
HEN-IND-0000	
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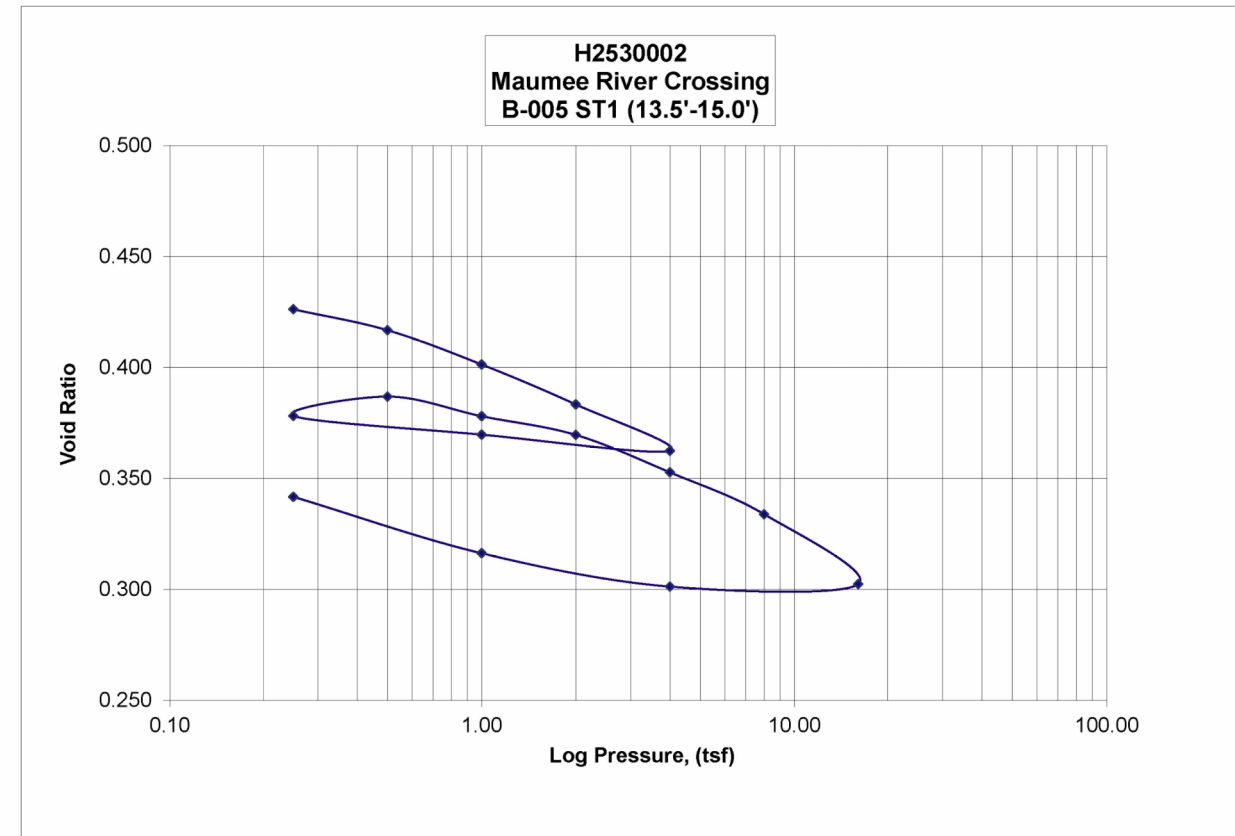
 Civil Engineering, Surveying and Geoenvironmental Consulting TOLEDO CANTON LANSING DETROIT MONROE CLEVELAND COLUMBUS		One-Dimensional Consolidation Properties of Soils ASTM D2435

Project Name:	Maumee River Crossing PID 22984	Project Number:	H2530002
Sample Number:	B-005-ST1	Sample Depth:	13.5'-15'
Soil Classification:	A-4a	Specific Gravity (G):	2.71
		Method Used:	Floating Ring

d_0 = Initial zero reading, (in)	0
Hs = (cm)	1.75
H0 = (in)	1.00
A = (in ²)	4.91
	0.70 (in)
	31.91 (cm ²)

Load Increment, Pressure (tsf)	Load Increment, Pressure (psi)	Deformation @ end of loading or ΔH (in)	Height of sample at the end of loading (in)	Height of Voids, (in)	Strain ($\Delta H/H_0$) (%)	Coefficient of Compressibility a_v (m ² /N)	Void Ratio	Permeability (cm/sec)	Coefficient of Consolidation, C_v @ 50% (in ² /sec)	Coefficient of Consolidation, C_v @ 50% (cm ² /sec)
seating load	0.69	0.000	1.000	0.30	0.00		0.430			
0.25	1.20	0.003	0.997	0.30	0.26	2.09E-05	0.426	3.819E-05	4.090E-03	2.659E-02
0.50	2.00	0.009	0.991	0.29	0.92	3.39E-05	0.417	9.811E-06	6.477E-04	4.210E-03
1.00	3.60	0.020	0.980	0.28	2.00	2.77E-05	0.401	1.803E-05	1.445E-03	9.394E-03
2.00	6.70	0.033	0.967	0.27	3.26	1.67E-05	0.383	1.962E-05	2.584E-03	1.679E-02
4.00	13.00	0.047	0.953	0.25	4.72	9.52E-06	0.362	3.305E-06	7.533E-04	4.897E-03
1.00	3.60	0.042	0.958	0.26	4.21	2.23E-06	0.370	7.362E-08	7.061E-05	4.589E-04
0.25	1.20	0.036	0.964	0.26	3.62	1.01E-05	0.378	1.190E-05	2.531E-03	1.645E-02
0.50	2.00	0.030	0.970	0.27	3.01	-3.13E-05	0.387	-7.471E-06	5.157E-04	3.352E-03
1.00	3.60	0.036	0.964	0.26	3.63	1.59E-05	0.378	1.245E-05	1.702E-03	1.106E-02
2.00	6.70	0.042	0.958	0.26	4.22	7.82E-06	0.370	1.368E-06	3.782E-04	2.458E-03
4.00	13.00	0.054	0.946	0.25	5.40	7.69E-06	0.353	2.656E-06	7.418E-04	4.822E-03
8.00	25.60	0.067	0.933	0.23	6.72	4.30E-06	0.334	9.759E-07	4.812E-04	3.128E-03
16.00	50.80	0.089	0.911	0.21	8.92	3.59E-06	0.302	9.500E-07	5.543E-04	3.603E-03
4.00	13.00	0.090	0.910	0.21	9.00	-8.69E-08	0.301	-2.884E-07	6.777E-03	4.405E-02
1.00	3.60	0.080	0.921	0.22	7.95	4.59E-06	0.316	5.184E-06	2.306E-03	1.499E-02
0.25	1.20	0.062	0.938	0.24	6.17	3.05E-05	0.342	1.924E-05	1.304E-03	8.477E-03

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SOIL PROFILE
LABORATORY TEST DATA

HEN - IND - 0000

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\\msgfilesrv\MSGData\Projects\Projects F - J\H2530002\22984\geotechnical\sheets\22984\005.dgn 5/22/2015 4:06:37 PM SValentin

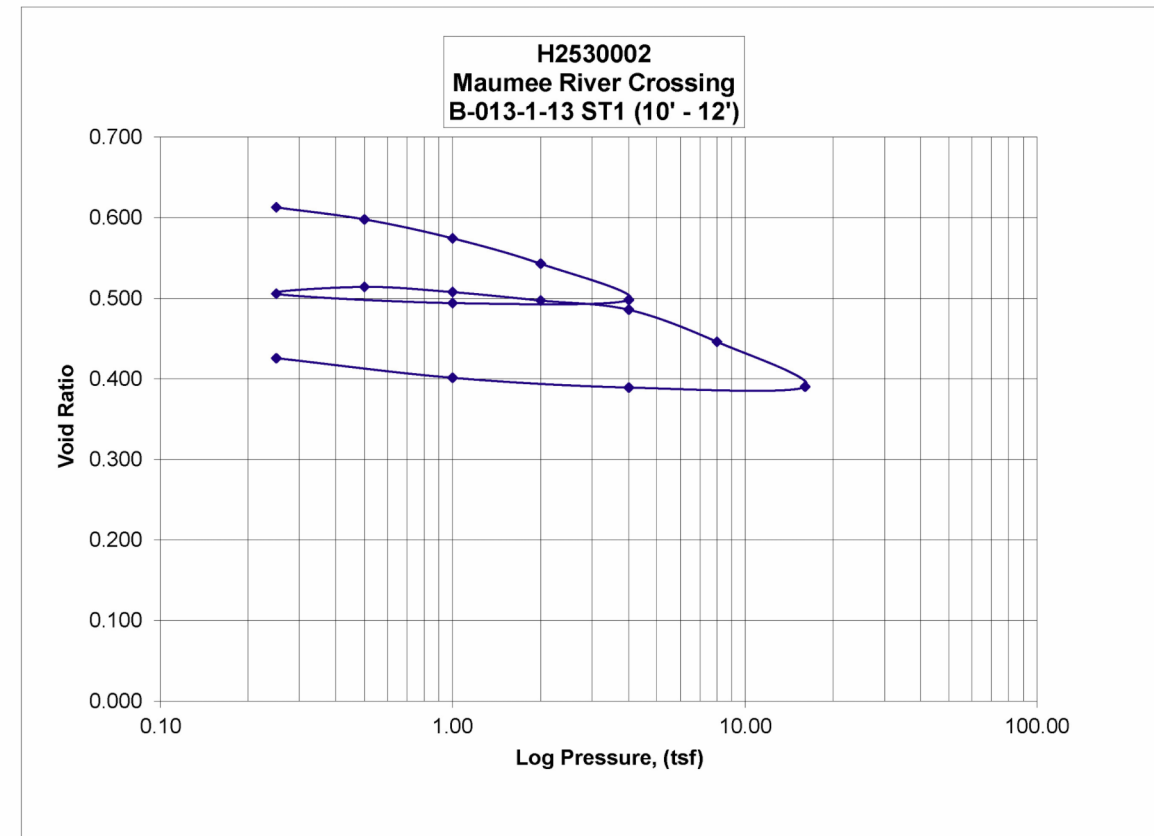
 <p>Civil Engineering, Surveying and Geoenvironmental Consulting TOLEDO CANTON LANSING DETROIT MONROE CLEVELAND COLUMBUS</p>		One-Dimensional Consolidation Properties of Soils ASTM D2435
---	---	--

Project Name: _____	Maumee River Crossing	Project Number: _____	H2530002
Sample Number: _____	B-013-1-13 ST1	Sample Depth: _____	10'-12'
Soil Classification: _____	A-6b	Specific Gravity (G): _____	2.71
		Method Used: _____	Floating Ring

d _s = Initial zero reading, (in)	0			
H _s = (cm)	1.53		0.61	(in)
H ₀ = (in)	1.00			
A= (in ²)	4.91		31.91	(cm ²)

Load Increment, Pressure (tsf)	Pressure (psi)	Deformation @ end of loading or Δ H (in)	Height of sample at the end of loading (in)	Height of Voids, (in)	Strain (Δ H/H ₀) (%)	Coefficient of Compressibility a _v (m ² /N)	Void Ratio	Permeability (cm/sec)	Coefficient of Consolidation, C _v @ 50% (in ² /sec)	Coefficient of Consolidation, C _v @ 50% (cm ² /sec)
0.25	1.40	0.014	0.986	0.37	1.40	9.26E-05	0.613	1.458E-05	4.037E-04	2.624E-03
0.50	2.20	0.023	0.977	0.37	2.31	5.34E-05	0.598	1.463E-07	6.925E-06	4.502E-05
1.00	3.80	0.038	0.963	0.35	3.75	4.23E-05	0.574	1.625E-06	9.634E-05	6.262E-04
2.00	6.90	0.057	0.943	0.33	5.68	2.92E-05	0.543	2.202E-06	1.859E-04	1.208E-03
4.00	13.30	0.084	0.916	0.30	8.42	2.01E-05	0.498	2.345E-06	2.821E-04	1.834E-03
1.00	3.80	0.087	0.913	0.30	8.66	-1.19E-06	0.494	-3.453E-07	6.836E-04	4.443E-03
0.25	1.40	0.080	0.920	0.31	7.96	1.37E-05	0.506	3.859E-07	6.600E-05	4.290E-04
0.50	2.20	0.074	0.926	0.31	7.43	-3.11E-05	0.514	-1.861E-06	1.412E-04	9.179E-04
1.00	3.80	0.078	0.922	0.31	7.83	1.17E-05	0.508	1.385E-06	2.800E-04	1.820E-03
2.00	6.90	0.085	0.915	0.30	8.47	9.70E-06	0.497	1.415E-06	3.450E-04	2.243E-03
4.00	13.30	0.092	0.908	0.30	9.17	5.14E-06	0.486	2.979E-06	1.361E-03	8.850E-03
8.00	25.90	0.116	0.884	0.27	11.60	9.06E-06	0.446	1.019E-06	2.622E-04	1.704E-03
16.00	51.30	0.150	0.850	0.24	15.02	6.33E-06	0.390	7.572E-07	2.715E-04	1.765E-03
4.00	13.30	0.151	0.849	0.24	15.07	-6.18E-08	0.389	-3.341E-08	1.176E-03	7.658E-03
1.00	3.80	0.143	0.857	0.25	14.33	3.66E-06	0.401	5.022E-07	2.990E-04	1.944E-03
0.25	1.40	0.128	0.872	0.26	12.84	2.92E-05	0.426	2.069E-06	1.559E-04	1.013E-03

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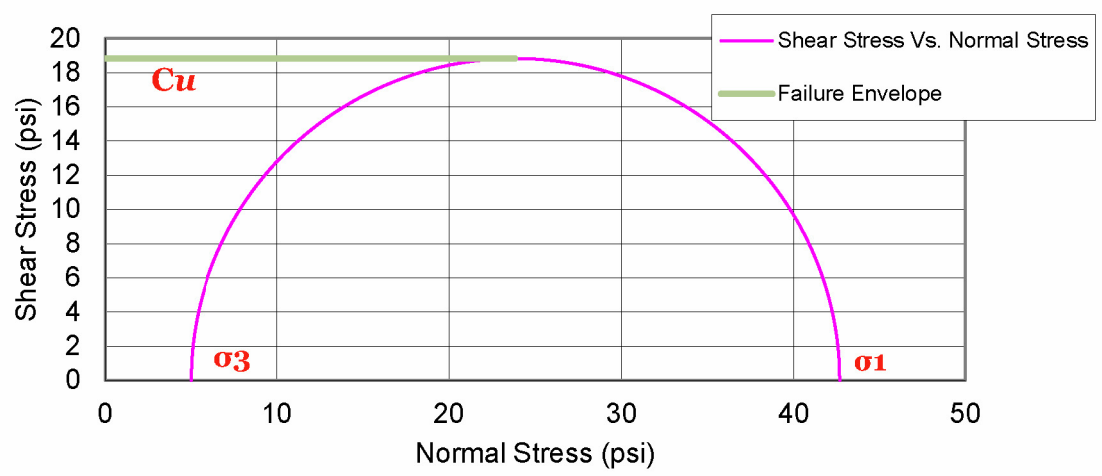
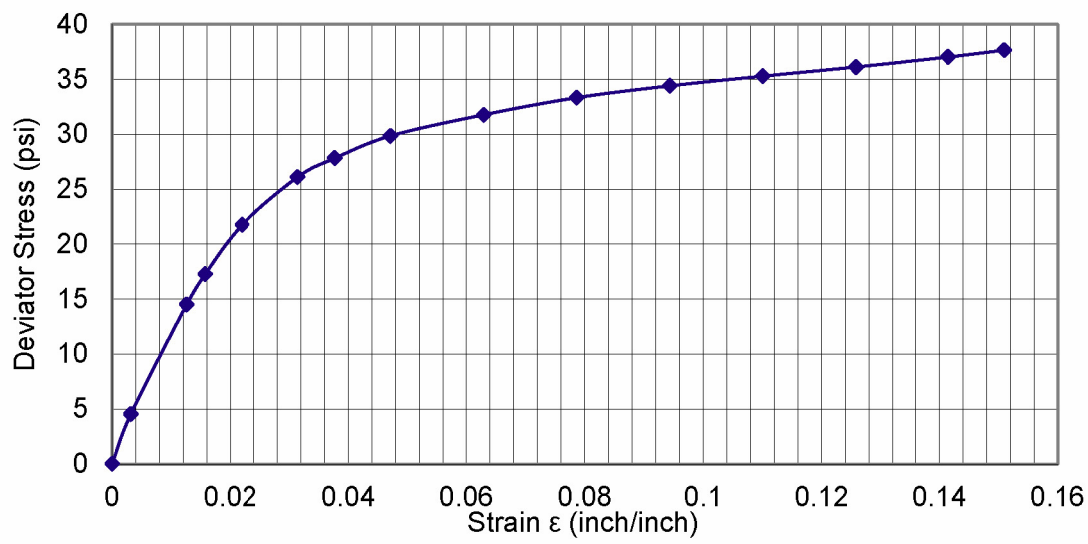
**SOIL PROFILE
LABORATORY TEST DATA**

HEN - IND - 0000

155
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	CANTON	MAUMEE	Unconsolidated-Undrained Triaxial Compression Test ASTM D2850
	DETROIT	COLUMBUS	
	MONROE	CLEVELAND	
	LANSING	TRAVERSE CITY	
Civil Engineering, Surveying and Environmental Consulting			

Project Name:	Maumee River Crossing PID 22984		Project No:	H2530002	
Sample No:	B-002-ST1		Date:	5/27/2014	
Visual Description of Soil:	Gray Silty Clay		Depth:	3.5-5.0'	
Wet Unit Wt. (lb/ft ³):	132.51	Dry Unit Wt. (lb/ft ³):	108.24	MC %:	22.43
Specific Gravity	2.70	Loading Rate (inch/min)	0.050	Void Ratio:	0.557
Chamber Pressure (psi)	5	Deviator Stress q _u (psi)	37.66	Cu (psi)	18.83

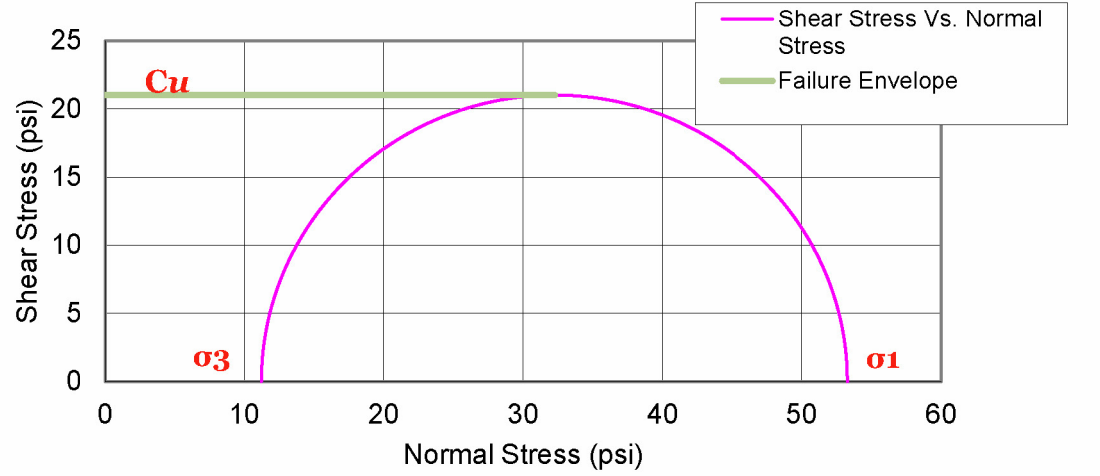
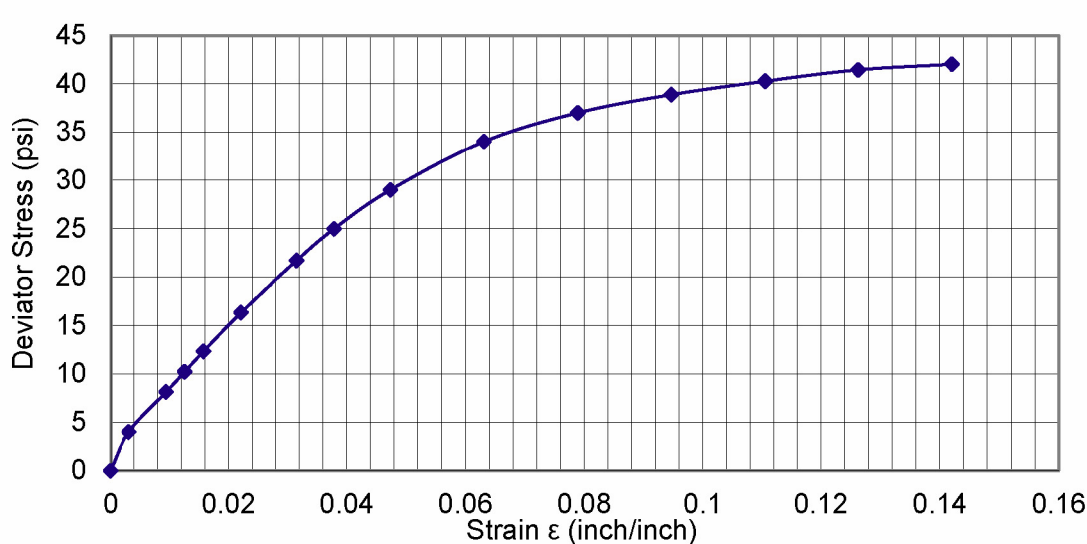


Tested By: KL Reviewed By: TR/CAR Date: 6/4/2014

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	CANTON	MAUMEE	Unconsolidated-Undrained Triaxial Compression Test ASTM D2850
	DETROIT	COLUMBUS	
	MONROE	CLEVELAND	
	LANSING	TRAVERSE CITY	
Civil Engineering, Surveying and Environmental Consulting			

Project Name:	Maumee River Crossing PID 22984		Project No:	H2530002	
Sample No:	B-005-ST1		Date:	5/27/2014	
Visual Description of Soil:	Gray Sandy Silt, some Clay		Depth:	13.5-15.0'	
Wet Unit Wt. (lb/ft ³):	135.03	Dry Unit Wt. (lb/ft ³):	113.73	MC %:	18.73
Specific Gravity	2.70	Loading Rate (inch/min)	0.050	Void Ratio:	0.481
Chamber Pressure (psi)	11.25	Deviator Stress q _u (psi)	42.04	Cu (psi)	21.01

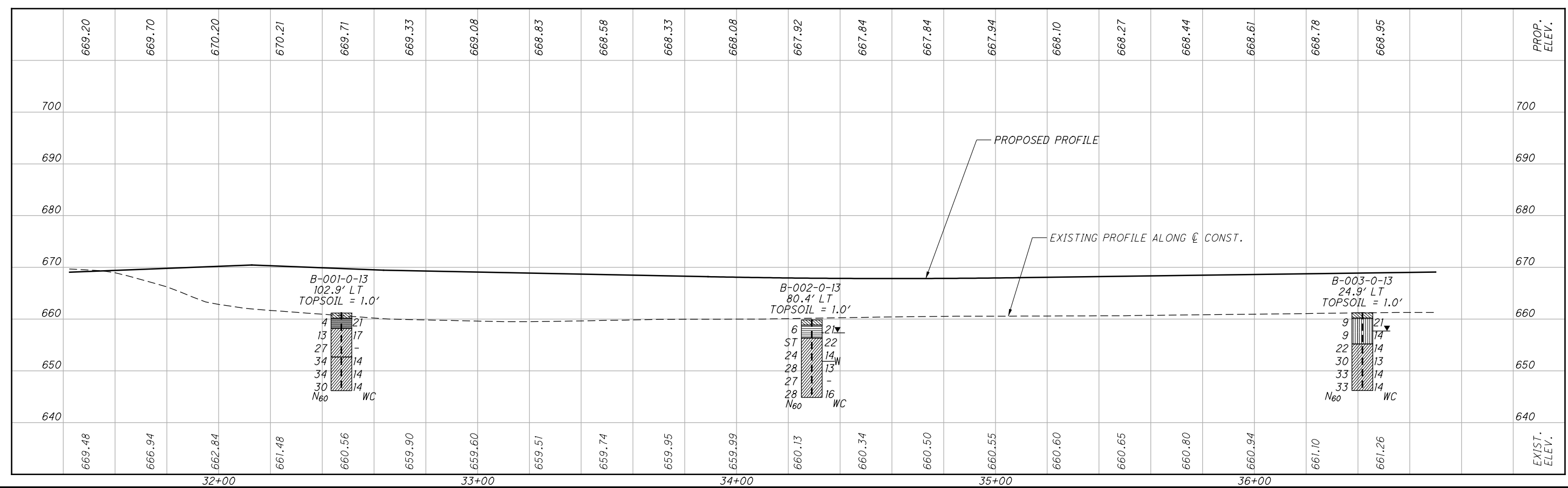
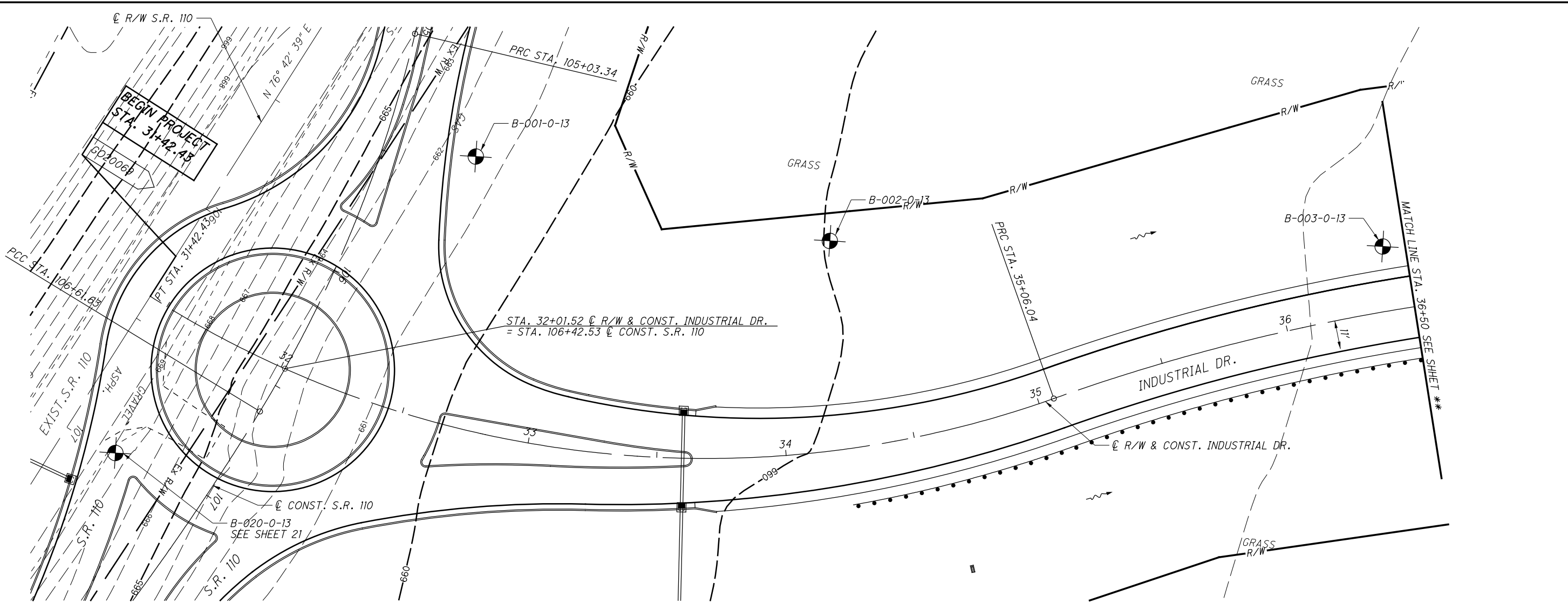


Tested By: KL Reviewed By: TR Date: 6/4/2014

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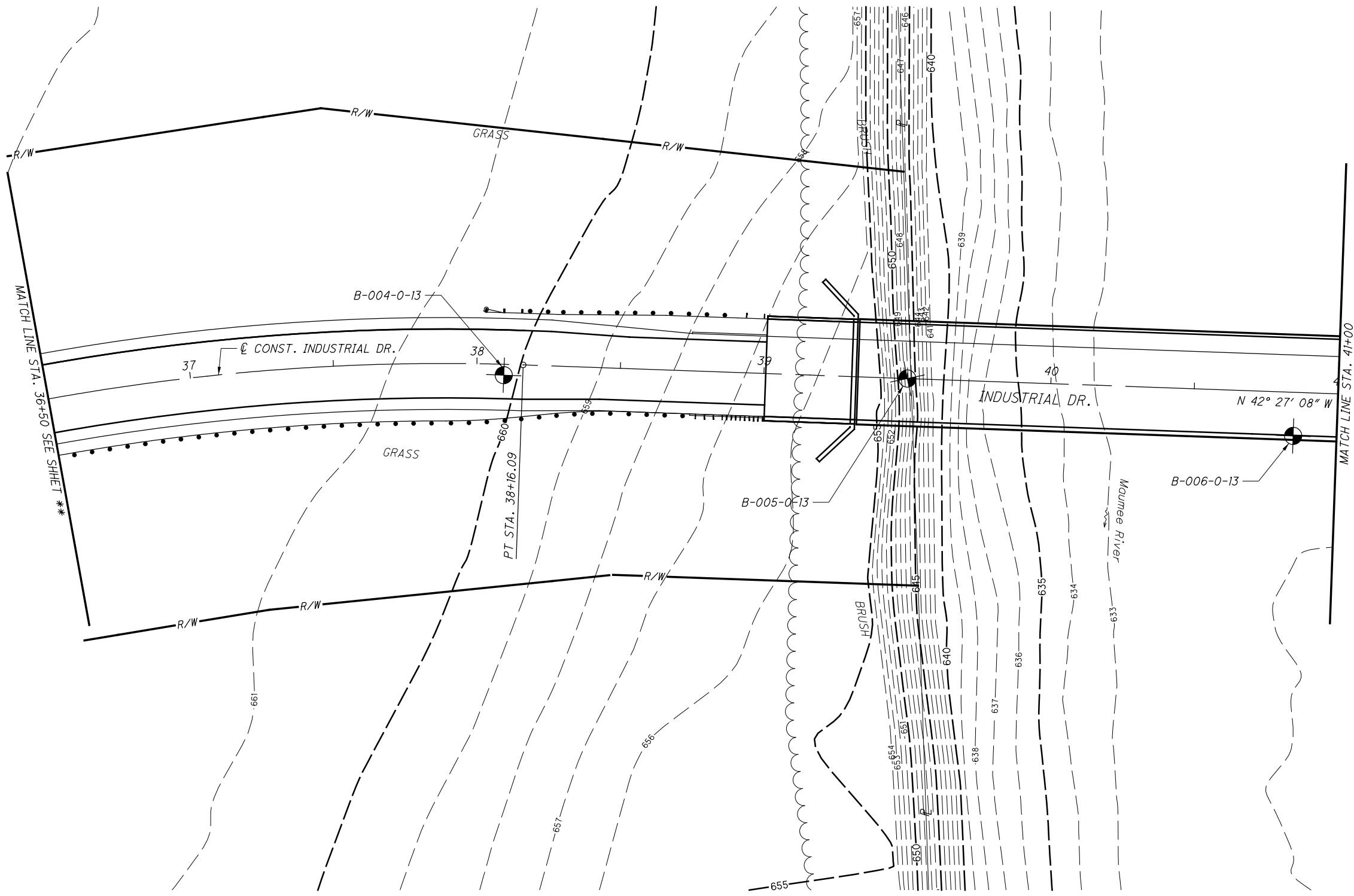
HEN-IND-0000

**PLAN AND PROFILE - INDUSTRIAL DR,
STA. 31+42.43 TO STA. 36+50.00**

157
180

DRAWN SVJ
CHECKED

0 20 40
HORIZONTAL SCALE IN FEET





 DRAWN SVJ

 CHECKED _____

 0 20 40

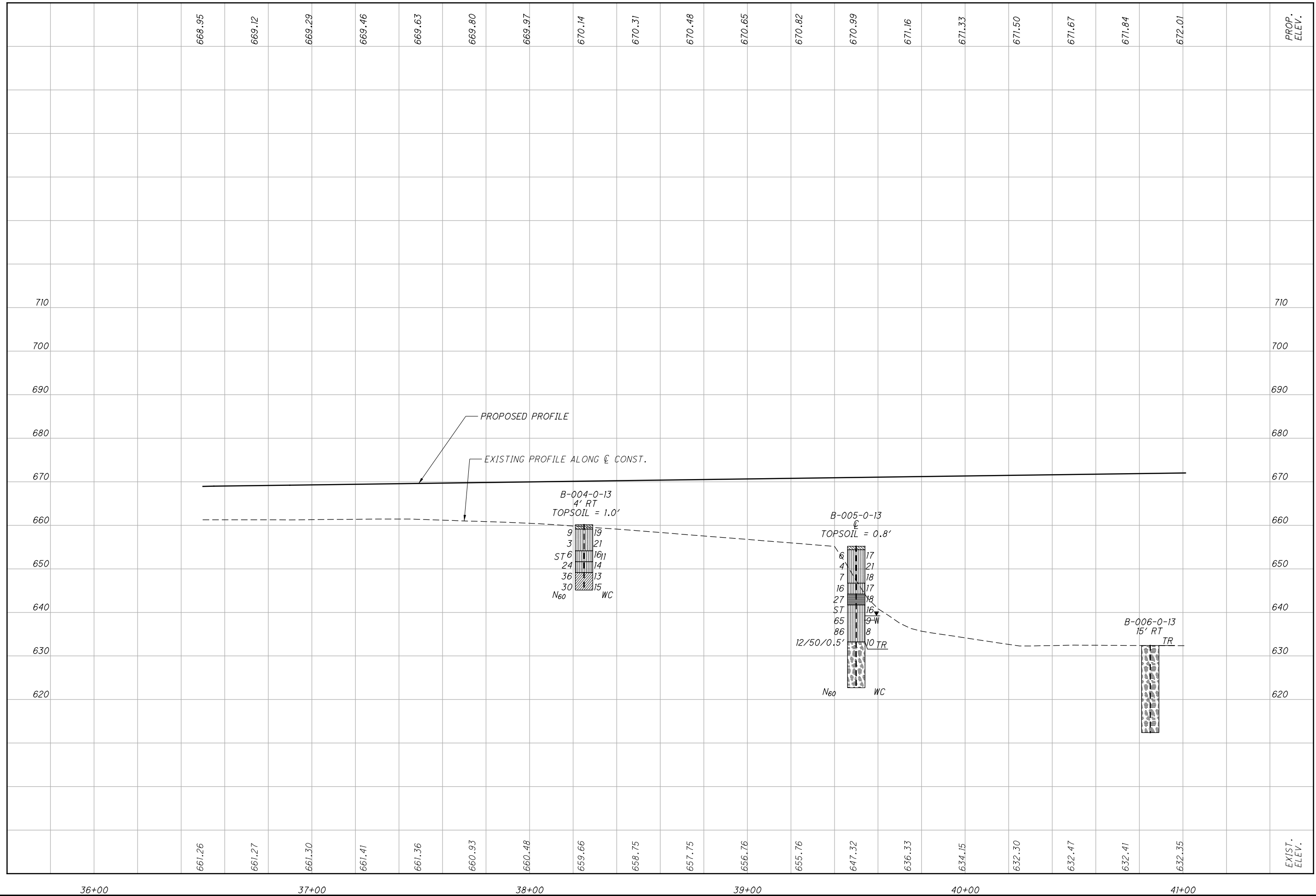
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 SCALE IN FEET

PLAN AND PROFILE - INDUSTRIAL DR.
STA. 36+50.00 TO STA. 41+00.00

HEN-IND-0000

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DRAWN SVJ
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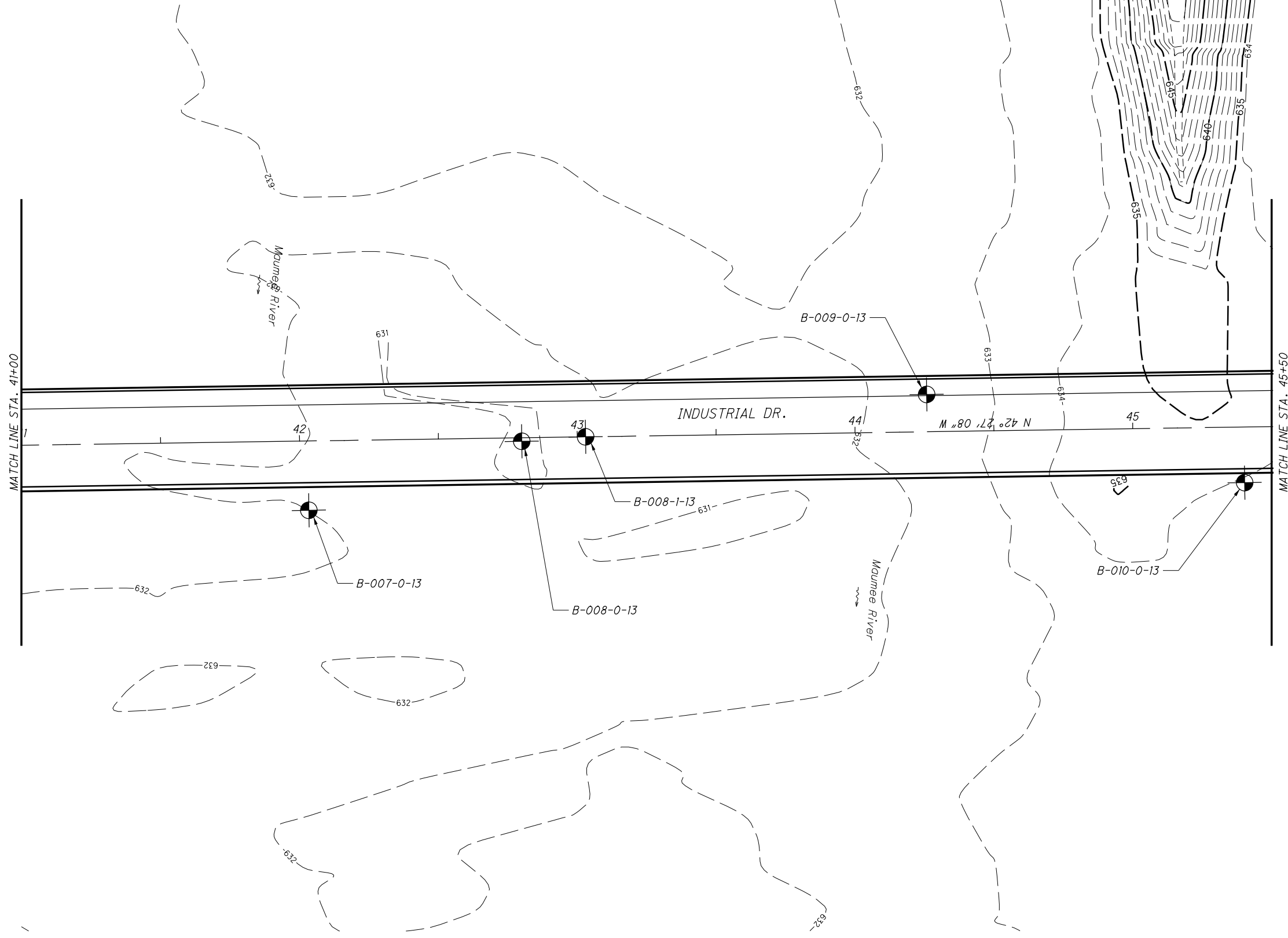
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 0 10 20 40

PLAN AND PROFILE - INDUSTRIAL DR.
STA. 36+50.00 TO STA. 41+00.00

HEN-IND-0000

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EXIST. ELEV.



160
180

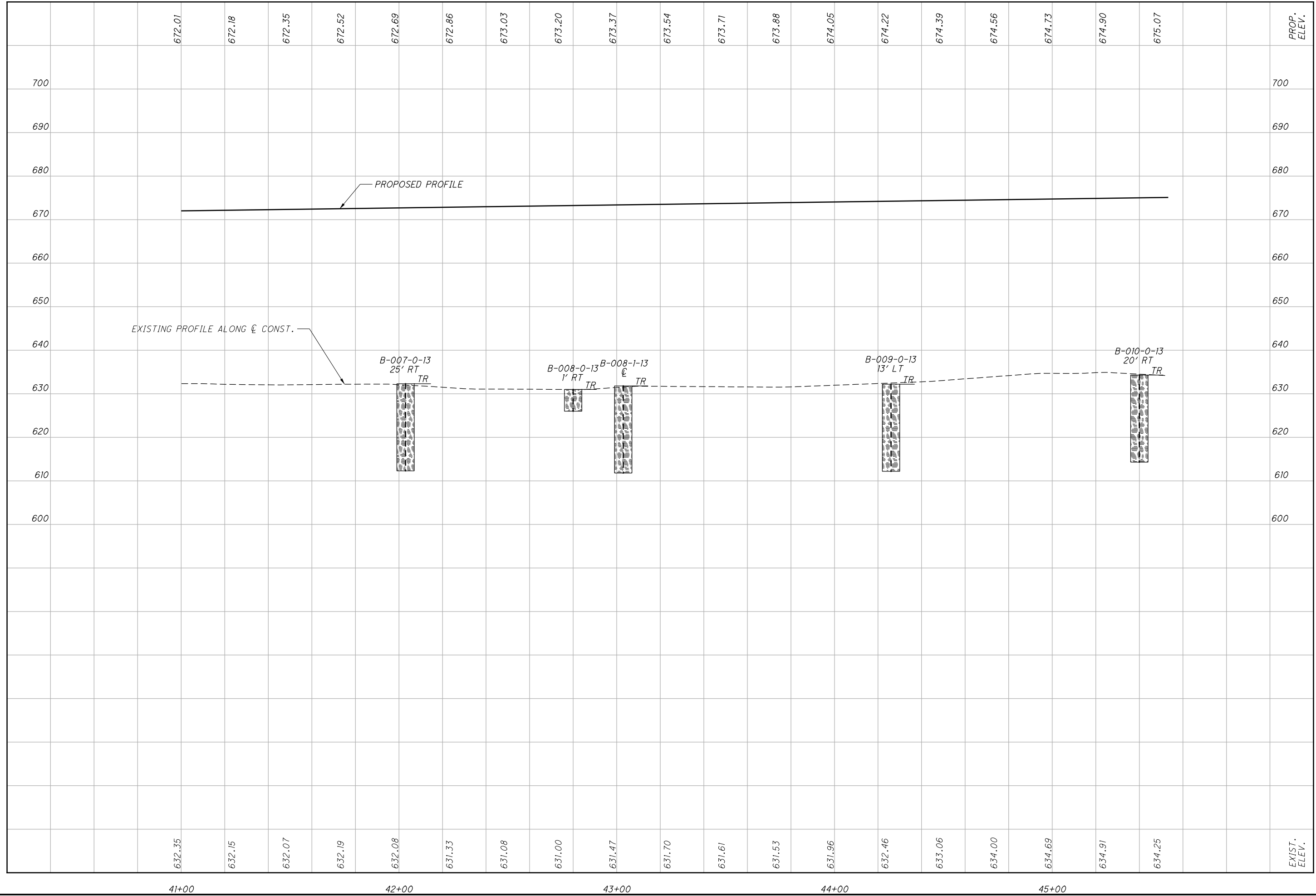
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PLAN AND PROFILE - INDUSTRIAL DR.
STA. 41+00.00 TO STA. 45+50.00

DRAWN SVJ
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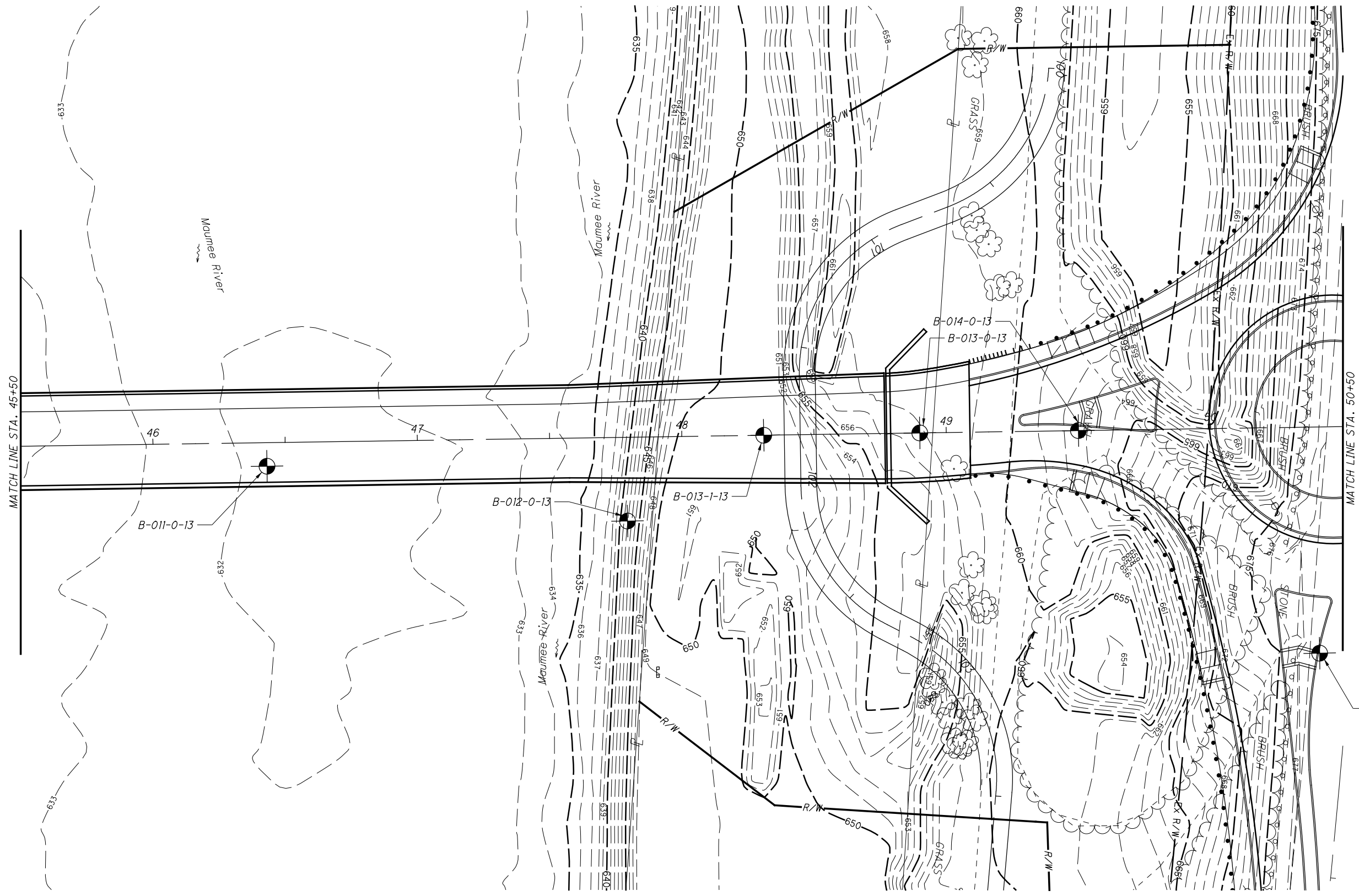


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HORIZONTAL SCALE IN FEET
0 10 20 40

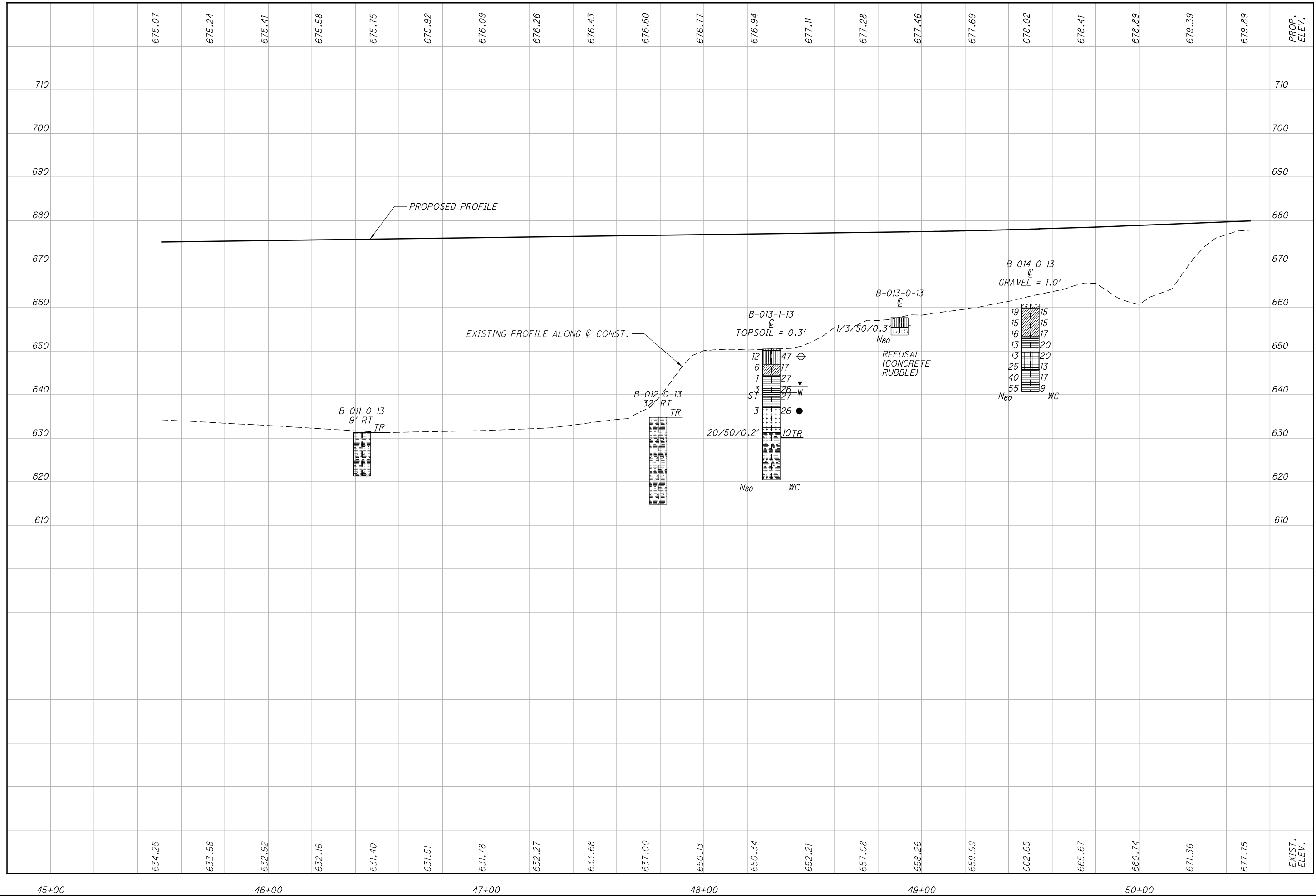
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HEN-IND-0000

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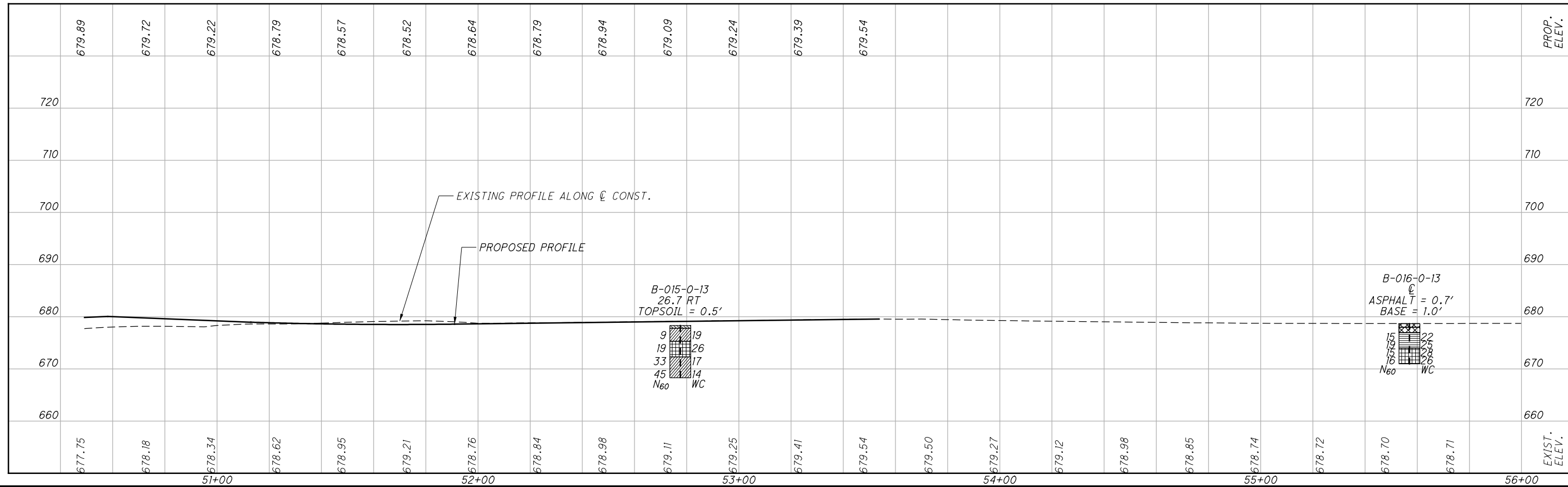
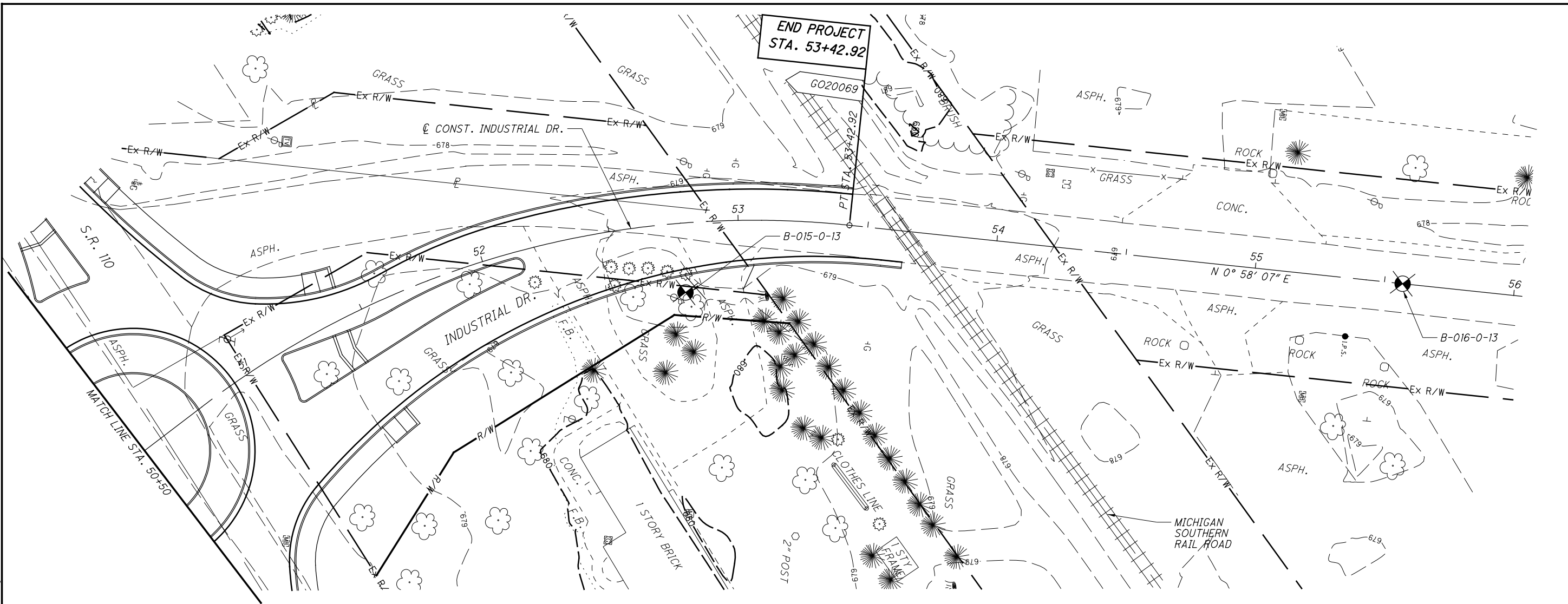
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 CHECKED _____
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 HORIZONTAL SCALE IN FEET

PLAN AND PROFILE - INDUSTRIAL DR.
STA. 45+50.00 TO STA. 50+50.00

HEN-IND-0000

163
 180
 EXIST. ELEV.

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PLAN AND PROFILE - INDUSTRIAL DR.
STA. 50+50.00 TO STA. 56+00.00

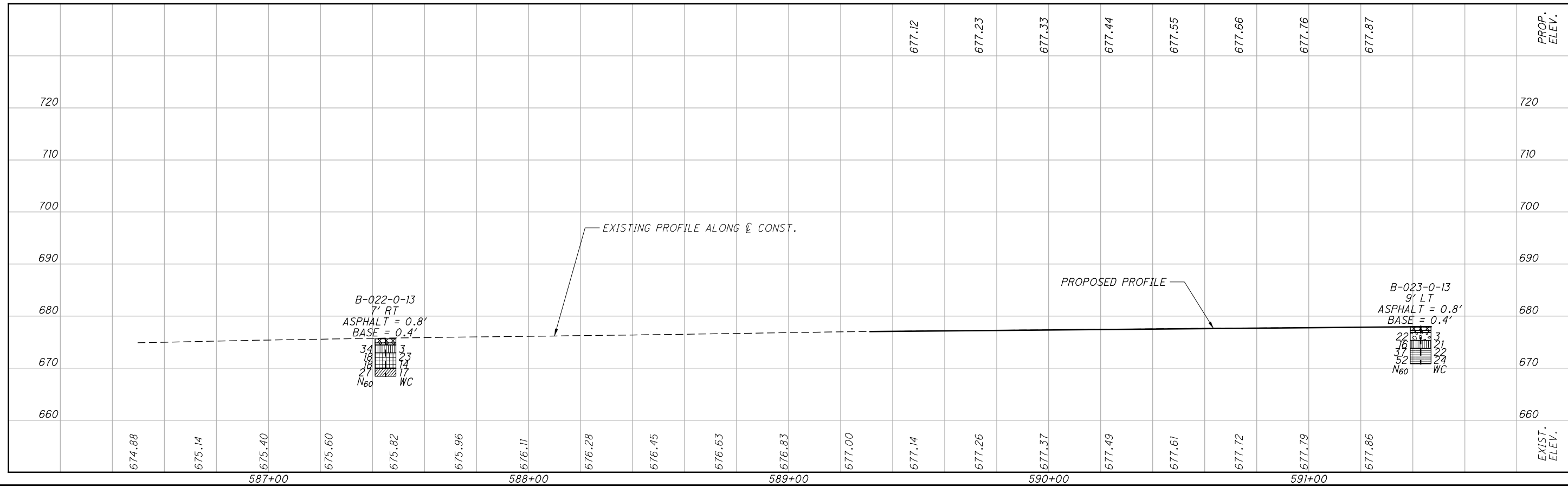
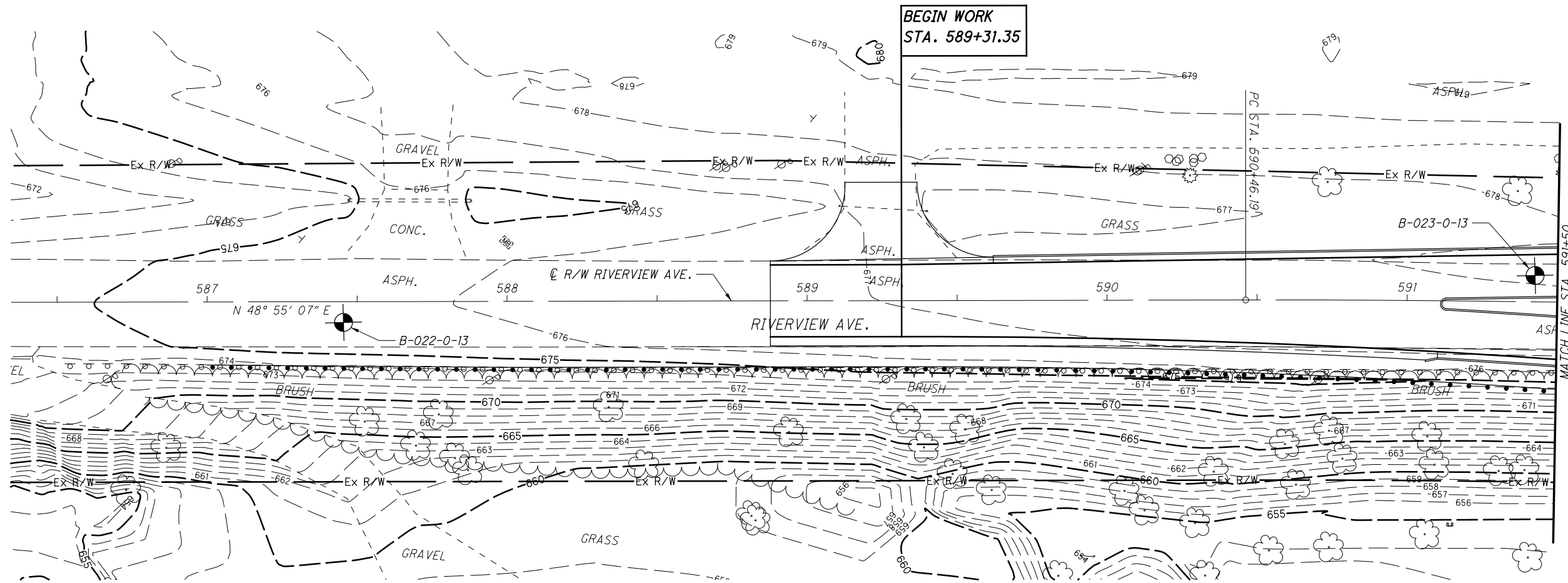
HEN-IND-0000

DRAWN SVJ
 CHECKED

164
 180

N
 10
 20
 40
 HORIZONTAL SCALE IN FEET

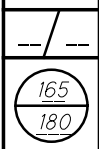
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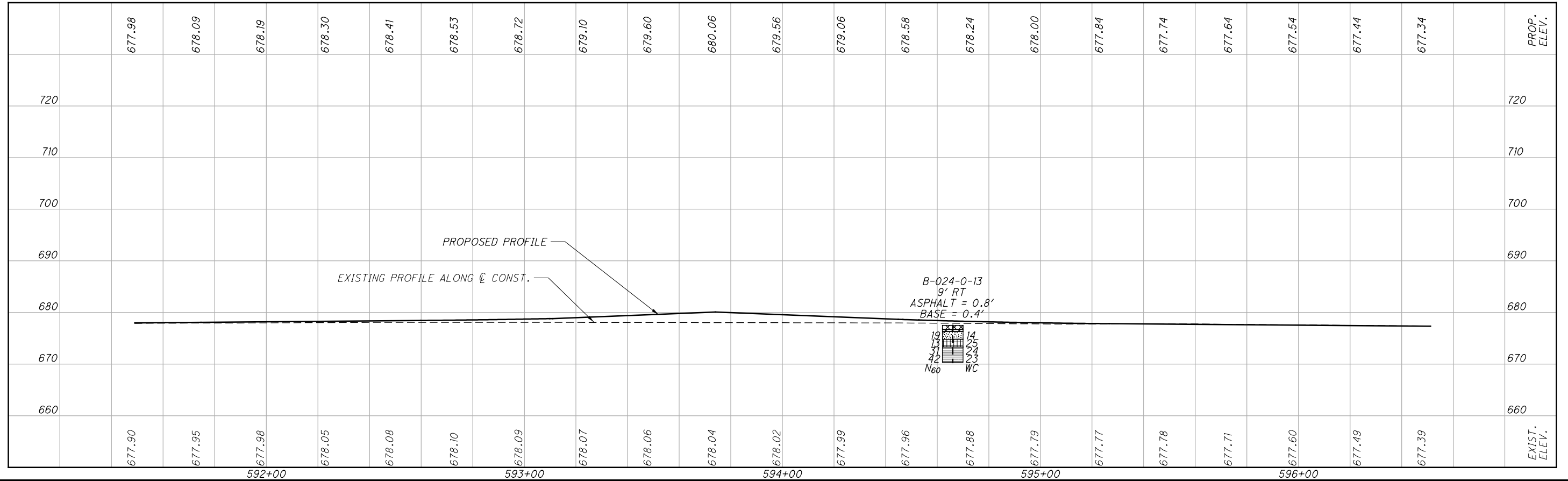
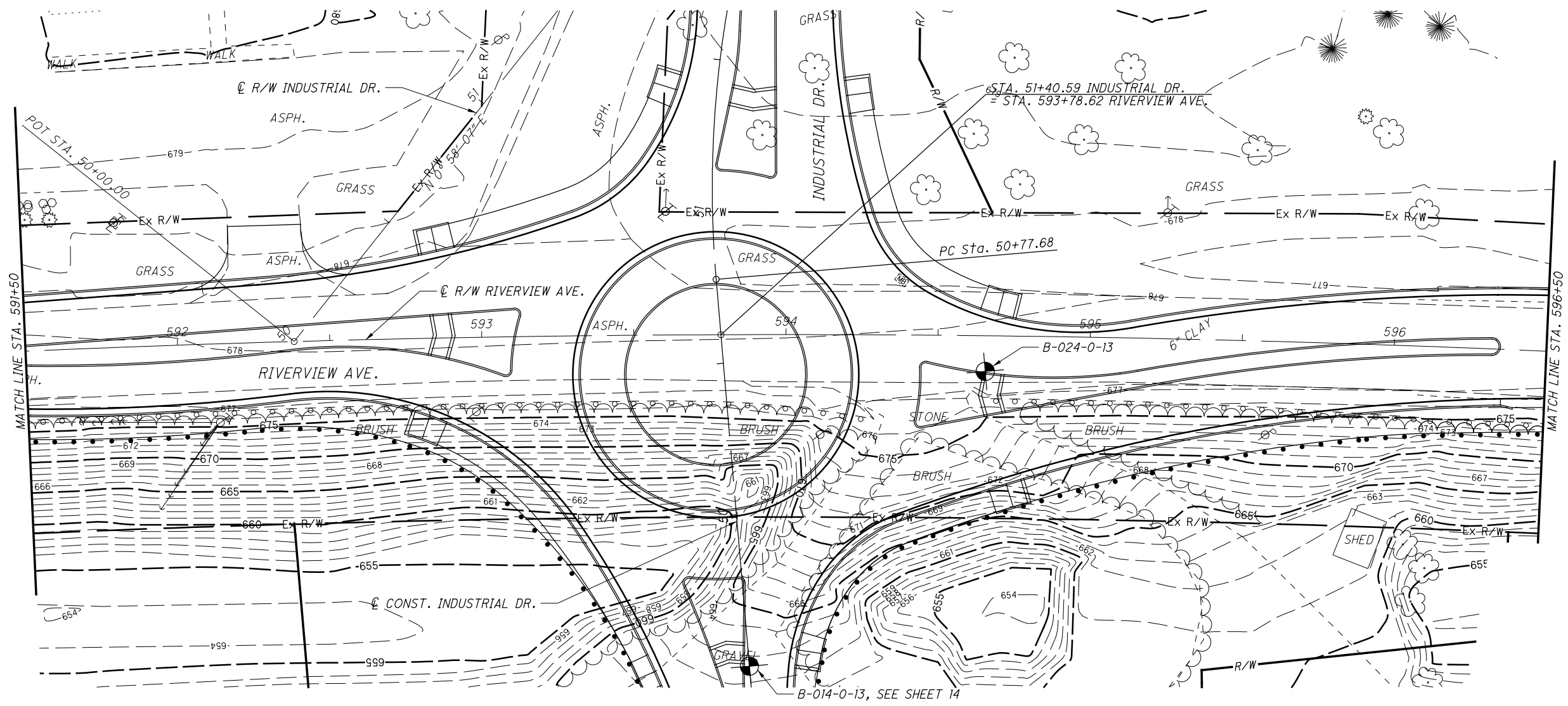
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CHECKED

**PLAN AND PROFILE - RIVERVIEW AVE.
STA. 586+50.00 TO STA. 591+50.00**

HEN-IND-0000



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PLAN AND PROFILE - RIVERVIEW AVE.
STA. 591+50.00 TO STA. 596+50.00

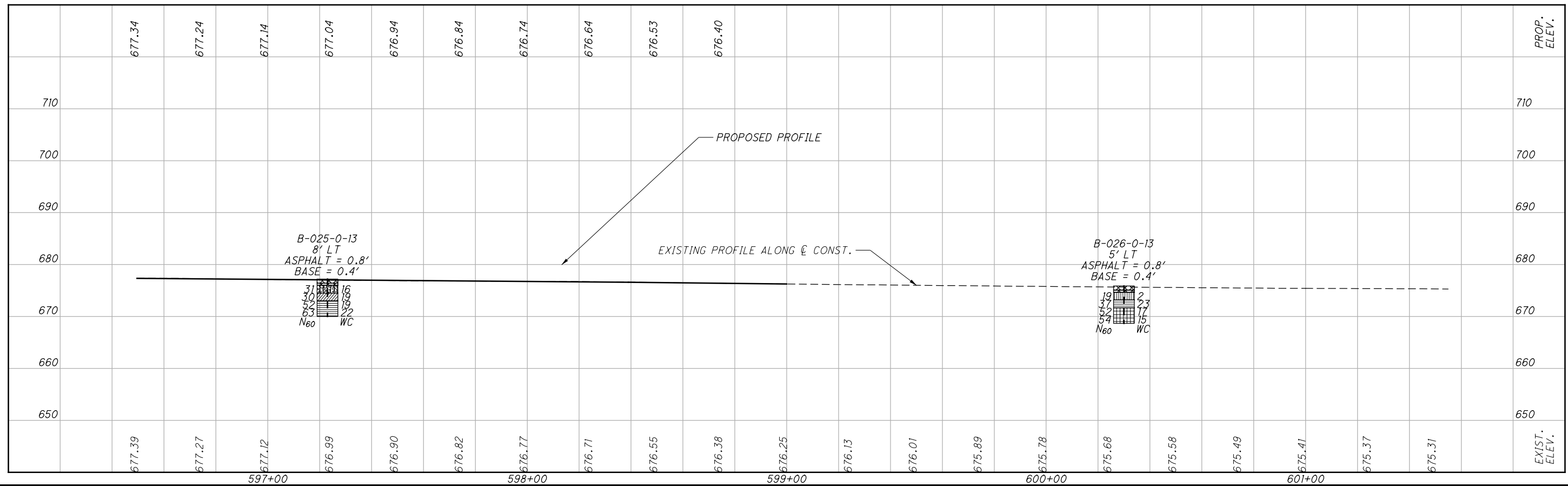
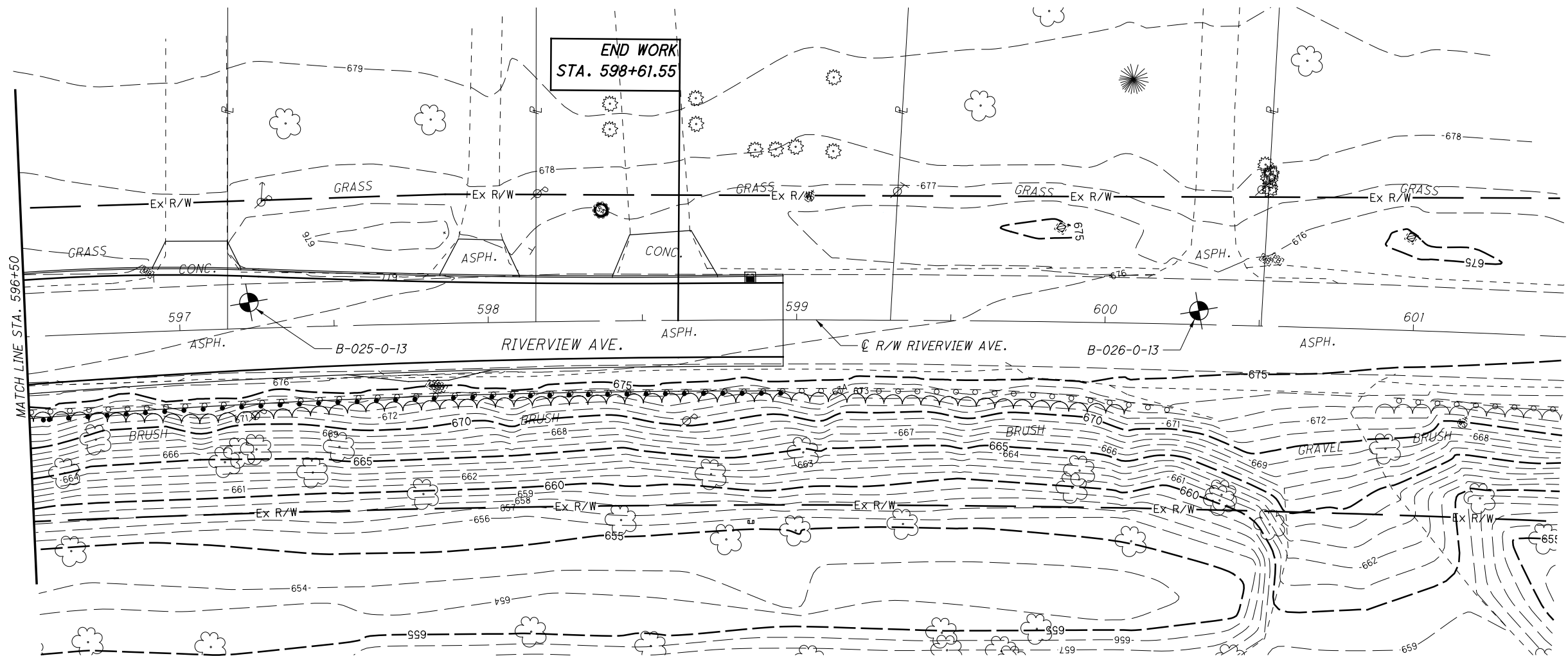
HEN-IND-0000

DRAWN SVJ
CHECKED

0 20 40
HORIZONTAL
SCALE IN FEET

166
180

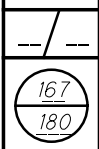
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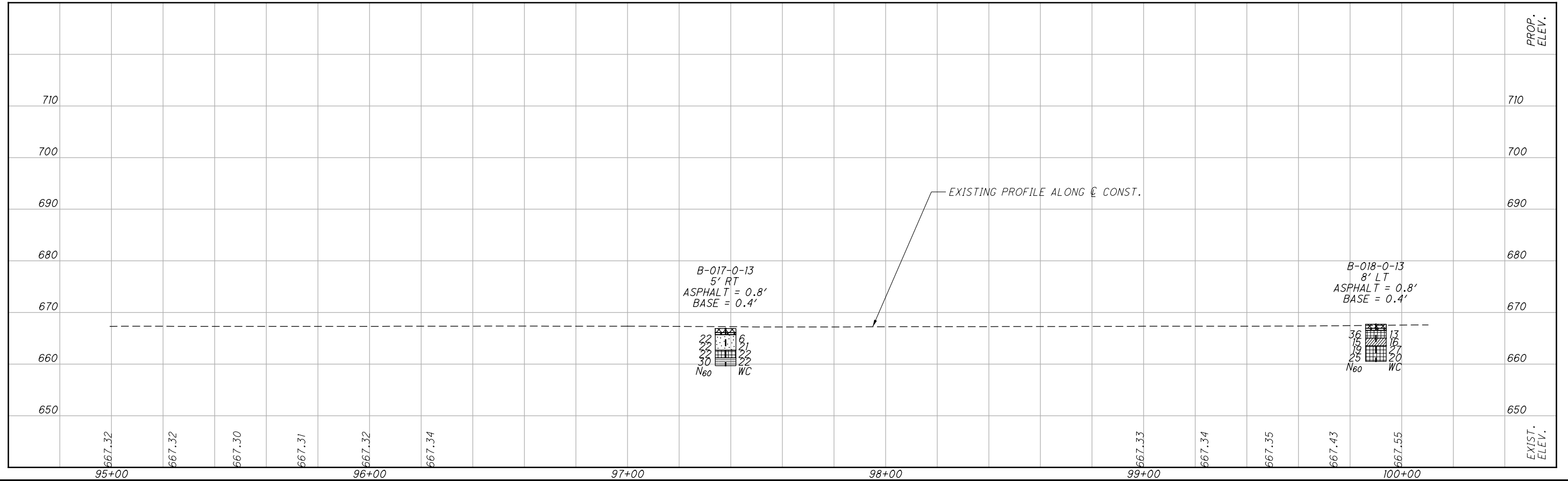
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PLAN AND PROFILE - RIVERVIEW AVE.
STA. 596+50 TO STA. 601+50.00

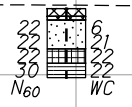
HEN-IND-0000



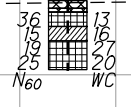
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B-017-0-13
5' RT
ASPHALT = 0.8'
BASE = 0.4'

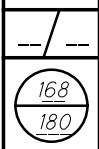


B-018-0-13
8' LT
ASPHALT = 0.8'
BASE = 0.4'



PLAN AND PROFILE - S.R.110
STA. 95+00.00 TO STA. 100+00.00

HEN-IND-0000



PROJECT: HEN-IND-0000
 TYPE: NEW ALIGNMENT
 PID: 22984 BR ID: N/A
 START: 4/23/14 END: 4/23/14
 DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT
 SAMPLING FIRM / LOGGER: MSG / M. WELKER
 DRILLING METHOD: 4.25" HSA / NW
 SAMPLING METHOD: SPT/ST/NW CORE BARREL
 DRILL RIG: GEOPROBE 7822DT
 HAMMER: AUTOMATIC HAMMER
 CALIBRATION DATE: 5/10/13
 ENERGY RATIO (%): 89.3
 STATION / OFFSET: 39+50. CL
 ALIGNMENT: INDUSTRIAL DR.
 ELEVATION: 655.2 (MSL) EOB: 32.5 ft.
 COORD: 636926.243 N, 1529055.188 E
 EXPLORATION ID: B-005-0-13
 PAGE: 1 OF 1

DEPTH (ft)	SPT/ROD	N ₆₀	REC SAMPLE (%)	HP ID	GR	GRADATION (%)						WC	ODOT CLASS (GI)	INST.	
						LL	PL	PI	FS	SI	CL				
1	1														
2	2	6	61	SS-1	0	1	25	45	29	16	9	17	A-4a (8)		
3															
4	1	4	94	SS-2	-	-	-	-	-	-	-	21	A-4a (V)		
5	2														
6	1	7	83	SS-3	-	-	-	-	-	-	-	18	A-4a (V)		
7	1	4													
8															
9	3	5	39	SS-4	14	5	19	39	23	23	16	17	A-4a (5)		
10	6														
11															
12	4	8	17	SS-5	-	-	-	-	-	-	-	18	A-6b (V)		
13	10														
14															
15															
16															
17	12	18	65	SS-6	-	-	-	-	-	-	-	9	A-4a (V)		
18	26														
19	19	25	86	SS-7	-	-	-	-	-	-	-	8	A-4a (V)		
20	33														
21	12	50	78	SS-8	-	-	-	-	-	-	-	10	A-4a (V)		
22	50														
23															
24															
25	8		72	RC-1									CORE		
26															
27															
28															
29															
30	9		100	RC-2									CORE		
31															
32															

SOIL DESCRIPTIONS:
 TOPSOIL
 Soft to medium stiff, brown **SANDY SILT**, some clay, damp
 Very stiff, brown **SANDY SILT**, some clay, little gravel, damp
 Very stiff, gray **SILTY CLAY**, trace sand and gravel, damp
 Hard, dark gray **SANDY SILT**, some clay, trace gravel, damp
SHALE, dark brown to black, slightly to moderately weathered, thinly laminated, weak to moderately strong

NOTES: NONE
 ABANDONMENT METHODS, MATERIALS, QUANTITIES: 1 BAG BENTONITE CHIPS; SOIL CUTTINGS

PROJECT: HEN-IND-0000
 TYPE: NEW ALIGNMENT
 PID: 22984 BR ID: N/A
 START: 6/10/14 END: 6/10/14

DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT
 SAMPLING FIRM / LOGGER: MSG / N. BREJIAK
 DRILLING METHOD: NW
 SAMPLING METHOD: NW CORE BARREL

DRILL RIG: GEOPROBE 7822DT
 HAMMER: AUTOMATIC HAMMER
 CALIBRATION DATE: 5/10/13
 ENERGY RATIO (%): 89.3

STATION / OFFSET: 40+85.15 RT
 ALIGNMENT: INDUSTRIAL DR
 ELEVATION: 632.4 (MSL) EOB: 20.0 ft.
 COORD: 637037.666 N, 1528976.800 E

EXPLORATION ID: B-006-0-13
 PAGE: 1 OF 1

MATERIAL DESCRIPTION AND NOTES

SHALE, dark gray to gray, moderately weathered, slightly strong to strong

-Unconfined compressive strength (Qu) = 1,423 psi

Becomes severely weathered and weak

Becomes moderately weathered and strong

-Unconfined compressive strength (Qu) = 1,641 psi

DEPTH	SPT/ RQD	N ₆₀	REC SAMPLE (%)	HP (tsf)	GRADATION (%)					ATTERBERG	INST.	
					GR	CS	FS	SI	CL			LL
1												
2	22		97									CORE
3												
4												
5												
6												
7	45		100									CORE
8												
9												
10												
11												
12												
13	17		83									CORE
14												
15												
16												
17	62		100									CORE
18												
19												
20												

612.4 EOB

STANDARD ODOT SOIL BORING LOG (11 X 17) - OH DOT.GDT - 10/6/14 11:34 - W:\PROJECTS\PROJECTS F-J\H2530002\22984\GEOTECHNICAL\LABUPDATED.GPJ

NOTES: BORING PERFORMED ON BARGE. WATER LEVEL IN RIVER ESTIMATED AT ELEVATION 639 FEET DURING DRILLING
 ABANDONMENT METHODS, MATERIALS, QUANTITIES: NATURAL COLLAPSE

PROJECT: HEN-IND-0000
 TYPE: NEW ALIGNMENT
 PID: 22984 BR ID: N/A
 START: 6/5/14 END: 6/5/14

DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT
 SAMPLING FIRM / LOGGER: MSG / N. BRELUAK
 DRILLING METHOD: NW
 SAMPLING METHOD: NW CORE BARREL

DRILL RIG: GEOPROBE 7822DT
 HAMMER: AUTOMATIC HAMMER
 CALIBRATION DATE: 5/10/13
 ENERGY RATIO (%): 89.3

STATION / OFFSET: 42+03.25 RT
 ALIGNMENT: INDUSTRIAL DR
 ELEVATION: 632.3 (MSL) EOB: 20.0 ft.
 COORD: 637131.112 N, 1528904.521 E

EXPLOSION ID: B-007-0-13
 PAGE: 1 OF 1

MATERIAL DESCRIPTION
AND NOTES

SHALE, dark brown to gray, moderately weathered, slightly strong to strong

-Unconfined compressive strength (Qu) = 4,446 psi

DEPTH	ELEV.	SPT/ROD		REC SAMPLE ID	HP (tsf)	GRADATION (%)						DOOT CLASS (GI)	INST.
		N ₆₀	QD			GR	CS	FS	SI	CL	LL		
1	632.3	9		90 RC-1									CORE
2													
3													
4													
5													
6													
7		29		83 RC-2									CORE
8													
9													
10													
11													
12													
13		42		48 RC-3									CORE
14													
15													
16													
17													
18		43		80 RC-4									CORE
19													
20	612.3												

NOTES: BORING PERFORMED ON BARGE. WATER LEVEL IN RIVER ESTIMATED AT ELEVATION 639 FEET DURING DRILLING
 ABANDONMENT METHODS, MATERIALS, QUANTITIES: NATURAL COLLAPSE

PROJECT: HEN-IND-0000 TYPE: NEW ALIGNMENT PID: 22984 BR ID: N/A START: 6/11/14 END: 6/11/14		DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT SAMPLING FIRM / LOGGER: MSG / N. BREJIAK DRILLING METHOD: NW SAMPLING METHOD: NW CORE BARREL		DRILL RIG: GEOPROBE 7822DT HAMMER: AUTOMATIC HAMMER CALIBRATION DATE: 5/10/13 ENERGY RATIO (%): 89.3		STATION / OFFSET: 44+26, 13 LT ALIGNMENT: INDUSTRIAL DR. ELEVATION: 632.2 (MSL) EOB: 20.0 ft COORD: 637269.805 N, 1528725.628 E		EXPLORATION ID B-009-0-13 PAGE 1 OF 1												
MATERIAL DESCRIPTION AND NOTES		ELEV.	DEPTH	SPT/ RQD	N ₆₀	REC SAMPLE (%)	ID	HP (tst)	GRADATION (%)			ATTERBERG			ODOT CLASS (GI)	INST.				
		632.2							GR	CS	FS	SI	CL	LL	PL	PI	WC			
<p>SHALE, dark gray to gray, moderately weathered, weak to strong</p> <p>-Unconfined compressive strength (Qu) = 1,451 psi</p>			1																	
			2	23	95	RC-1													CORE	
			3																	
			4																	
			5																	
			6																	
			7			37	100	RC-2												CORE
			8																	
			9																	
			10																	
			11																	
			12																	
			13			40	100	RC-3												CORE
			14																	
			15																	
			16																	
			17																	
			18			65	100	RC-4												CORE
			19																	
			20	612.2	EOB															

STANDARD ODOT SOIL BORING LOG (11 X 17) - OH DOT.GDT - 10/6/14 11:34 - W:\PROJECTS\PROJECTS F-J\H2530002\22984\GEOTECHNICAL\LAB\RP\DATED.GPJ

NOTES: BORING PERFORMED ON BARGE. WATER LEVEL IN RIVER ESTIMATED AT ELEVATION 640 FEET DURING DRILLING
ABANDONMENT METHODS, MATERIALS, QUANTITIES: NATURAL COLLAPSE

PROJECT: HEN-IND-0000
 TYPE: NEW ALIGNMENT
 PID: 22984 BR ID: N/A
 START: 6/6/14 END: 6/6/14

DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT
 SAMPLING FIRM / LOGGER: MSG / N. BREJIAK
 DRILLING METHOD: NW
 SAMPLING METHOD: NW CORE BARREL

MATERIAL DESCRIPTION
AND NOTES
 SHALE dark gray to gray, moderately to severely weathered, weak to strong

DRILL RIG: GEOPROBE 7822DT
 HAMMER: AUTOMATIC HAMMER
 CALIBRATION DATE: 5/10/13
 ENERGY RATIO (%): 89.3

STATION / OFFSET: 45+40, 20 RT
 ALIGNMENT: INDUSTRIAL DR.
 ELEVATION: 634.3 (MSL) EOB: 20.0 ft.
 COORD: 637376.638 N, 1528673.227 E

EXPLORATION ID
 B-010-0-13

REC SAMPLE HP (tst)
 ID (%)

SPT/ RQD N₆₀

GRADATION (%)
 GR CS FS SI CL LL PL PI WC

ODOT CLASS (GI)
 INST.

DEPTH	REC SAMPLE HP (%)	SPT/ RQD N ₆₀	GRADATION (%)	ODOT CLASS (GI)	INST.
1					
2					
3					
4					
5					
6					
7	100 RC-1	7			CORE
8					
9					
10					
11					
12					
13					
14					
15					
16					
17					
18					
19					
20					

ELEV. 634.3
 614.3 EOB

-Unconfined compressive strength (Qu) = 7,676 psi

NOTES: BORING PERFORMED ON BARGE. WATER LEVEL IN RIVER ESTIMATED AT ELEVATION 640 FEET DURING DRILLING
 ABANDONMENT METHODS, MATERIALS, QUANTITIES: NATURAL COLLAPSE

STANDARD ODOT SOIL BORING LOG (11 X 17) - OH DOT.GDT - 10/6/14 11:34 - W:\PROJECTS\PROJECTS F-J\H2530002\22984\GEOTECHNICAL\LAB\RP\DATED.GPJ

PROJECT: HEN-IND-0000 TYPE: NEW ALIGNMENT PID: 22984 BR ID: N/A START: 6/6/14 END: 6/6/14	DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT SAMPLING FIRM / LOGGER: MSG / N. BREJIAK DRILLING METHOD: NW SAMPLING METHOD: NW CORE BARREL	DRILL RIG: GEOPROBE 7822DT HAMMER: AUTOMATIC HAMMER CALIBRATION DATE: 5/10/13 ENERGY RATIO (%): 89.3	STATION / OFFSET: 46+43.9 RT ALIGNMENT: INDUSTRIAL DR. ELEVATION: 631.3 (MSL) EOB: 10.0 ft COORD: 637445.066 N, 1528595.585 E	EXPLORATION ID B-011-0-13 PAGE 1 OF 1																																																															
MATERIAL DESCRIPTION																																																																			
AND NOTES																																																																			
SHALE dark brown-gray, moderately to highly weathered, strong to weak																																																																			
-Unconfined compressive strength (Qu) = 4,644 psi																																																																			
<table border="1"> <thead> <tr> <th rowspan="2">SPT/ RQD</th> <th rowspan="2">N₆₀</th> <th rowspan="2">REC SAMPLE (%)</th> <th rowspan="2">ID</th> <th rowspan="2">HP (tst)</th> <th colspan="5">GRADATION (%)</th> <th rowspan="2">ODOT CLASS (GI)</th> <th rowspan="2">INST.</th> </tr> <tr> <th>GR</th> <th>CS</th> <th>FS</th> <th>SI</th> <th>CL</th> <th>LL</th> <th>PL</th> <th>PI</th> <th>WC</th> </tr> </thead> <tbody> <tr> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>27</td> <td></td> <td>100</td> <td>RC-1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>CORE</td> </tr> <tr> <td>17</td> <td></td> <td>100</td> <td>RC-2</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>CORE</td> </tr> </tbody> </table>					SPT/ RQD	N ₆₀	REC SAMPLE (%)	ID	HP (tst)	GRADATION (%)					ODOT CLASS (GI)	INST.	GR	CS	FS	SI	CL	LL	PL	PI	WC															27		100	RC-1										CORE	17		100	RC-2										CORE
SPT/ RQD	N ₆₀	REC SAMPLE (%)	ID	HP (tst)						GRADATION (%)							ODOT CLASS (GI)	INST.																																																	
					GR	CS	FS	SI	CL	LL	PL	PI	WC																																																						
27		100	RC-1										CORE																																																						
17		100	RC-2										CORE																																																						

621.3 EOB 10

NOTES: BORING PERFORMED ON BARGE. WATER LEVEL IN RIVER ESTIMATED AT ELEVATION 640 FEET DURING DRILLING
ABANDONMENT METHODS, MATERIALS, QUANTITIES: NATURAL COLLAPSE

PROJECT: HEN-IND-0000 TYPE: NEW ALIGNMENT PID: 22984 BR ID: N/A START: 6/7/14 END: 6/7/14		DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT SAMPLING FIRM / LOGGER: MSG / N. BREJIAK DRILLING METHOD: NW SAMPLING METHOD: NW CORE BARREL		DRILL RIG: GEOPROBE 7822DT HAMMER: AUTOMATIC HAMMER CALIBRATION DATE: 5/10/13 ENERGY RATIO (%): 89.3		STATION / OFFSET: 47+79.32 RT ALIGNMENT: INDUSTRIAL DR. ELEVATION: 634.8 (MSL) EOB: 20.0 ft. COORD: 637560.130 N, 1528521.160 E						EXPLORATION ID B-0120-13 PAGE 1 OF 1				
MATERIAL DESCRIPTION AND NOTES				REC SAMPLE HP (%) ID (tst)		GRADATION (%)						ODOT CLASS (GI)				
ELEV. 634.8				SPT/ RQD		GR CS FS SI CL		LL PL PI		WC		INST.				
DEPTHS				N ₆₀		RC-1		RC-2		RC-3		RC-4				
<p>SHALE dark brown-dark gray, moderately weathered, weak to strong</p> <p>-Unconfined compressive strength (Qu) = 5,539 psi</p> <p>Unconfined compressive strength (Qu) = 6,652 psi</p>				1												
				2	13	100									CORE	
				3												
				4												
				5												
				6												
				7	18	100										CORE
				8												
				9												
				10												
				11												
				12												
				13	40	92										CORE
				14												
				15												
				16												
				17	8	83										CORE
				18												
				19												
				20												
ELEV. 614.8				EOB												

NOTES: BORING PERFORMED ON BARGE. WATER LEVEL IN RIVER ESTIMATED AT ELEVATION 641 FEET DURING DRILLING
ABANDONMENT METHODS, MATERIALS, QUANTITIES: NATURAL COLLAPSE

STANDARD ODOT SOIL BORING LOG (11 X 17) - OH DOT.GDT - 10/6/14 11:34 - W:\PROJECTS\PROJECTS F-J\H2530002\22984\GEOTECHNICAL\LAB\PDATED.GPJ

PROJECT: HEN-IND-0000		DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT		DRILL RIG: GEOPROBE 7822DT		STATION / OFFSET: 48+90. CL		EXPLORATION ID	
TYPE: NEW ALIGNMENT		SAMPLING FIRM / LOGGER: MSG / J. FAITEL		HAMMER: AUTOMATIC HAMMER		ALIGNMENT: INDUSTRIAL DR.		B-013-0-13	
PID: 22984 BR ID: N/A		DRILLING METHOD: 4.25" HSA		CALIBRATION DATE: 5/10/13		ELEVATION: 657.7 (MSL) EOB: 4.0 ft.		PAGE	
START: 4/22/14 END: 4/22/14		SAMPLING METHOD: SPT		ENERGY RATIO (%): 89.3		COORD: 637621.240 N, 1528422.325 E		1 OF 1	
MATERIAL DESCRIPTION		ELEV.		SPT/ RQD		GRADATION (%)		ODOT CLASS (GI)	
AND NOTES		DEPTHS		N ₆₀		GR CS FS SI CL		LL PL PI WC	
								INST.	
Very loose, brown SANDY SILT, little gravel, trace clay; damp (FILL)		1		1					
		2		38		-		-	
CONCRETE RUBBLE		3		-		-		-	
		4		-		-		-	
		EOB		653.7					

NOTES: REFUSAL IN CONCRETE RUBBLE AT 4'
 ABANDONMENT METHODS, MATERIALS, QUANTITIES: BENTONITE CHIPS; SOIL CUTTINGS

PROJECT: HEN-IND-0000 NEW ALIGNMENT		DRILLING FIRM / OPERATOR: MSG / R. SCHIPPERT SAMPLING FIRM / LOGGER: MSG / J. FAITEL		DRILL RIG: GEOPROBE 7822DT HAMMER: AUTOMATIC HAMMER		STATION / OFFSET: 48+31. CL		EXPLORATION ID B-013-1-13											
PID: 22984 BR ID: N/A		DRILLING METHOD: 4.25" HSA / NW		CALIBRATION DATE: 5/10/13		ALIGNMENT: INDUSTRIAL DR.		ELEVATION: 650.5 (MSL) EOB: 30.0 ft.											
START: 4/22/14 END: 4/22/14		SAMPLING METHOD: SPT/ST/NW CORE BARREL		ENERGY RATIO (%): 89.3		COORD: 637577.840 N, 1528462.050 E		PAGE 1 OF 1											
MATERIAL DESCRIPTION AND NOTES		ELEV.	DEPTH	SPT/ RQD	N ₆₀	REC SAMPLE (%)	ID	HP (tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	ODOT CLASS (GI)	INST.
TOPSOIL Loose, brown SANDY SILT , some clay, trace organics; damp		650.5 650.2																	
Medium stiff, dark brown SILT AND CLAY , some sand; damp		647.0		2 4 4	12	100	SS-1	-	-	-	-	-	-	-	-	-	47	A-4a (V)	
Very soft to soft, dark brown SILTY CLAY , trace sand; moist		644.5		2 2 2	6	39	SS-2	1.25	0	1	28	41	30	30	16	14	17	A-6a (9)	
Soft to medium stiff, brown mottled with gray SILTY CLAY , trace sand; moist		640.5	▼	0 0 1	1	100	SS-3	0.00	-	-	-	-	-	-	-	-	27	A-6b (V)	
Soft, brown SILT , some clay and sand; wet		637.0		0 0 2	3	100	SS-4	1.00	-	-	-	-	-	-	-	-	26	A-6b (V)	
Hard, brown SILT , some clay and sand; wet		632.5 631.3		20 50/2"	-	100	SS-6	-	-	-	-	-	-	-	-	-	10	A-4b (V)	
SHALE , dark brown to brown, slightly to moderately weathered, thinly laminated, weak to moderately strong				43	95	95	RC-1											CORE	
				47	100	100	RC-2											CORE	
		620.5	EOB																

NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: 1 BAG BENTONITE CHIPS; SOIL CUTTINGS



HEN - IND - 0000

SOIL PROFILE
SOIL BORING LOGS

DRAWN
CHECKED